

# 244 PERIMETER CENTER

244 PERIMETER CENTER PKWY NE, DUNWOODY, GA

THE GEORGETOWN COMPANY **ROCAPOINT PARTNERS** 

SCHEMATIC DESIGN SUBMISSION

12.18.2020

**S9**ARCHITECTURE

#### **PROJECT DESCRIPTION**

244 PERIMETER CENTER IS A PROPOSED RENOVATION AND EXPANSION FOR AN EXISTING OFFICE BUILDING IN DUNWOODY, GA. THE PROJECT IS INCLUDED IN A MASTER PLAN PROJECTING A MIXED-USE RE-DEVELOPMENT OF THE SITE.

THE EXISTING BUILDING HAS THREE LEVELS AND A CELLAR WITH A CONCRETE STRUCTURE AND THE EXPANSION INCLUDES TWO MORE LEVELS ABOVE THEM WITH A HEAVY RUCTURE. THE SCOPE IN THE EXISTING BUILDING INCLUDES A WINDOW REPLACEMENT. CLEANING OF FACADE. ADDITION OF TERRACES, CREATION OF A PASSAGE ON GROUND FLOOR, RELOCATION OF THE BUILDING ELEVATORS, AND GENERAL RENOVATION. THE NEW FLOORS INCLUDE ADDITIONAL OFFICE AREA AND BUILDING AMENITIES.

OCCUPANCY GROUP B-BUSINESS, GROUP M-MERCANTILE

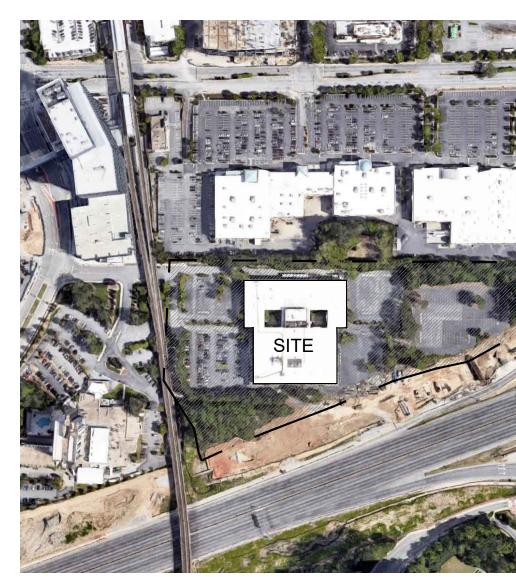
ADDRESS:	244 PERIMETER CENTER PKWY NE
CITY	DUNWOODY, GA
COUNTY	DEKALB COUNTY
LAND LOT:	329 & 300 - 18TH DISTRICT

PARCEL NO: 18-329-04-005 ZONING OI, PERIMETER CENTER OVERLAY CODE: 2018 INTERNATIONAL BUILDING CODE

TYPE OF CONSTRUCTION: TYPE I, IV **PROPOERTY AREA:** YEAR OF BUILDING: FIRE SPRINKLERS:

12.84 ACRES 1974 YES

### AREA MAP



#### **GENERAL NOTES**

1. <u>CODE COMPLIANCE:</u> ALL WORK PERFORMED IN ACCORDANCE TO THE 2018 INTERNATIONAL BUILDING CODE. PLUMBING, MECHANICAL, PROPERTY MAINTENANCE, ENERGY, AND FIRE CODES PER INTERNATIONAL CODE. SEE CODE DATA SUMMARY FOR ADDITIONAL INFORMATION.
2. TO THE FULLEST EXTENT PERMITTED BY LAW, THE CONTRACTOR SHALL INDEMNIFY AND HOLD HARMLESS THE OWNER, ARCHITECT, ENGINEERS, AND AGENTS AND EMPLOYEES OF ANY OF THEM FROM AND AGAINST CLAIMS, DAMAGES, LOSSES AND EXPENSESNCLUDING BUT NOT LIMITED TO ATTORNEYS' FEES, ARISING OUT OF OR RESULTING FROM PERFORMANCE OF THE WORK, PROVIDED THAT SUCH CLAIM, DAMAGE, LOSS OR EXPENSE IS ATTRIBUTABLE TO BODILY INJURY, SICKNESS, DISEASE OR DEATH, OR TO INJURY TO OR DESTRUCTION OF TANGIBLE PROPERTY (OTHER THAN THE WORK ITSELF) INCLUDING LOSS OF USE RESULTING THEREFROM, BUT ONLY TO THE EXTENT CAUSED IN WHOLE OR IN PART BY NEGLIGENT ACTS OR OMISSIONS OF THE CONTRACTOR, A SUBCONTRACTOR, ANYONE DIRECTLY OR INDIRECTLY EMPLOYED BY THEM OR ANYONE FOR WHOSE ACTS THEY MAY BE LIABLE, REGARDLESS OF WHETHER OR NOT SUCH CLAIM, DAMAGE, LOSS, OR EXPENSE IS CAUSED IN PART BY A PARTY INDEMNIFIED HEREUNDER. SUCH OBLIGATION SHALL NOT BE CONSTRUED TO NEGATE, ABRIDGE, OR REDUCE OTHER RIGHTS OR OBLIGATIONS OF INDEMNITY WHICH WOULD OTHERWISE EXIST AS TO A PARTY OR PERSON DESCRIBED IN THIS PARAGRAPH.
3. DRAWINGS ARE NOT TO BE SCALED, USE DIMENSIONS ONLY.
4. ALL CONTRACTOR SHALL VISIT THE JOB SITE TO BECOME FAMILIAR WITH ALL CONDITIONS. THE CONTRACTOR SHALL CHECK ALL DIMENSIONS AND CONDITIONS AT JOB SITE PRIOR TO COMMENCEMENT OF WORK AND IMMEDIATELY REPORT ANY DISCREPANCIES BETWEEN DESIGN AND EXISTING CONDITIONS TO THE ARCHITECT.
5. GENERAL CONTRACTOR AND ALL SUB CONTRACTORS MUST REVIEW ENTIRE SET OF DRAWINGS AND SPECIFICATIONS.
6. THE GENERAL CONTRACTOR SHALL MAINTAIN A COMPLETE SET OF "APPROVED FOR CONSTRUCTION" ARCHITECTURAL AND ENGINEERING DRAWINGS ON SITE FOR ALL TRADES TO REFER TO.
7. GENERAL CONTRACTOR SHALL SUBMIT TO EACH BIDDING SUBCONTRACTOR A COMPLETE SET OF ARCHITECTURAL AND ENGINEERING DRAWINGS. GENERAL CONTRACTOR SHALL CONTACT ARCHITECT IMMEDIATELY IF THERE ARE DISCREPANCIES IN THE DRAWINGS WHICH WILL AFFECT THE INDIVIDUAL BIDS.
8. THE CONTRACTOR SHALL VERIFY SIZE AND LOCATION OF ALL WALL AND ROOF OPENINGS FOR DUCTS, PIPES, ETC. WITH HVAC, ELECTRICAL, PLUMBING AND/OR ANY AFFECTED CONTRACTOR.
9. THE CONTRACTOR SHALL PROVIDE ALL SHORING, FALSEWORK, BRACING, CONSTRUCTION MATERIAL OR EQUIPMENT REQUIRED TO PROTECT PERSONS, PROPERTY AND ALL STRUCTURES NEW OR EXISTING DURING ALL PHASES OF THE WORK.
10. THE CONTRACTOR IS RESPONSIBLE FOR KEEPING THE JOB SITE CLEAN OF ALL DEBRIS DURING CONSTRUCTION AND PROVIDE A FINAL CLEAN UP BEFORE AND AFTER ARCHITECT'S AND OWNER'S FINAL REVIEW OF CONSTRUCTION.
11. ALL CONTRACTORS AND SUBCONTRACTORS SHALL BE RESPONSIBLE TO ACCEPT THE SURFACE, ETC. PREPARED BY OTHERS (AS APPLICABLE) PRIOR TO THE INSTALLATION AND START OF THEIR RESPECTIVE WORK. ALL 'UNACCEPTABLE' WORK SHALL BE REPORTED TO THE GENERAL CONTRACTOR FOR CORRECTION BY THE APPROPRIATE SUBCONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
12. THE CONTRACTOR IS RESPONSIBLE FOR ALL DEMOLITION, REMOVALS, CUTTING AND PATCHING TO ACCOMMODATE THE NEW WORK UNLESS SPECIFICALLY NOTED OTHERWISE.
13. ALL MATERIALS ARE TO BE PURCHASED AND INSTALLED BY THE CONTRACTOR UNLESS SPECIFICALLY NOTED OTHERWISE.
14. ALL WORK IS TO BE DONE IN STRICT ACCORDANCE WITH LOCAL LAWS, CODES AND FEDERAL SAFETY REGULATIONS.
15. CONTRACTOR WILL CONSTRUCT TEMPORARY FENCING IN EACH AREA OF WORK AS REQUIRED BY DEVELOPER OR THE LOCAL AUTHORITIES.
16. THE CONTRACTOR SHALL SECURE THE APPLICABLE PERMITS AND PAY ALL FEES, PRIOR TO THE START OF CONSTRUCTION AND RENEW THOSE PERMITS AS APPLICABLE FOR THE DURATION OF CONSTRUCTION; AND SHALL OBTAIN ALL CERTIFICATES OF OCCUPANCY.
17. ALL PLYWOOD TO BE FIRE-RETARDANT TREATED.
18. <u>ALL WOOD BLOCKING TO BE FIRE RETARDANT TREATED. ALL WOOD IN CONTACT W/ CONCRETE</u> OR MASONRY TO BE PRESSURE TREATED.
19. CONTRACTOR SHALL FURNISH AND INSTALL CORNER BEADS AT ALL EXPOSED ENDS IN DRYWALL CONSTRUCTION. TAPE, POLISH AND SPACKLE ALL WORK.
20. PROVIDE DOUBLE METAL STUDS, FULL HEIGHT ON BOTH SIDES OF NEW DOOR BUCK LOCATIONS.
21. ALL METAL STUDS MUST EXTEND FROM FLOOR TO UNDERSIDE OF BASE BUILDING CONSTRUCTION OR FLOOR SYSTEM ABOVE UNLESS SPECIFICALLY INDICATED OTHERWISE ON PARTITION SCHEDULE.
22. WHEN ALL FLOOR PARTITIONS HAVE BEEN SECURED, CONTRACTOR SHALL NOTIFY THE ARCHITECT FOR INSPECTION AND APPROVAL OF ENTIRE NEW PARTITION LAYOUT PRIOR TO PROCEEDING WITH THE WORK.
23. ALL DIMENSIONS ARE TO FINISHED SURFACE UNLESS NOTED OTHERWISE.
24. NO PIPING, CONDUIT, LIGHT FIXTURES, CEILING SUSPENSION SYSTEMS, WALLS, ETC. SHALL BE HUNG AND/OR SECURED TO ANY MECHANICAL DUCT.
25. NO PIPING, EQUIPMENTS, DUCTS, HANGERS, ETC. SHALL BE HUNG FROM METAL ROOF DECK.
26. THE FUTURE CEILING SUSPENSION SYSTEM SHALL BE SUSPENDED AND/OR SUPPORTED IN A MANNER WHICH MEETS THE REQUIREMENTS FOR SEISMIC RESTRAINT PER CURRENT APPLICABLE CODE.
27. THE GENERAL CONTRACTOR SHALL PROVIDE AND ARRANGE FOR ALL NECESSARY POWER, LIGHTING AND WATER AS MAY BE REQUIRED DURING CONSTRUCTION.
28 AT THE COMPLETION OF THE WORK THE CONTRACTOR SHALL ARRANGE FOR A FINAL CLEANING

28. AT THE COMPLETION OF THE WORK, THE CONTRACTOR SHALL ARRANGE FOR A FINAL CLEANING

OF THE ALL SPACES AND SURFACES INCLUDING GLASS AND METALS, FLOORS, HARDWARE, WALLS,

## GENERAL NOTES (CONTINUED)

29.	THE TERM "FURNISH" IS USED TO MEAN "SUPPL UNLOADING, UNPACKING, ASSEMBLY, INSTALLA
30.	THE TERM "INSTALL" IS TO BE USED TO DESCRI THE ACTUAL "UNLOADING, UNPACKING, ASSEM WORKING TO DIMENSION, FINISHING, CURING, F OPERATIONS".
31.	THE TERM "REGULATIONS" INCLUDES LAWS, OF ISSUED BY AUTHORITIES HAVING JURISDICTION AGREEMENTS WITHIN THE CONSTRUCTION IND WORK.
32.	ALL CEILING DEVICES INCLUDING LIGHT FIXTUR DETECTORS ARE TO CENTERED IN TILE, U.N.O INFORMATION ONLY. SEE M.E.P.F. DRAWINGS F DESIGNATIONS.
33.	STARTING POINT OF CEILINGS TO BE CENTER O
15.	THE CONTRACTOR SHALL PREPARE "COORDINA TRADES INVOLVED IN THE PROJECT HAVE COOR AS TO AVOID CONFLICTS AND/OR MISALIGNMEN BE SIMULTANEOUSLY REPRESENTED ON A SING WITHIN THE ALLOTTED SPACE. ALL CONFLICTS DRAWINGS FOR THE ARCHITECT'S (OR ENGINEE THE WORK WITHIN THE CEILING CAVITY.
16.	ALL SURFACES VISIBLE THROUGH MECHANICAL DIFFUSERS, ETC. SHALL BE PAINTED MATTE BLA
17.	ALL PIECES OF ARCHITECTURAL WOODWORK T SUPPLY SHALL BE SECURED TO SLAB OR PARTI
18.	PROVIDE ARCHITECTURAL SHEETMETAL, ACOU UNDERSIDE OF STRUCTURE WHEREVER PARTIT MULLIONS. PROVIDE ACOUSTICAL PILLOWS AR PENETRATIONS.
19.	PROVIDE 16 GAUGE (6" HIGH) WALL REINFORCE ALL WALL MOUNTED HANDRAILS, AND DOOR ST
20.	INSTALL ADEQUATE SOLID BLOCKING AND PART CABINETRY, ACCESSORIES AND EQUIPMENT.
21.	REFER TO MECHANICAL AND ELECTRICAL DRAV CONTRACT LIMIT LINE AND ON FLOOR BELOW.
22.	THE LOCATIONS OF ALL ELECTRICAL, FIRE PRO CEILING MOUNTED DEVICES, ACCESS DOORS A VERIFIED WITH THE ARCHITECT. REFER TO MEI DRAWINGS FOR ALL REQUIRED WALL AND FLOO
23.	USE DIELECTRIC SEPARATOR ON ALL DISSIMILA
24.	COORDINATE LOCATION OF OUTLET BOX FACES
25.	PROVIDE 14 GAUGE SHEETMETAL WALL REINFO PARTITIONS WITH WALL MOUNTED CABINETS.
26.	COORDINATE LOCATION OF OUTLET BOX FACES
27.	THE ARCHITECT HAS NOT BEEN RETAINED FOR NO RESPONSIBILITY FOR MEANS, METHODS AND
28.	SPECIFICATIONS INDICATE TYPES & COLORS OF SUBMITTED TO ARCHITECT FOR APPROVAL.
29.	WHERE THE FIREPROOFING OF EXISTING COLU BY NEW WORK, THE FIREPROOFING SHALL BE F RATING AS REQUIRED BY THE LOCAL BUILDING
30.	SEE MEP DRAWINGS FOR ALL NEW EQUIPMENT INSTALLATION.
31.	ANY MULTIPLE LAYER OF DRY WALL SHOULD BE
32.	PRIOR TO CONSTRUCTION, G.C. TO VERIFY ANY WITH ARCHITECT.
33.	FIRESTOP ALL VERTICAL PENETRATIONS THROUSE SPECIFICATIONS.
34.	FIRESTOP ALL VERTICAL PENETRATION THROUG LEAST EQUAL TO THAT OF THE PARTITION. SEE

CEILINGS, ETC.

### FEMA MAP

National Flood Hazard Layer FIRMette

#### **ZONING MAP**

D AND INDEX MAP FOR FIRM PANEL Without Base Flood Elevation (BFE)

With BFE or Depth Zone AE, AO, AH, VE, AR Regulatory Floodway

of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone* X

Future Conditions 1% Annual Chance Flood Hazard Zone X

Area with Reduced Flood Risk due to Levee. See Notes. Zone X Area with Flood Risk due to Levee Zone D

Effective LOMRs

CTURES IIIIIII Levee, Dike, or Floodwall

GENERAL ---- Channel, Culvert, or Storm Sewer

17.5 Water Surface Elevation

Limit of Study
Jurisdiction Boundary Coastal Transect Baselin Profile Baseline Hydrographic Feature Digital Data Available

e superseded by new data over time nis map image is void if the one or more of the follo ements do not appear: basemap imagery, flood zor gend, scale bar, map creation date, community ide RM panel number, and FIRM effective date. Map im

NO SCREEN Area of Minimal Flood Hazard Zone X

Coastal Transect
 Base Flood Elevation Line (BFE)

No Digital Data Available Unmapped

point selected by the user and does not represent an authoritative property location

Area of Undetermined Flood Hazard Zo



Y AND DELIVER TO THE PROJECT SITE, READY FOR TION, AND SIMILAR OPERATIONS. IBE OPERATIONS AT THE PROJECT SITE INCLUDING IBLY, ERECTION, PLACING, ANCHORING, APPLYING, PROTECTING, CLEANING, AND SIMILAR

RDINANCES, STATUTES, AND LAWFUL ORDERS N, AS WELL AS RULES, CONVENTIONS, AND OUSTRY THAT CONTROL PERFORMANCE OF THE

RES, SPEAKERS, SPRINKLERS, EXIT SIGNS, SMOKE . ALL DEVICES SHOWN ARE FOR LOCATION FOR QUANTITIES AND FIXTURE/DEVICE TYPE

OF ROOM UNLESS OTHERWISE NOTED. IATION DRAWINGS" THAT REPRESENT THAT ALL ORDINATED THE INSTALLATION OF THEIR WORK SO NTS. ALL WORK CONTAINED IN ALL CEILINGS SHALL IGLE DRAWINGS TO CONFIRM THAT ALL TRADES "FIT" S SHALL BE RESOLVED PRIOR TO SUBMITTING SHOP ER'S) REVIEW AND PRIOR TO EXECUTING ANY OF

L OR ARCHITECTURAL SLOTS, LOUVERS, GRILLES, ACK. THAT HAVE INTEGRATED ELECTRICAL OR PLUMBING TITIONS, AS REQUIRED.

JSTICALLY TREATED CLOSURES FROM SLAB TO TIONS MEET EXTERIOR CURTAINWALLS OR WINDOW ROUND ANY STRUCTURAL OR MECHANICAL

EMENT BACKER PLATES (FASTENED TO STUDS) AT TOPS ON GYPSUM WALLBOARD PARTITIONS. TITION REINFORCING FOR ALL MOUNTED

WINGS FOR SCOPE OF WORK BEYOND THE

DTECTION, PLUMBING AND MECHANICAL, WALL AND AND PANELS ARE TO BE COORDINATED AND EP DRAWINGS AS WELL AS THE ARCHITECTURAL OR PENETRATIONS. AR METALS.

ES WITH SCHEDULED APPLIED FINISH THICKNESS. ORCEMENT (FASTENED TO STUDS) AT ALL

ES WITH SCHEDULED APPLIED FINISH THICKNESS. R SUPERVISION OF CONSTRUCTION, AND ASSUMES ID TECHNIQUES OF CONSTRUCTION. OF FINISHES. FINAL SELECTION OF COLORS TO BE

UMNS, BEAMS, HANGARS, TUBES, ETC. IS DISTURBED RESTORED TO PROVIDE A MINIMUM RESISTANCE CODE.

. VERIFY LOCATIONS WITH ARCHITECT PRIOR TO

E APPLIED WITH STAGGERED JOINTS. Y/ ALL ADDITIONAL PARTITION BRACING REQUIRED

OUGH FLOOR SLAV WITH MIN. 2HR FIRE RATING.

IGH RATED PARTITIONS WITH MIN. FIRE RATING AT SPECIFICATIONS.

## CODE COMPLIANCE NOTES

#### GENERAL 1. THE GENERAL CONTRACTOR SHALL WORK WITH THE FIRE INSPECTOR TO SCHEDULE A FUNCTIONAL TEST OF EMERGENCY LIGHTING SYSTEM AND SHALL INSTALL ANY ADDITIONAL EXIT DIRECTIONAL LIGHTS AND EMERGENCY LIGHTS THAT THE FIRE INSPECTOR MAY REQUIRE 2. THE GENERAL CONTRACTOR SHALL WORK WITH THE FIRE INSPECTOR OFFICE TO DETERMINE FINAL LOCATIONS AND MOUNTING OF 10 LB ABC DRY CHEMICAL FIRE EXTINGUISHERS AS PER LOCAL CITY FIRE CODE 3. THE GENERAL CONTRACTOR SHALL WORK WITH THE FIRE INSPECTOR OFFICE AND PROVIDE ALL DOCUMENTS WHICH ARE REQUIRED TO BE COMPLETED BY THE ELECTRICAL CONTRACTOR INSTALLING THE ALARM INDICATING DEVICES AND NOTIFICATION APPLIANCES SHOWN ON THE DRAWINGS. FIRE-RESISTANCE-RATED CONSTRUCTION 4. FIRE WALLS, FLOORS, BARRIERS, PARTITIONS, AND SHAFT ENCLOSURES SHALL COMPLY WITH PROVISIONS OF LOCAL BUILDING CODE AND SHALL BE CONTINUOUS THROUGH ANY CONCEALED SPACE IN FLOOR OR ROOF CONSTRUCTION. 5. OPENINGS IN FIRE WALLS, FLOORS, BARRIERS, PARTITIONS, AND SHAFT ENCLOSURES TO COMPLY WITH LOCAL CODE. 6. PENETRATIONS IN FIRE WALLS, FLOORS, BARRIERS, PARTITIONS, AND SHAFT ENCLOSURES FOR PIPE SLEEVES, ELECTRIC DEVICES, ETC., SHALL BE PACKED SEALED WITH APPROVED MATERIALS TO MEET OR EXCEED RATED CONDITION AND SHALL COMPLY WITH LOCAL BUILDING CODE. 7. WHERE PIPES, WIRES, CONDUITS, DUCTS, ETC., PIERCE FIRE PROTECTION OF INDIVIDUALLY ENCASED STRUCTURAL MEMBERS, SUCH PENETRATION SHALL NOT EXCEED 2 PERCENT OF ANY ONE FACE SUCH PROTECTION AND SHALL BE CLOSED OFF WITH CLOSE FITTING METAL ESCUTCHEONS OR PLATES. 8. FIRE BLOCKING: CONCEALED SPACES WITHIN PARTITIONS, WALLS, FLOORS, ROOFS, STAIRS, FURRING PIPE SPACES, COLUMN ENCLOSURES, ETC., SHALL BE FIRE-BLOCKED AND DRAFT-STOPPED IN COMPLIANCE WITH THE LOCAL CODE (EXCEPT WHERE CONCEALED SPACE IS SPRINKLERED OR IS CONSTRUCTED AS A SHAFT). 9. CONDUITS IN FIRE-RATED PARTITIONS SHALL NOT EXCEED 3/4 INCH DIAMETER. OUTLETS IN SUCH PARTITIONS WILL BE BACKED UP WITH APPROVED MATERIALS. 10. SMOKE/FIRE DAMPERS OR DOORS SHALL BE PROVIDED WHERE DUCTS PENETRATE FIRE RATED WALLS OR CEILING. **INTERIOR FINISHES** 11. ALL INTERIOR FINISHES SHALL COMPLY WITH THE LOCAL BUILDING CODE, CHAPTER 8 AND CERTIFICATES WILL BE PROVIDED TO THE FIRE INSPECTOR OFFICE AS REQUIRED. 12. ALL FRAMING MATERIALS SHALL BE OF NON COMBUSTIBLE OR FIRE RETARDANT MATERIALS. 13. CEILING TILES TO BE MINERAL FIBER WITH UL LABEL AND FLAME SPREAD RATING OF 25 OF LESS. FIRE PROTECTION EQUIPMENT 14. PROVIDE A PORTABLE FIRE EXTINGUISHER WITH A RATING OF NOT LESS THAN 2-A-A10BC WITHIN 75 FEET TRAVEL DISTANCE TO ALL PORTIONS OF THE BUILDING ON EACH FLOOR AND DURING CONSTRUCTION. VERIFY WITH CITY ON DISTANCE FOR FIRE EXTINGUISHERS. 15. PROVIDE FIRE EXTINGUISHERS AS REQUIRED BY THE FIRE DEPARTMENT FIELD INSPECTOR. 16. REFER TO FIRE SYSTEMS DRAWINGS FOR TYPICAL MOUNTING HEIGHTS OF FIRE PROTECTION EQUIPMENT. 17. FIRE EXTINGUISHERS SHALL BE TYPE APPROVED BY UL, FM, FIRE MARSHAL'S OFFICE AND LOCAL REGULATORY AGENCIES. 18. FIRE EXTINGUISHERS SHOULD BE FABRICATED AND LABELED TO COMPLY WITH NEPA-10. 19. MATERIALS: 19.A. EXTINGUISHERS: MULTIPURPOSE, DRY CHEMICAL TYPE WITH A 5 LB NOMINAL CAPACITY AND A MINIMUM UL RATING OF 2A-10BC. 19.B. EXTINGUISHER BODIES AND VALVE ASSEMBLIES SHALL BE OF ALL METAL CONSTRUCTION. PLASTIC COMPONENTS ARE NOT ACCEPTABLE. 19.C. PROVIDE MANUFACTURER'S STANDARD BRACKET DESIGNED TO PREVENT ACCIDENTAL DISLODGEMENT OF EXTINGUISHER OR PROPER SIZE FOR TYPE AND CAPACITY OF EXTINGUISHER INDICATED, IN MANUFACTURER'S STANDARD PLATED FINISH. 20. INSTALLATION: 20.A. INSTALL EXTINGUISHERS IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. 20.B. SECURELY FASTEN MOUNTING BRACKETS TO STRUCTURE, SQUARE AND PLUMB. 20.C. LOCATE BOTTOM OF EXTINGUISHERS 36" ABOVE FINISHED FLOORS. 1. EACH EXTINGUISHER SHALL BE FULL CHARGED AND OTHERWISE INSPECTED JUST PRIOR TO TURNOVER AS EVIDENCE BY THE NATIONAL ASSOCIATION OF FIRE EQUIPMENT DISTRIBUTORS CERTIFICATION TAG CERTIFYING SUCH SERVICE HAS BEEN PERFORMED. FIRE ALARM 22. PROVIDE A FIRE/LIFE SAFETY SYSTEM AS REQUIRED BY CODE. 23. SUBMIT COMPLETE PLANS IN TRIPLICATE (3) TO THE DEPARTMENT OF BUILDING FOR THE ABOVE REQUIREMENT, ONE TO BE DESIGNATED AS THE FIRE DEPARTMENT COPY. MEANS OF EGRESS

24. REFER TO FOR DOOR SCHEDULE.

25. EVERY EXIT DOOR SHALL BE OPENABLE FROM THE INSIDE WITHOUT THE USE OF A KEY, TOOL, SPECIAL KNOWLEDGE OR EFFORT. SPECIAL LOCKING DEVICES SHALL BE AN APPROVED TYPE. 26. PANIC HARDWARE SHALL BE PROVIDED IN EXIT DOORS SERVING ROOMS, CORRIDORS OR STAIRWAYS HANDLING AN OCCUPANCY OF 50 OR MORE PERSONS.

27. ALL EXITS SHALL BE KEPT READILY ACCESSIBLE AND UNOBSTRUCTED MIN 44" CLEAR AT ALL TIMES. AISLES LEADING TO REQUIRED EXITS SHALL HAVE A MINIMUM WIDTH OF 44" CLEAR AND MUST BE PERMANENTLY IDENTIFIED AND MARKED ON FLOOR THROUGH STORAGE AREA.



## **DRAWING LIST**

244 PERIME	TER CENTER. DUNWOODY, GA				
			Issue	Date	
	Schematic Set Drawing List	12.18.2020 SD			
SHEET	TITLE				
RCHITECT	JRAL	· · ·	,		
G-001	COVER SHEET	•			
G-002	DRAWING LIST AND GENERAL NOTES	•			
G-003	ABBREVIATIONS AND SYMBOLS	•			
			_		+
		•			+
A-001		•			+
A-002		•			+
A-003	ADA NOTES & MOUNTING HEIGHTS				
A-100	FLOOR PLAN - LEVEL CELLAR	•			
A-101	FLOOR PLAN - LEVEL 01	•			
A-102	FLOOR PLAN - LEVEL 02	•			
A-103	FLOOR PLAN - LEVEL 03	•			
A-104	FLOOR PLAN - LEVEL 04	•			
A-105	FLOOR PLAN - LEVEL 05	•			
A-106	FLOOR PLAN - ROOF	•			
		•			
A-200		•			+
A-201	EAST & NORTH BUILDING ELEVATIONS	•			+
A-202		•			-
A-203		•			+
A-204	OVERALL BUILDING SECTION				-
A-300	WALL SECTIONS	•			
A-301	WALL SECTIONS	•			
A-310	ENLARGED PLAN DETAILS	•			
A-320	ENLARGED SECTION DETAILS	•			
A-321	ENLARGED SECTION DETAILS	•			

#### CODE COMPLIANCE NOTES (CONTINUED)

#### SIGNS AND EMERGENCY LIGHTING

28. LOCATION OF EVERY EXIT SHALL BE CLEARLY INDICATED BY EXIT SIGN PLACED, IF REQUIRED, AT AN ANGLE WITH THE EXIT OPENING. INSTALL DIRECTIONAL SIGNS TO SERVE AS GUIDE FROM ALL PORTIONS OF THE CORRIDOR OR FLOOR.

29. EXIT SIGNS SHALL BE INTERNALLY LIGHTED, HAVING AN INITIAL BRIGHTNESS OF THE LETTERS OF AT LEAST 25 FOOTLAMBERTS. LETTERS SHALL BE RED, THE BACKGROUND SHALL BE CLEAR OR WHITE. LETTERS SHALL BE BLOCK LETTERING, AT LEAST 8" HIGH WITH 3/4" STROKES. EXIT LIGHTING SHALL BE ON CIRCUITS THAT ARE SEPARATE FROM ANY OTHER CIRCUITS, TAKEN OFF AHEAD OF THE MAIN SWITCH.

30. EXIT DOORS SHALL BE READILY OPERABLE AT ALL TIMES FROM THE SIDE WHICH EGRESS IS TO BE MADE. DOORS MAY BE LOCKED TO PREVENT ACCESS FROM THE OUTDOORS AT STREET LEVEL. 31. PROVIDE EXIT SIGNS AND DIRECTION EXIT SIGNS WITH A MINIMUM 6" HIGH BY 3/4" STROKE BLOCK. LETTERS ON A CONTRASTING BACKGROUND.

32. WHENEVER SPACE IS OCCUPIED, EXIT SIGNS SHALL BE LIGHTED SO THEY ARE CLEARLY VISIBLE. 33. EXIT SIGNS SHALL BE ELECTRICALLY ILLUMINATED. TWO LAMPS SHALL BE ENERGIZED FROM SEPARATE CIRCUITS. ILLUMINATION SHALL NORMALLY BE PROVIDED BY THE PREMISES WIRING SYSTEM, IN THE EVENT OF FAILURE OF THIS SYSTEM, ILLUMINATION SHALL BE AUTOMATICALLY

PROVIDED FROM AN EMERGENCY SYSTEM. EMERGENCY LIGHTING SHALL GIVE A VALUE OF ONE FOOT CANDLE AT FLOOR LEVEL. VERIFY IF ILLUMINATED SIGN ARE TO HAVE BATTERY BACKUP. 34. EXIT PATH LIGHTING SHALL BE PROVIDED FOR ANY STAIRWAY, HALLWAY, EXIT PASSAGEWAY OR EGRESS TO A PUBLIC WAY ANYTIME THE BUILDING IS OCCUPIED. EMERGENCY LIGHTING SHALL BE PROVIDED GIVING LIGHT VALUE OF ONE FOOT CANDLE AT THE FLOOR LEVEL.

FIRE DEPARTMENT NOTES

35. CONTRACTOR SHALL SUBMIT SPRINKLER PLANS AND OBTAIN APPROVAL PRIOR TO INSTALLATION. 36. CONTRACTOR SHALL SUBMIT FIRE ALARM PLANS AND OBTAIN APPROVAL PRIOR TO INSTALLATION. ACCESSIBILITY

37. BUILDINGS AND FACILITIES SHALL BE DESIGNED AND CONSTRUCTED TO BE ACCESSIBLE IN ACCORDANCE WITH THE RULES AND REGULATIONS OF LOCAL BUILDING CODE AND ICC/ANSI A117.1-2009 ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES, 2010 ADA STANDARDS FOR ACCESSIBLE DESIGN.

SAFETY DURING CONSTRUCTION

38. ALL CONSTRUCTION SHALL CONFORM TO THE RULES AND REGULATIONS OF THE LOCAL BUILDING CODE.

## SPRINKLER NOTES

1. THE ENTIRE BUILDING SHALL BE SPRINKLERED IN ACCORDANCE WITH ALL STATE AND LOCAL CODES HAVING JURISDICTION, THE LATEST N.F.P.A. REQUIREMENTS AND THE REQUIREMENTS OF THE OWNER'S RATING ORGANIZATION.

2. SPRINKLER CONTRACTOR TO SUBMIT COMPLETE SPRINKLER DESIGN, HYDRAULIC CALCULATIONS, SHOP DRAWINGS, SIGNED AND SEALED BY A NJ PROFESSIONAL ENGINEER FOR

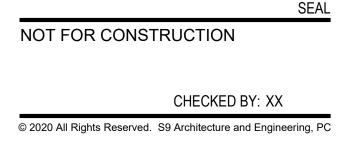
3. BUILDING SHALL BE DESIGNED AS NON-SEPARATED USES AND THEREFORE REQUIRED TO BE SPRINKLERED TO THE HIGHEST SPRINKLER HAZARD FOR THAT DESIGNATION, UNLESS OTHERWISE NOTED. REFER TO FIRE PROTECTION DRAWINGS FOR FURTHER INFORMATION. 4.SEE FP SERIES FOR SPRINKLER HEAD TYPES, LOCATIONS TO BE COORDINATED WITH ARCH. ID

### **DATUM NOTES**

ALL ELEVATION NOTED IN THE CONSTRUCTION DOCUMENTS CONTAINING AN ASTERISK " \* " INDICATE ELEVATION REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM IF 1988 (NAVD 88)

### FLOOD ZONE NOTES

THE PROPERTY IS NOT LOCATED IN SPECIAL FLOOD HAZARD AREA AS PER FIRM PANEL 13089C0012K DATED 08/15/2019



& CODE COMPLIANCE PAGE: 2 OF 23 SCALE: N/A G-002.00DOB BSCAN STICKER

244 PERIMETER CENTER PKWY NE, DUNWOODY, GA 30346 **PROJECT NO:** 66380.00 DRAWING TITLE: **DRAWING LIST** 

PROJECT TITLE: 244 PERIMETER CENTER

Abbotsford BC V2T 6B1 Canada, +1-604-940-8889 MEP Engineer: **CIVIL ENGINEER** 6991 Peachtree Industrial Blvd Building 700 Peachtree Corners GA 30092 404-330-9798

Atlanta GA 30305 404-424-9608 Architect/Interior Designer: S9 ARCHITECTURE 322 8th Avenue New York NY 10001

212-457-4077

Structural Engineer:

Atlanta GA 30309

1230 Peachtree Street

STRUCTURECRAFT

UZUN+CASE

678-553-5200

1919 Foy St

500 Park Avenue New York NY 1002 212-755-2323 **ROCAPOINT PARTNERS** 

309 East Paces Ferry Road Suite 825

NEW YORK, NY 10001 T 212.457.4077 S9ARCHITECTURE.COM THE GEORGETOWN COMPAN

**S9**ARCHITECTURE

NO. DATE ISSUE

NO. DATE

01 12.18.2020 SCHEMATIC DESIGN SET

KEY PLAN

322 8TH AVENUE

### MATERIALS (U.O.N.)

ALUMINUM	BATT INSULATION
CONCRETE	PRECAST
GLASS	GRAVEL
RIGID INSULATION	TERRAZZO
BRICK IN SECTION	WOOD - SOLID
CONCRETE MASONRY UNIT	PLYWOOD
EARTH	BRICK
GYPSUM / PLASTER	CULTURED STONE
STEEL	EXTERIOR COMPOSITE PANEL
PEA GRAVEL	WOOD BLOCKING
STONE	WOOD SHIM

#### SYMBOLS LEGEND

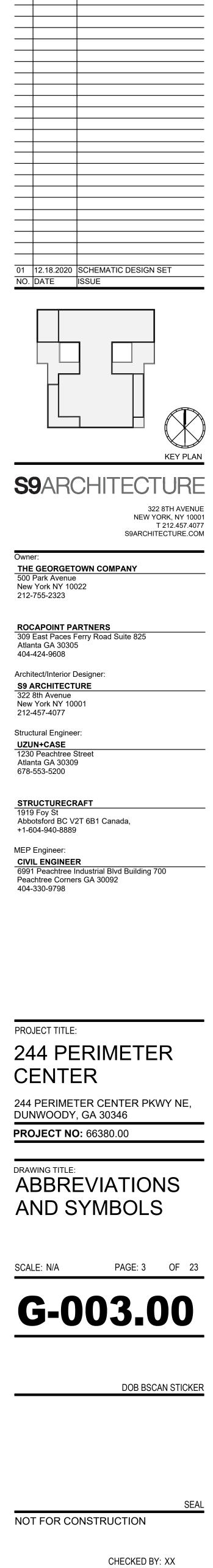
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<u>A 101</u>	SHEET No. — Sheet No. Sequence — Sheet Type	DETAIL SHEET	EXTERIOR ELEVATION
	Discipline NORTH ARROW	DETAIL SHEET 3	INTERIOR ELEVATION
		DETAIL SHEET	SECTION
0	GRID LINE	DETAIL SHEET	DETAIL
Room Name 0-000	ROOM No.	•	SPOT ELEVATION MARKERS
0.00	DOOR No.	0'-0" A.F.F.	CEILING TAG
Â)	WALL TYPE	$\sqrt{1}$	REVISION No.
(00.00)	SPECIALTY EQUIPMENT / FURNITURE TAG	$\bigcirc$	REVISION CLOUD
		Ę	CENTER LINE
1	SCOPE NOTE		
	FLOW ARROW		
	WINDOW TYPE		

## ABBREVIATIONS

@	AT	CW	CLOCKWISE
A/C	AIR CONDITIONER / CONDITIONING	CW	COLD WATER PIPING
ACC	ACCESSIBLE	•	
ACCSY	ACCESSORY	D	DEEP
ACOUS INSUL		DBL	DOUBLE
ACOUS PNL		DEG	DEGREE
ACP		DEMO	DEMOLITION
	CEILING PADS	DET	DETAIL
ACS DR	ACCESS DOOR	DF	DRINKING FOUNTAIN
ACS FLR	ACCESS FLOOR	DH	DOUBLE HUNG
ACS PNL	ACCESS PANEL	DIA	DIAMETER
ACST	ACOUSTICAL	DIAG	DIAGONAL
ACT	ACOUSTICAL CEILING TILE	DIM	DIMENSION
AD	AREA DRAIN	DISP	DISPENSER
ADA	AMERICANS WITH DISABILITIES ACT	DIV	DIVISION
ADDL	ADDITIONAL	DMPF	DAMPPROOFING
ADDM	ADDENDUM	DR	DOOR / DRAIN
ADJ	ADJUSTABLE / ADJACENT	DS	DOWNSPOUT
ADMIN	ADMINISTRATION	DW	DISHWASHER
AFC	ABOVE FINISHED COUNTER	DWG	DRAWING
AFF	ABOVE FINISHED FLOOR	DWR	DRAWER
AFG	ABOVE FINISHED GRADE		
AFS	ABOVE FINISHED SLAB	E	EAST
AHR	ANCHOR	EA	EACH
AHU	AIR HANDLING UNIT	EF	EACH FACE
ALNMT	ALIGNMENT	EG	VERTICAL END GUARDS
ALT	ALTERNATE	EHD	ELECTRIC HAND DRYER
ALUM	ALUMINUM	EIFS	EXTERIOR INSULATION AND FINISH
ANOD	ANODIZED		SYSTEM
APC	ACOUSTICAL PANEL CEILING	EJ	EXPANSION JOINT
APPROX	APPROXIMATELY	EL	ELEVATION
APPVD	APPROVED	ELEC	ELECTRICAL
APT	APARTMENT	ELEV	ELEVATOR
		EM	ELECTROMAGNETIC
		EM	ELECTROMAGNETIC EXPANDED METAL
ASPH	ASPHALT		EMERGENCY
ASTM	AMERICAN SOCIETY FOR TESTING	EMER ENCL	EMERGENCY ENCLOSED / ENCLOSURE
	MATERIALS		
ATC	ACOUSTICAL TILE CEILING	ENGR	ENGINEER
AUTO	AUTOMATIC	ENTR	
AVG	AVERAGE	EOS	EDGE OF SLAB
AWP	WALL-MOUNTED SOUND ABSORPTION PANEL	EPB	ELECTRIC PANEL BOARD
		EQ	EQUAL(LY)
AWT	ACOUSTIC WALL TREATMENT	EQUIP	EQUIPMENT
		ER	POURED EPOXY
BD	BOARD	ERF	POURED EPOXY FLOORING
BITUM	BITUMINOUS	ESCAL	ESCALATOR
BL	BASELINE	EST	ESTIMATE
BL	BLACK	EW	EACH WAY
BLDG	BUILDING	EWC	ELECTRIC WATER COOLER
BM	BEAM	EXF	EXTERIOR FINISH
BO	BLOWOFF	EXH	EXHAUST
вот	BOTTOM	EXIST	EXISTING
BR	BEDROOM	EXP	EXPANSION
BRG	BEARING	EXT	EXTERIOR
BS	BOTH SIDES	EAT	EXTENSIO
BSMT	BASEMENT	FA	FIRE ALARM
BTWN	BETWEEN	FAB	FABRIC
BUR	BUILT UP ROOFING	FAI	FRESH AIR INTAKE
BW	BOTH WAYS	FBRK	FIREBRICK
		FCP	PREFABRICATED FABRIC CEILING PANEL
С	CELSIUS	FCU	FAN COIL UNIT
C	CELSIUS CHANNEL	FD	FLOOR DRAIN
C TO C	*******	FDTN	FOUNDATION
CAB	CENTER TO CENTER CABINET	FE	FIRE EXTINGUISHER
CAB	-	FEC	FIRE EXTINGUISHER CABINET
		FF EL	FINISH FLOOR ELEVATION
CEM		FGL	FIBERGLASS
CEM PLAS	CEMENT PLASTER (STUCCO)	FH	FIRE HOSE
CER		FHC	FIRE HOSE CABINET
CG		FIN	FINISH(ED)
CH	CHILLER	FIN FLR	FINISHED FLOOR
CH	COAT HOOK	FIN FLR	FINISH FLOOR
CHBD	CHALKBOARD		
CHFR	CHAMFER	FIXT	FIXTURE
CI	CASTIRON	FLASH	FLASH(ING)
CIP	CAST IN PLACE	FLR	FLOOR
CJ	CONTROL JOINT	FLRSK	FLOOR SINK
CL	CENTERLINE	FLUOR	FLUORESCENT
CL D	CLOTHES DRYER	FM	FACTORY MUTUAL
CLG	CEILING	FOC	FACE OF CONCRETE
CLG HT	CEILING HEIGHT	FOF	FACE OF FINISH
CLL	CONTRACT LIMIT LINE	FOM	FACE OF MASONRY
CLO	CLOSET	FOS	FACE OF STUDS
CLR	CLEAR	FOW	FACE OF WALL
CMU	CONCRETE MASONRY UNIT	FP	FIREPROOF
CNTR	COUNTER	FPL	FIREPLACE
CO	CARBON MONOXIDE	FR	FRAME
CO	CASED OPENING	FRG	FIBER REINFORCED GYPSUM
CO	CERTIFICATE OF OCCUPANCY	FRP	FIBERGLASS REINFORCED PLASTIC
CO	CLEANOUT	FRT	FIRE RETARDANT TREATED
CO	COMPANY	FSP	FIRE STAND PIPE
CO	CUTOUT	FSTNR	FASTENER
COL	COLUMN	FTG	FOOTING
COMB	COMBINATION	FURG	FURRING
CONC	CONCRETE	FUT	FUTURE
CONC CONC FLR	SEALED CONCRETE FLOOR	FWC	FABRIC WALL COVER
CONSTR	CONSTRUCTION	FWP	NON-ACOUSTIC STRETCHED FABRIC
CONSTR	CONTINUE		PANEL WALL INSTALATION
CONT	CONTROLLER		
CONT	CONVERT	G	GROUND
CORR	CORVERT CORRIDOR / CORRUGATED	GA	GAGE
CORR	CORRIGATED	GALV	GALVANIZE(D)
		GB	GRAB BAR
			GENERAL CONTRACTOR
CR	CLOSET ROD	GC	
CR CR	CONTROL RELAY	GC GDISP	GARBAGE DISPOSAL
CR CR CR	CONTROL RELAY CONTROL ROOM		GARBAGE DISPOSAL GENERAL
CR CR CR CR	CONTROL RELAY CONTROL ROOM CRASH RAIL	gdisp gen	
CR CR CR CR CS	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE	gdisp gen gfp	GENERAL GFRP OR GFRG FREESTANDING PANEL
CR CR CR CR CS CS	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH	gdisp gen gfp gfrg	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM
CR CR CR CR CS CS CSWK	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK	GDISP GEN GFP GFRG GFRP	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER
CR CR CR CR CS CS CSWK CT	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE	GDISP GEN GFP GFRG GFRP GL	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING
CR CR CR CS CS CSWK CT CTB	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE	gdisp gen gfp gfrg gfrp gl gl blk	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK
CR CR CR CS CS CSWK CT CTB CTR	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER	gdisp gen gfp gfrg gfrp gl gl gl blk glt	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE
CR CR CR CS CS CSWK CT CTB CTR CU	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE	GDISP GEN GFP GFRG GL GL BLK GLT GLZ	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING
CR CR CR CS CS CSWK CT CTB CTB CTR CU CU FT	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT	GDISP GEN GFP GFRG GFRP GL GL BLK GLT GLZ CMU	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT
CR CR CR CS CS CSWK CT CTB CTB CTR CU CU FT CU YD	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER	GDISP GEN GFP GFRG GFRP GL GL BLK GLT GLZ GLZ CMU GPM	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE
CR CR CR CS CS CSWK CT CTB CTB CTR CU CU FT CU YD CUB	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT	GDISP GEN GFP GFRG GL GL BLK GLT GLZ GLZ CMU GPM GR	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE GROSS
CR CR CR CS CS CSWK CT CTB CTB CTR CU CU FT CU YD	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT CUBIC YARD	GDISP GEN GFP GFRG GL BLK GL BLK GLZ GLZ CMU GPM GR GR LN	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE GROSS GRADE LINE
CR CR CR CS CS CSWK CT CTB CTB CTR CU CU FT CU YD CUB	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT CUBIC YARD CUBICLE CURTAIN	GDISP GEN GFP GFRG GL GL BLK GLT GLZ GLZ CMU GPM GR	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE GROSS
CR CR CR CS CS CSWK CT CTB CTR CTR CU CU FT CU YD CUB CURT	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT CUBIC FOOT CUBIC YARD CUBICLE CURTAIN CURTAIN	GDISP GEN GFP GFRG GL BLK GL BLK GLZ GLZ CMU GPM GR GR LN	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE GROSS GRADE LINE
CR CR CR CS CS CSWK CT CTB CTR CTR CU CU FT CU YD CUB CURT CW	CONTROL RELAY CONTROL ROOM CRASH RAIL CAST STONE CONTROL SWITCH CASEWORK CERAMIC TILE CERAMIC TILE BASE CENTER COPPER CUBIC FOOT CUBIC FOOT CUBIC YARD CUBICLE CURTAIN CURTAIN CASEMENT WINDOW	GDISP GEN GFP GFRG GL BLK GL BLK GLZ GLZ CMU GPM GR GR LN	GENERAL GFRP OR GFRG FREESTANDING PANEL GLASS FIBER REINFORCED GYPSUM GLASS FIBER REINFORCED POLYMER GLASS / GLAZING GLASS BLOCK GLASS MOSIAC TILE GLAZING GLAZED CONCRETE MASONRY UNIT GALLONS PER MINUTE GROSS GRADE LINE

GT	GREASE TRAP	OPNG	OPENING	SST	STAINLESS STEEL
GT GT	GROSS TON GROUT	OPP OZ	OPPOSITE OUNCE	SSTR ST	STAINLESS STEEL RUB RAIL SINGLE THROW
GYP	GROUT GYPSUM	<u>5</u> 2		ST	SINGLETHROW
GYP BD	GYPSUM BOARD	PAR	PARALLEL	ST	STREET
		PAR	PARAPET	ST	STAIN
H HC	HIGH HOLLOW CORE	PART PAT	PARTIAL PATTERN	STA STD	STATION STANDARD
HCP	HANDICAPPED	PBD	PARTICLE BOARD	STIF	STANDARD
HD	HEAVY DUTY	PCC	PRECAST CONCRETE	STON	STONE
HD	HAND DRYER	PED	PEDESTAL	STOR	STORAGE
HDBD	HARDBOARD	PERF	PERFORATED	STRUCT	STRUCTURE / STRUCTURAL
HDR HDW	HEADER HARDWARE	PERIM PERP	PERIMETER PERPENDICULAR	SUH SURF	SUSPENDED UNIT HEATER SURFACE
HDWD	HARDWOOD	PL	PROPERTY LINE	SUSP	SUSPENDED
HDWL	HEADWALL	PLAM	PLASTIC LAMINATE	SVP	VINYL SOLID PANEL (ACROVYN)
HM	HOLLOW METAL	PLAS	PLASTIC	SW	SWITCH
HNDRL HORIZ	HANDRAIL HORIZONTAL	PLAS PLYWD	PLASTER PLYWOOD	SYMM SYNTH	SYMMETRICAL SYNTHETIC
HP	HORIZONTAL HEAT PUMP	PR	PAIR	SYS	SYSTEM
HP	HIGH PRESSURE	PREFAB	PREFABRICATED	010	
HP	HORSEPOWER	PREFIN	PRE-FINISHED	Т	TREAD
HR		PRF	POLYURETHANE SPORTS FLOORING	T&B	
HS HT	HAND SINK HEIGHT	PRM PROP	PROTECTED ROOF MEMBRANE PROPERTY	T&G T/S	TONGUE AND GROOVE TUB / SHOWER
HVAC	HEATING / VENTILATION / AIR	PRST	PRESSURE TREATED	TB	TOWEL BAR
	CONDITIONING	PS	POURED SEALANT	TC	TERRA COTTA
HW HWP	HOT WATER HOT WATER PUMP	PSF	POUNDS PER SQUARE FOOT	TCZ	CEMENTITIOUS TERRAZZO
HYD	HYDRANT	PSI PT	POUNDS PER SQUARE INCH PAINT(ED)	TEL TEMP	TELEPHONE TEMPERATURE
		PT/E	EPOXY PAINT	TEMP	TEMPORARY
IC		PT/F	FLAT PAINT	THK	THICKNESS
ID		PT/HG	HIGH GLOSS PAINT	THRES	THRESHOLD
IN INCL	INCH / INCHES INCLUDE(D) / INCLUDING	PT/SG	SEMIGLOSS PAINT	THRU	
INCL	INSULATE / INSULATION	PTD PTN	PAPER TOWEL DISPENSER PARTITION	TK BD TKP	TACK BOARD TACKABLE PANELING
INT	INTERIOR	PV	PAVED	TOC	TOP OF CONCRETE
INV		PVC	POLYVINYL CHLORIDE	TOS	TOP OF SLAB
ITS	INTERTEK TESTING SERVICES	PVG	PAVING	TOW	
J-BOX	JUNCTION BOX	0		TPD	TOILET PAPER DISPENSER
JAN CLO	JANITOR'S CLOSET	Q Q	HEAT TRANSFER RATE OF FLOW	TR TRTD	ACCENT RAIL TREATED
JS	JANITOR'S SINK	QT	QUARRY TILE	TRZ	INTERIOR POURED EPOXY RESIN TERRAZO
•		QTY	QUANTITY		
KIT	KITCHEN	QZF		TV TVP	
L	ANGLE	QZR QZT	QUARTZ RESIN QUARTZ TILE	TYP	TYPICAL
L	LITER	QZI	QUARTZ TILL	UC	UNDERCUT
LAB	LABORATORY	R	RISER	UGND	UNDERGROUND
LAM		R	RADIUS	UL	UNDERWRITER'S LABORATORY
LAV LB(S)	LAVATORY POUND(S)	RA		UNFIN UON	UNFINISHED UNLESS OTHERWISE NOTED
LB(S) LC	LINEN CLOSET	RB RB	RUBBER BASE RUBBER BASE	UR	UNLESS OTHERWISE NOTED
LF	LINEAR FOOT (FEET)	RCP	REFLECTED CEILING PLAN		
LH	LEFT HAND	RD	ROOF DRAIN	V	VOLT
LIB		REC	RECESSED	VAC	VACUUM VINVI BASE
lino Lkr	LINOLEUM LOCKER	RECPT REF	RECEPTACLE REFERENCE	VB VCT	VINYL BASE VINYL COMPOSITION TILE
LNO	LINOLEUM	REF	REFERENCE REFRIGERATOR	VENT	VINTE COMPOSITION THE
LPT	LOW POINT	REFL	REFLECTED	VERT	VERTICAL
LT		REG	REGISTER	VEST	VESTIBULE
LTF		REINF	REINFORCE(D)		
LTWT LVR	LIGHT WEIGHT LOUVER	REQ		VNR VOL	VENEER VOLUME
		RESIL REST	RESILIENT RESTROOM	VOL	VACUUM PUMP
m	METER	RET	RETURN	VP	VANISHING POINT
MACH		REV	REVISE / REVISION	VP	
MAT MATL	WALK-OFF MAT MATERIAL	RF	RESILIENT FLOORING	VP VPC	VENEER PLASTER VENEER PLASTIC CEILING/SOFFIT
MATL	MATERIAL MAXIMUM	RFG RH	ROOFING RIGHT HAND	VPC VR	VENEER PLASTIC CEILING/SOFFI
MB	MACHINE BOLT	RM	ROOM	VRR	VINYL (ACROVYN) RAIL
MB	MAIL BOX	RO	ROUGH OPENING	VS	VENT STACK
MB		ROW	RIGHT OF WAY	VS	
MBH MC	MOP/BROOM HOLDER MEDICINE CABINET	RR		VS VT	SHEET VINYL SOLID VINYL TILE
MCP	ACOUSTIC LAY-IN METAL CEILING PADS	RR RS	ROLL ROOFING RAPID START	VTR	VENT THRU ROOF
ME	MECHANICAL ENGINEER	RS	ROUGH SAWN	VTS	VINYL TRANSITION STRIP
MECH	MECHANICAL	RS	RUBBER SHEET	VWC	VINYL WALLCOVERING
MED MEZZ	MEDIUM MEZZANINE	RSF		W	WIDE
MEZZ	MEZZANINE MANUFACTURE(R)	RST RT	REINFORCING STEEL RIGHT	W	WEST
MH	MANHOLE	RTF	RUBBER TILE FLOORING	W / W	WALL TO WALL
MIN	MINIMUM	RTS	RUBBER TRANSITION STRIP	W/	
MIRR MISC	MIRROR MISCELLANEOUS	RWD		W/D W/O	WASHER / DRYER WITHOUT
MISC MKR BD	MISCELLANEOUS MARKER BOARD	RWL	RAIN WATER LEADER	W/O WB	WITHOUT WOOD BASE
MLDG	MOULDING	S	SOUTH	WC	WATER CLOSET
mm	MILLIMETER	SAN	SANITARY	WC	WALLCOVERING
MO MR	MASONRY OPENING	SC	SOLID CORE	WD WDH	
MR MS	MOISTURE RESISTANT MOP SINK	SCHED SCRN	SCHEDULE	WDH WDM	WOOD HAND RAIL WOOD WALL MOULDINGS
MSB	MOP SINK MOP SERVICE BASIN	SCRN SCT	SCREEN STRUCTURAL CLAY TILE	WDP	WOOD WALL MOOLDINGS
MTD	MOUNTED	SD	SOAP DISPENSER	WDR	WOOD CHAIR, CRASH, OR HAND RAIL
MTL	METAL	SECT	SECTION	WDS	ATHLETIC/ACOUSTIC ISOLATED WOOD FLOORING
MTS MULL	METAL TRANSITION STRIP MULLION	SF	SQUARE FEET	WDSHV	WOOD SHELVING
MULL MVBL	MOULION MOVABLE	SGL SH	SINGLE SINGLE HUNG	WDT	WOOD TILE OR PARQUET FLOORING
MW	MICROWAVE	SHR	SHOWER	WDV	WOOD VENEER
	NODTI	SHR HD	SHOWER HEAD	WDW WE	
N NA	NORTH NOT AVAILABLE / APPLICABLE	SHT	SHEATHING	WF WGL	WIDE FLANGE WIRED GLASS
NA NAT	NOT AVAILABLE / APPLICABLE NATURAL	SHTHG SIM	SHEATHING SIMILAR	WH	WALL HUNG
NFPA	NATIONAL FIRE PROTECTION	SIM	SIMILAR SEA LEVEL	WI	WROUGHT IRON
	ASSOCIATION	SL	SPOT LIGHT	WP	
NIC		SLDG	SLIDING	WP WPM	WORKING POINT WATERPROOF MEMBRANE
NO NOM		SND	SANITARY NAPKIN DISPENSER	WPM WR	WATERPROOF MEMBRANE WEATHER RESISTANT
NOM NTS	NOMINAL NOT TO SCALE	SNDU SPC	SANITARY NAPKIN DISPOSAL UNIT SUSPENDED PLASTER CEILING	WR	WIRE ROPE
		SPEC	SUSPENDED PLASTER CEILING SPECIFY / SPECIFICATION	WRP	WATER REPELLENT
OA	OVERALL	SPH	SOLID PHENOLIC POLYMER (TRESPA)	WT	
00	ON CENTER	SPKR	SPEAKER	WWF	WELDED WIRE FABRIC
OD OF	OUTSIDE DIAMETER OUTSIDE FACE	SPR	SOLID POLYMER RESIN (3FORM)	XPS	EXTRUDED POLYSTYRENE
OF OF/	OUTSIDE FACE OWNER FURNISHED / CONTRACTOR	SQ SQIN	SQUARE SQUARE INCH(ES)	2 ··· •	
CI	INSTALLED	SSP	SOLID SURFACE POLYMER (CORIAN)	YD	YARD
OFF	OFFICE	SSPR	SOLID POLYMER (CORIAN) RUB RAIL		
OPH	OPPOSITE HAND				



NO. DATE

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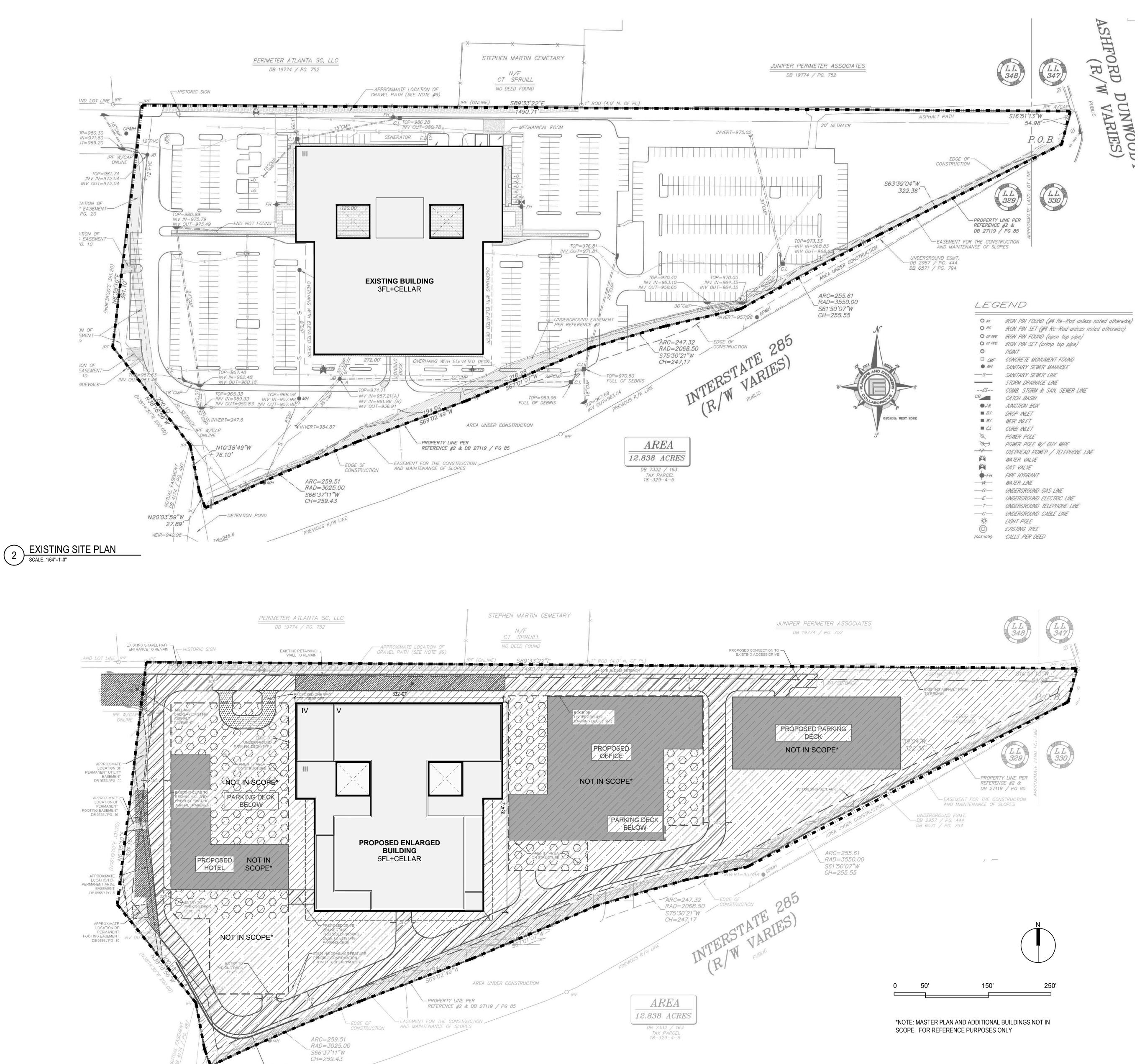
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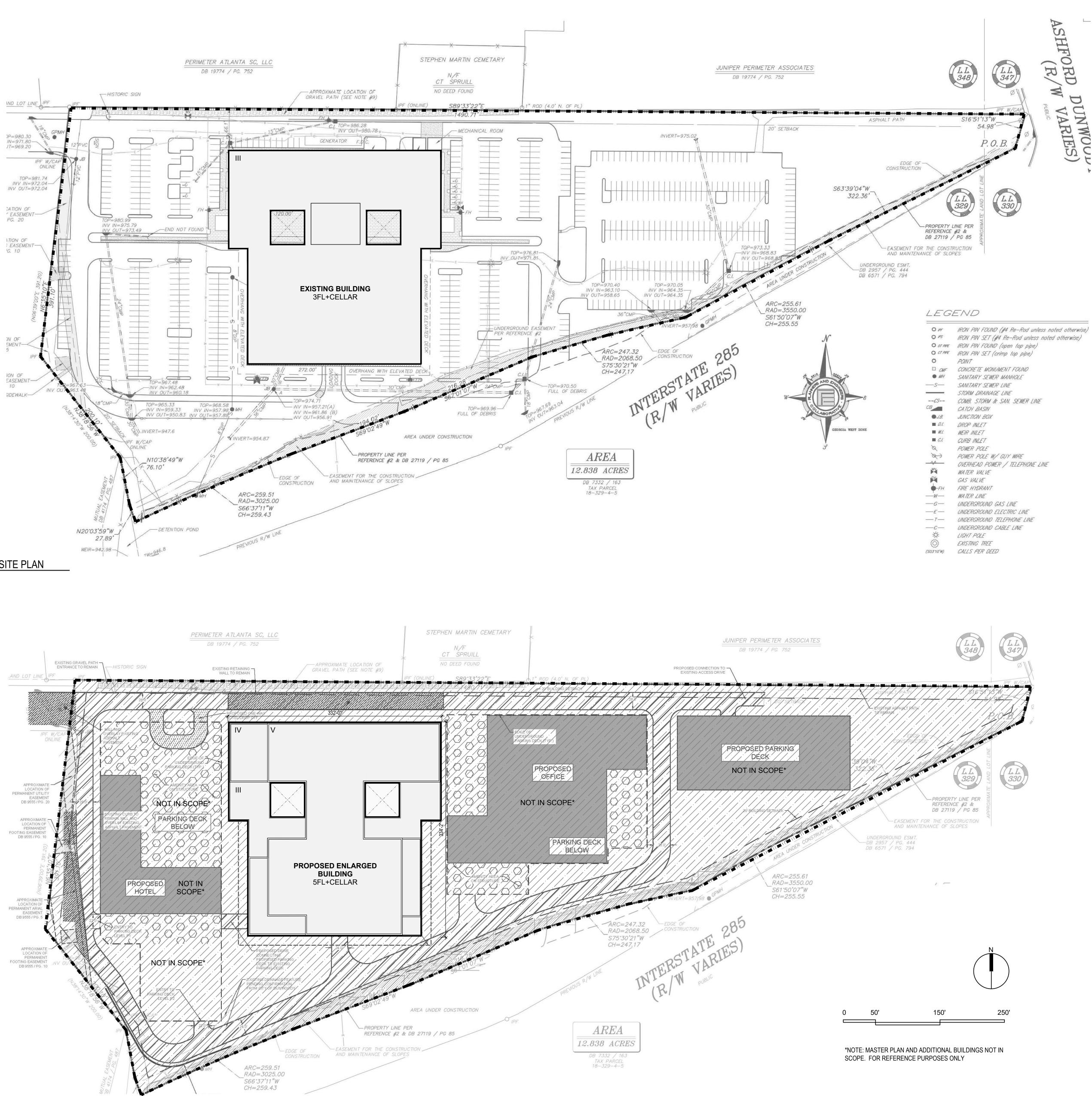
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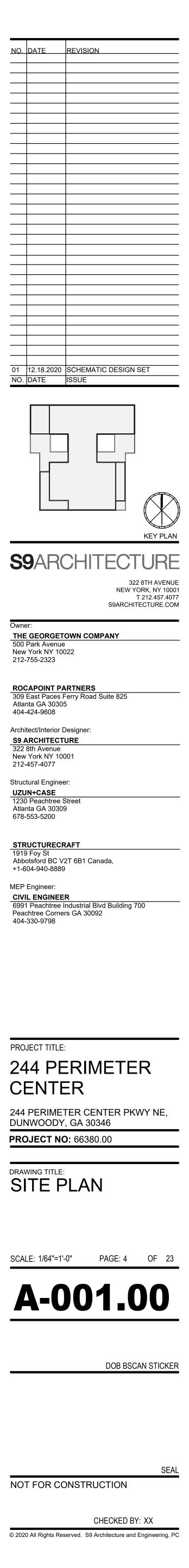
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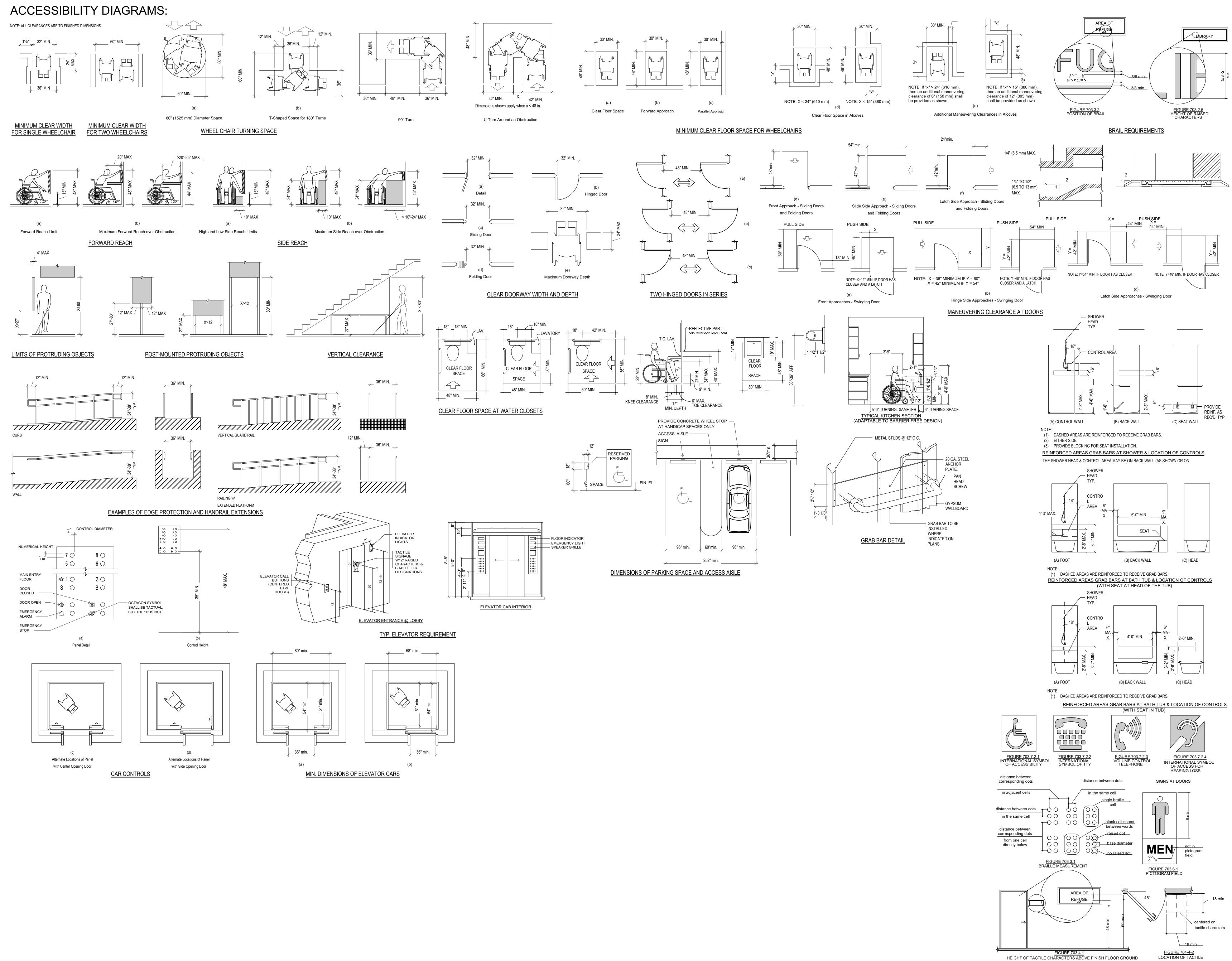




1 PROPOSED SITE PLAN SCALE: 1/64"=1'-0"

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#### ADA NOTES

1. ALL WORK SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF LOCAL BUILDING CODE.

2. ALL WORK SHALL COMPLY WITH THE APPLICABLE REQUIREMENTS OF 2010 ADA STANDARDS FOR ACCESSIBLE DESIGN. 3. THE MINIMUM CLEAR WIDTH FOR SINGLE WHEELCHAIR PASSAGE SHALL BE 32 IN

AT A POINT AND 36 IN CONTINUOUSLY. 4. THE MINIMUM WIDTH FOR TWO WHEELCHAIRS TO PASS IS 60 IN.

5. THE SPACE REQUIRED FOR A WHEELCHAIR TO MAKE A 180-DEGREE TURN IS A CLEAR SPACE OF 60 IN. DIAMETER.

6. THE MINIMUM CLEAR FLOOR OR GROUND SPACE REQUIRED TO ACCOMMODATE A SINGLE, STATIONARY WHEELCHAIR OCCUPANT IS 30 IN BY 48THE MINIMUM CLEAR FLOOR OR GROUND SPACE FOR WHEELCHAIRS MAY BE POSITIONED FOR FORWARD OR PARALLEL APPROACH TO AN OBJECT. CLEAR FLOOR OR GROUND SPACE FOR WHEELCHAIRS MAY BE PART OF THE KNEE SPACE REQUIRED UNDER SOME OBJECTS.

7. ONE FULL UNOBSTRUCTED SIDE OF THE CLEAR FLOOR OR GROUND SPACE FOR A WHEELCHAIR SHALL ADJOIN OR OVERLAP AN ACCESSIBLE ROUTE OR ADJOIN ANOTHER WHEELCHAIR CLEAR FLOOR SPACE. IF A CLEAR FLOOR SPACE IS LOCATED IN AN ALCOVE OR OTHERWISE CONFINED ON ALL OR PART OF THREE SIDES, ADDITIONAL MANEUVERING CLEARANCES SHALL BE PROVIDED.

8. IF THE CLEAR FLOOR SPACE ONLY ALLOWS FORWARD APPROACH TO AN OBJECT. THE MAXIMUM HIGH FORWARD REACH ALLOWED SHALL BE 48 IN. THE MINIMUM LOW FORWARD REACH IS 15 IN.

9. IF THE CLEAR FLOOR SPACE ALLOWS PARALLEL APPROACH BY A PERSON IN A WHEELCHAIR, THE MAXIMUM HIGH SIDE REACH ALLOWED SHALL BE 48 IN. AND THE LOW SIDE REACH SHALL BE NO LESS THAN 15 IN. ABOVE THE FLOOR. 10. AT LEAST ONE ACCESSIBLE ROUTE WITHIN THE BOUNDARY OF THE SITE

SHALL BE PROVIDED FROM PUBLIC TRANSPORTATION STOPS. ACCESSIBLE PARKING, AND ACCESSIBLE PASSENGER LOADING ZONES, AND PUBLIC STREETS OR SIDEWALKS TO THE ACCESSIBLE BUILDING ENTRANCE THEY SERVE. 11. AT LEAST ONE ACCESSIBLE ROUTE SHALL CONNECT ACCESSIBLE BUILDINGS, FACILITIES, ELEMENTS, AND SPACES THAT ARE ON THE SAME SITE.

12. AT LEAST ONE ACCESSIBLE ROUTE SHALL CONNECT ACCESSIBLE BUILDING OR FACILITY ENTRANCES WITH ALL ACCESSIBLE SPACES AND FLEMENTS AND WITH ALL ACCESSIBLE DWELLING UNITS WITHIN THE BUILDING OR FACILITY. 13. AN ACCESSIBLE ROUTE SHALL CONNECT AT LEAST ONE ACCESSIBLE ENTRANCE OF EACH ACCESSIBLE DWELLING UNIT WITH THOSE EXTERIOR AND INTERIOR SPACES AND FACILITIES THAT SERVE THE ACCESSIBLE DWELLING UNIT.

14. THE MINIMUM CLEAR WIDTH OF AN ACCESSIBLE ROUTE SHALL BE 36 IN. EXCEPT AT DOORS.

15. IF AN ACCESSIBLE ROUTE HAS LESS THAN 60 IN. CLEAR WIDTH, THEN PASSING SPACES AT LEAST 60 IN. BY 60 IN. SHALL BE LOCATED AT REASONABLE INTERVALS NOT TO EXCEED 200 FT. A T-INTERSECTION OF TWO CORRIDORS OR WALKS IS AN ACCEPTABLE PASSING PLACE.

16. ACCESSIBLE ROUTES SERVING ANY ACCESSIBLE SPACE OR ELEMENT SHALL ALSO SERVE AS A MEANS OF EGRESS FOR EMERGENCIES OR CONNECT TO AN ACCESSIBLE PLACE OF REFUGE. SUCH ACCESSIBLE ROUTES AND PLACES OF REFUGE SHALL COMPLY WITH THE REQUIREMENTS OF THE ADMINISTRATIVE AUTHORITY HAVING JURISDICTION. WHERE FIRE CODE PROVISIONS REQUIRE MORE THAN ONE MEANS OF EGRESS FROM ANY SPACE OR ROOM, THEN MORE THAN ONE ACCESSIBLE MEANS OF EGRESS SHALL ALSO BE PROVIDED FOR HANDICAPPED PEOPLE. ARRANGE EGRESS SO AS TO BE READILY ACCESSIBLE FROM ALL ACCESSIBLE ROOMS AND SPACES.

17. OBJECTS PROJECTING FROM WALLS (FOR EXAMPLE, TELEPHONES) WITH THEIR LEADING EDGES BETWEEN 27 IN. AND 80 IN. ABOVE THE FINISHED FLOOR SHALL PROTRUDE NO MORE THAN 4 IN. INTO WALKS, HALLS, CORRIDORS, PASSAGEWAYS, OR AISLES. OBJECTS MOUNTED WITH THEIR LEADING EDGES AT OR BELOW 27 IN. ABOVE THE FINISHED FLOOR MAY PROTRUDE ANY AMOUNT. FREE-STANDING OBJECTS MOUNTED ON POSTS OR PYLONS MAY OVERHANG 12 IN MAXIMUM FROM 27 IN. TO 80 IN. ABOVE THE GROUND OR FINISHED FLOOR. PROTRUDING OBJECTS SHALL NOT REDUCE THE CLEAR WIDTH OF AN ACCESSIBLE ROUTE OR MANEUVERING SPACE.

18. WALKS, HALLS, CORRIDORS, PASSAGEWAYS, AISLES, OR OTHER CIRCULATION SPACES SHALL HAVE 80 IN. MINIMUM CLEAR HEAD ROOM. IF VERTICAL CLEARANCE OF AN AREA ADJOINING AN ACCESSIBLE ROUTE IS REDUCED TO LESS THAN 80 IN. (NOMINAL DIMENSION), A BARRIER TO WARN BLIND OR VISUALLY-IMPAIRED PERSONS SHALL BE PROVIDED.

19. GROUND AND FLOOR SURFACES ALONG ACCESSIBLE ROUTES AND IN ACCESSIBLE ROOMS AND SPACES INCLUDING FLOORS, WALKS, RAMPS, STAIRS, AND CURB RAMPS SHALL BE STABLE, FIRM, AND SLIP-RESISTANT.

20. CHANGES IN LEVEL UP TO 1/4 IN MAY BE VERTICAL AND WITHOUT EDGE TREATMENT. CHANGES IN LEVEL BETWEEN 1/4 IN AND 1/2 IN SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1:2. CHANGES IN LEVEL GREATER THAN 1/2 IN SHALL BE ACCOMPLISHED BY MEANS OF A RAMP.

21. IF CARPET OR CARPET TILE IS USED ON A GROUND OR FLOOR SURFACE, THEN

#### IT SHALL BE SECURELY ATTACHED; HAVE A FIRM CUSHION, PAD, OR BACKING OR NO CUSHION OR PAD: AND HAVE A LEVEL LOOP. TEXTURED LOOP. LEVEL CUT PILE, OR LEVEL CUT/UNCUT PILE TEXTURE. THE MAXIMUM PILE HEIGHT SHALL BE 1/2 IN. EXPOSED EDGES OF CARPET SHALL BE FASTENED TO FLOOR SURFACES AND HAVE TRIM ALONG THE ENTIRE LENGTH OF THE EXPOSED EDGE. IF CARPET TILE IS USED ON AN ACCESSIBLE GROUND OR FLOOR SURFACE, IT SHALL HAVE A MAXIMUM COMBINED THICKNESS OF PILE, CUSHION, AND BACKING HEIGHT OF 1/2

22. ANY PART OF AN ACCESSIBLE ROUTE WITH A SLOPE GREATER THAN 1:20 SHALL BE CONSIDERED A RAMP. 23. THE LEAST POSSIBLE SLOPE SHALL BE USED FOR ANY RAMP. THE MAXIMUM SLOPE OF A RAMP IN NEW CONSTRUCTION SHALL BE 1:12. THE MAXIMUM RISE

FOR ANY RUN SHALL BE 30 IN. 24. THE MINIMUM CLEAR WIDTH OF A RAMP SHALL BE 44 IN. HANDRAILS SHALL NOT REDUCE THE REQUIRED WIDTH CLEARANCES OF A RAMP RUN OR LANDING.

25. RAMPS SHALL HAVE LEVEL LANDINGS AT THE BOTTOM AND TOP OF EACH RUN. LANDINGS SHALL HAVE THE FOLLOWING FEATURES: (A) THE LANDING SHALL BE AT LEAST AS WIDE AS THE RAMP RUN LEADING TO IT. (B) THE LANDING LENGTH SHALL BE A MINIMUM OF 60 IN. CLEAR. (C) IF RAMPS CHANGE DIRECTION AT LANDINGS, THE MINIMUM LANDING SIZE SHALL BE 60 IN. BY 60 IN.

26. IF A RAMP RUN HAS A RISE GREATER THAN 6 IN. OR A HORIZONTAL PROJECTION GREATER THAN 72 IN., OR A SLOPE STEEPER THAN 1:20, THEN IT SHALL HAVE HANDRAILS ON BOTH SIDES. HANDRAILS ARE NOT REQUIRED ON CURB RAMPS. HANDRAILS SHALL HAVE THE FOLLOWING FEATURES: (A) HANDRAILS SHALL BE PROVIDED ALONG BOTH SIDES OF RAMP SEGMENTS. THE INSIDE HANDRAIL ON SWITCHBACK OR DOGLEG RAMPS SHALL ALWAYS BE CONTINUOUS. (B) IF HANDRAILS ARE NOT CONTINUOUS, THEY SHALL EXTEND AT LEAST 12 IN. BEYOND THE TOP AND BOTTOM OF THE RAMP SEGMENT AND SHALL BE PARALLEL WITH THE FLOOR OR GROUND SURFACE. (C) THE CLEAR SPACE BETWEEN THE HANDRAIL AND THE WALL SHALL BE 1-1/2 IN. (D) GRIPPING SURFACES SHALL BE CONTINUOUS. (E) TOP OF HANDRAIL GRIPPING SURFACES

SHALL BE MOUNTED BETWEEN 30 IN AND 34 IN ABOVE RAMP SURFACES. (F) ENDS OF HANDRAILS SHALL BE EITHER ROUNDED OR RETURNED SMOOTHLY TO FLOOR, WALL OR POST. (G) HANDRAILS SHALL NOT ROTATE WITHIN THEIR FITTINGS. (H) HANDRAILS SHALL NOT REDUCE THE REQUIRED WIDTH CLEARANCES OF A RAMP RUN OR LANDING. 27. RAMPS AND LANDINGS WITH DROP-OFFS SHALL HAVE CURBS, WALLS, RAILINGS, OR PROJECTING SURFACES THAT PREVENT PEOPLE FROM SLIPPING

OFF THE RAMP. CURBS SHALL BE A MINIMUM OF 4 IN HIGH 28. ON ANY GIVEN FLIGHT OF STAIRS, ALL STEPS SHALL HAVE UNIFORM RISER HEIGHTS AND UNIFORM TREAD WIDTHS. STAIR TREADS SHALL BE NO LESS THAN 11 IN WIDE, MEASURED FROM RISER TO RISER. OPEN RISERS ARE NOT PERMITTED ON ACCESSIBLE ROUTES.

29. THE UNDERSIDES OF NOSINGS SHALL NOT BE ABRUPT. THE RADIUS OF CURVATURE AT THE LEADING EDGE OF THE TREAD SHALL BE NO GREATER THAN 1/2 IN. RISERS SHALL BE SLOPED OR THE UNDERSIDE OF THE NOSING SHALL HAVE AN ANGLE NOT LESS THAN 60 DEGREES FROM THE HORIZONTAL. NOSINGS SHALL PROJECT NO MORE THAN 1-1/2 IN.

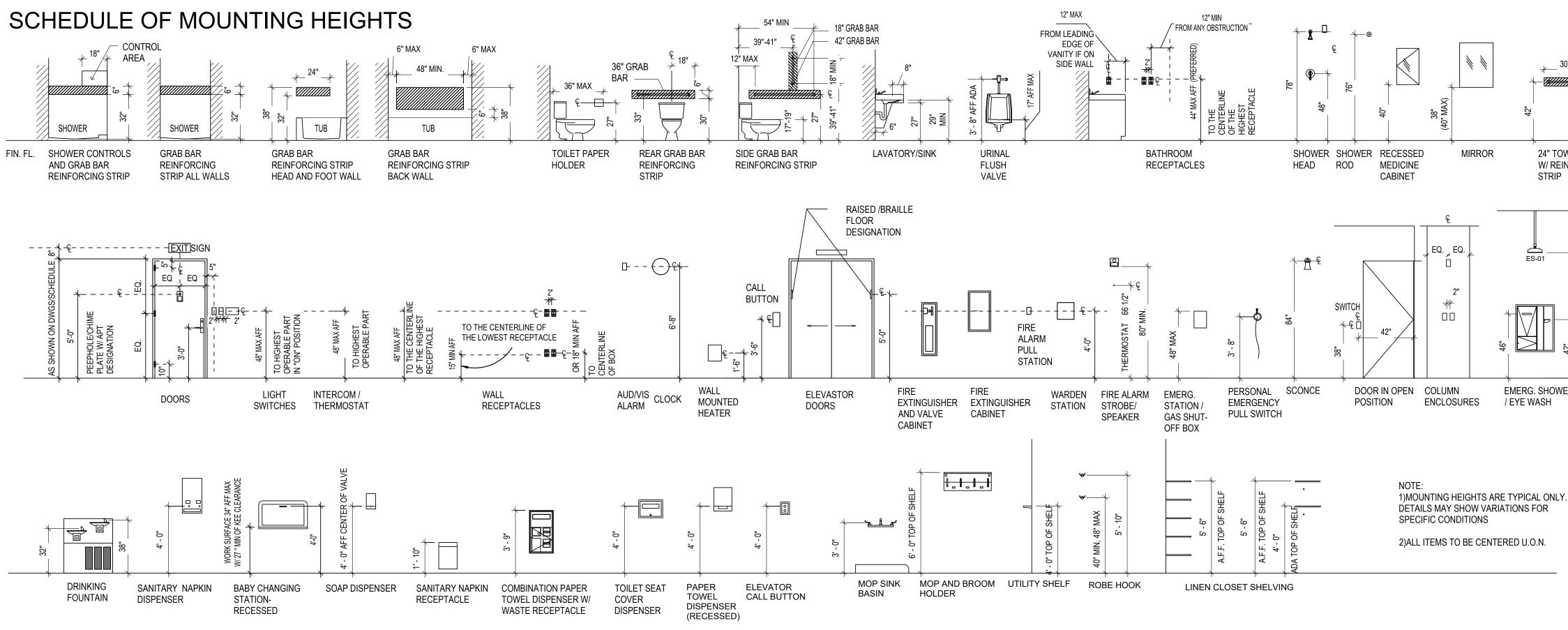
30. STAIRWAYS SHALL HAVE HANDRAILS AT BOTH SIDES OF ALL STAIRS. HANDRAILS SHALL HAVE THE FOLLOWING FEATURES. (A) HANDRAILS SHALL BE CONTINUOUS ALONG BOTH SIDES OF STAIRS. THE INSIDE HANDRAIL ON SWITCHBACK OR DOGLEG STAIRS SHALL ALWAYS BE CONTINUOUS. (B) IF HANDRAILS ARE NOT CONTINUOUS, THEY SHALL EXTEND AT LEAST 12 IN BEYOND THE TOP RISER AND AT LEAST 12 IN PLUS THE WIDTH OF ONE TREAD BEYOND THE BOTTOM RISER. AT THE TOP, THE EXTENSION SHALL BE PARALLEL WITH THE FLOOR OR GROUND SURFACE. AT THE BOTTOM, THE HANDRAIL SHALL CONTINUE TO SLOPE FOR A DISTANCE OF THE WIDTH OF ONE TREAD FROM THE BOTTOM RISER; THE REMAINDER OF THE EXTENSION SHALL BE

HORIZONTAL. (C) THE CLEAR SPACE BETWEEN HANDRAILS AND WALL SHALL BE 1-1/2 IN. (D) GRIPPING SURFACES SHALL BE UNINTERRUPTED BY NEWEL POSTS, OTHER CONSTRUCTION ELEMENTS, OR OBSTRUCTIONS. (E) TOP OF HANDRAIL GRIPPING SURFACE SHALL BE MOUNTED BETWEEN 34 IN AND 38 IN ABOVE STAIR NOSINGS (F) ENDS OF HANDRAILS SHALL BE EITHER ROUNDED OR RETURNED SMOOTHLY TÓ FLOOR, WALL, OR POST. (G) HANDRAILS SHALL NOT ROTATE WITHIN THEIR FITTINGS

31. ELEVATOR OPERATION SHALL BE AUTOMATIC. EACH CAR SHALL BE EQUIPPED WITH A SELF-LEVELING FEATURE THAT WILL AUTOMATICALLY BRING THE CAR TO FLOOR LANDINGS WITHIN A TOLERANCE OF <sup>1</sup>/<sub>2</sub> UNDER RATED LOADING TO ZERO LOADING CONDITIONS. THIS SELF-LEVELING FEATURE SHALL BE AUTOMATIC AND INDEPENDENT OF THE OPERATING DEVICE AND SHALL CORRECT THE OVER-TRAVEL OR UNDER-TRAVEL.

32. CALL BUTTONS IN ELEVATOR LOBBIES AND HALLS SHALL BE CENTERED AT 42 IN ABOVE THE FLOOR. SUCH CALL BUTTONS SHALL HAVE VISUAL SIGNALS TO INDICATE WHEN EACH CALL IS REGISTERED AND WHEN EACH CALL IS ANSWERED. CALL BUTTONS SHALL BE A MINIMUM OF  $\frac{3}{2}$  IN IN THE SMALLEST DIMENSION. THE BUTTON DESIGNATING THE UP DIRECTION SHALL BE ON TOP. BUTTONS SHALL BE RAISED OR FLUSH. OBJECTS MOUNTED BENEATH HALL CALL BUTTONS SHALL NOT PROJECT INTO THE ELEVATOR LOBBY MORE THAN 4 IN.

33. A VISIBLE AND AUDIBLE SIGNAL SHALL BE PROVIDED AT EACH HOIST WAY ENTRANCE TO INDICATE WHICH CAR IS ANSWERING A CALL. AUDIBLE SIGNS SHALL SOUND ONCE FOR THE UP DIRECTION AND TWICE FOR THE DOWN DIRECTION OR SHALL HAVE VERBAL ENUNCIATORS THAT SAY "UP" OR "DOWN."



VISIBLE SIGNALS SHALL HAVE THE FOLLOWING FEATURES: (A) HALL LANTERN FIXTURES SHALL BE MOUNTED SO THAT THEIR CENTERLINE IS AT LEAST 72 IN ABOVE THE LOBBY FLOOR. (B) VISUAL ELEMENTS SHALL BE AT LEAST 2-1/2 IN. IN THE SMALLEST DIMENSION. (C) SIGNALS SHALL BE VISIBLE FROM THE VICINITY OF THE HALL CALL BUTTON. IN-CAR LANTERNS LOCATED IN CARS, VISIBLE FROM THE VICINITY OF HALL CALL BUTTONS, AND CONFORMING TO THE ABOVE REQUIREMENTS, SHALL BE ACCEPTABLE.

DESIGNATIONS PROVIDED ON BOTH JAMBS. THE CENTERLINE OF THE CHARACTERS SHALL BE 60 IN FROM THE FLOOR. SUCH CHARACTERS SHALL BE 2 IN HIGH. PERMANENTLY APPLIED PLATES ARE ACCEPTABLE IF THEY ARE PERMANENTLY FIXED TO THE JAMBS.

34. ALL ELEVATOR HOIST WAY ENTRANCES SHALL HAVE RAISED FLOOR

35. ELEVATOR DOORS SHALL OPEN AND CLOSE AUTOMATICALLY. THEY SHALL BE PROVIDED WITH A REOPENING DEVICE THAT WILL STOP AND REOPEN A CAR DOOR AND HOIST WAY DOOR AUTOMATICALLY IF THE DOOR BECOMES OBSTRUCTED BY AN OBJECT OR PERSON. THE DEVICE SHALL BE CAPABLE OF COMPLETING THESE OPERATIONS WITHOUT REQUIRING CONTACT FOR AN OBSTRUCTION PASSING THROUGH THE OPENING AT HEIGHTS OF 5 IN AND 29 IN FROM THE FLOOR. DOOR REOPENING DEVICES SHALL REMAIN EFFECTIVE FOR AT LEAST 20 SECONDS. AFTER SUCH AN INTERVAL, DOORS MAY CLOSE IN ACCORDANCE WITH THE REQUIREMENTS OF ANSI A17.1-1978 AND A17.1A-1979.

36. THE MINIMUM ACCEPTABLE TIME FROM NOTIFICATION THAT A CAR IS ANSWERING A CALL UNTIL THE DOORS OF THAT CAR START TO CLOSE SHALL BE CALCULATION FROM THE FOLLOWING EQUATION: T=D OR T = D1.5 FT/S 445 MM/S WHERE T = TOTAL TIME IN SECONDS AND D = DISTANCE IN FEET FROM A POINT IN THE LOBBY OR CORRIDOR 60 IN DIRECTLY IN FRONT OF THE FARTHEST CALL BUTTON CONTROLLING THAT CAR TO THE CENTERLINE OF ITS HOIST WAY DOOR. FOR CARS WITH IN-CAR LANTERNS, T BEGINS WHEN THE LANTERN IS VISIBLE FROM THE VICINITY OF HALL CALL BUTTONS AND AN AUDIBLE SIGNAL IS SOUNDED. THE MINIMUM ACCEPTABLE NOTIFICATION TIME SHALL BE 5 SECONDS. 37. THE MINIMUM TIME FOR ELEVATOR DOORS TO REMAIN FULLY OPEN IN RESPONSE TO A CAR CALL SHALL BE 3 SECONDS.

38. THE FLOOR AREA OF ELEVATOR CARS SHALL PROVIDE SPACE FOR WHEELCHAIR USERS TO ENTER THE CAR, MANEUVER WITHIN REACH OF CONTROLS, AND EXIT FROM THE CAR. THE CLEARANCE BETWEEN THE CAR PLATFORM SILL AND THE EDGE OF ANY HOIST WAY LANDING SHALL BE NO GREATER THAN 1-1/4 IN.

39. THE LEVEL OF ILLUMINATION AT THE CAR CONTROLS, PLATFORM, AND CAR THRESHOLD AND LANDING SILL SHALL BE AT LEAST 5 FOOT CANDLES.

40. ELEVATOR CONTROL PANELS SHALL HAVE THE FOLLOWING FEATURES: (A) BUTTONS. ALL CONTROL BUTTONS SHALL BE AT LEAST 3/4 IN (19 MM) IN THEIR SMALLEST DIMENSION. THEY MAY BE RAISED OR FLUSH. (B) TACTILE AND VISUAL CONTROL INDICATORS. ALL CONTROL BUTTONS SHALL BE DESIGNATED BY RAISED STANDARD ALPHABET CHARACTERS FOR LETTERS. ARABIC CHARACTERS FOR NUMERALS, OR STANDARD SYMBOLS, AND AS REQUIRED IN ANSI A17.1-1978 AND A17.1A-1979. THE CALL BUTTON FOR THE MAIN ENTRY FLOOR SHALL BE DESIGNATED BY A RAISED STAR AT THE LEFT OF THE FLOOR DESIGNATION. ALL RAISED DESIGNATIONS FOR CONTROL BUTTONS SHALL BE PLACED IMMEDIATELY TO THE LEFT OF THE BUTTON TO WHICH THEY APPLY. APPLIED PLATES, PERMANENTLY ATTACHED, ARE AN ACCEPTABLE MEANS TO PROVIDE RAISED CONTROL DESIGNATIONS. FLOOR BUTTONS SHALL BE PROVIDED WITH VISUAL INDICATORS TO SHOW WHEN EACH CALL IS REGISTERED. THE VISUAL INDICATORS SHALL BE EXTINGUISHED WHEN EACH CALL IS ANSWERED. (C) HEIGHT. ALL FLOOR BUTTONS SHALL BE NO HIGHER THAN 48 IN, UNLESS THERE IS A SUBSTANTIAL INCREASE IN COST. IN WHICH CASE THE MAXIMUM MOUNTING HEIGHT MAY BE INCREASED TO 54 IN. ABOVE THE FLOOR. EMERGENCY CONTROLS. INCLUDING THE EMERGENCY ALARM AND EMERGENCY STOP, SHALL BE GROUPED AT THE BOTTOM OF THE PANEL AND SHALL HAVE THEIR CENTERLINES NO LESS THAN 35 IN ABOVE THE FLOOR. (D) LOCATION. CONTROLS SHALL BE LOCATED ON A FRONT WALL IF CARS HAVE CENTER OPENING DOORS, AND AT THE SIDE WALL OR AT THE FRONT WALL NEXT TO THE DOOR IF CARS HAVE SIDE OPENING DOORS.

41. IN ELEVATOR CARS, A VISUAL CAR POSITION INDICATOR SHALL BE PROVIDED ABOVE THE CAR CONTROL PANEL OR OVER THE DOOR TO SHOW THE POSITION OF THE ELEVATOR IN THE HOIST WAY. AS THE CAR PASSES OR STOPS AT A FLOOR SERVED BY THE ELEVATORS, THE CORRESPONDING NUMERALS SHALL ILLUMINATE, AND AN AUDIBLE SIGNAL SHALL SOUND. NUMERALS SHALL BE A MINIMUM OF 1/2 IN HIGH. THE AUDIBLE SIGNAL SHALL BE NO LESS THAN 20 DECIBELS WITH A FREQUENCY NO HIGHER THAN 1500 HZ. AN AUTOMATIC VERBAL ANNOUNCEMENT OF THE FLOOR NUMBER AT WHICH A CAR STOPS OR WHICH A CAR PASSES MAY BE SUBSTITUTED FOR THE AUDIBLE SIGNAL

42. EMERGENCY TWO-WAY COMMUNICATION SYSTEMS BETWEEN THE ELEVATOR AND A POINT OUTSIDE THE HOIST WAY SHALL COMPLY WITH ANSI A17.1-1978 AND A17.1A-1979. THE HIGHEST OPERABLE PART OF A TWO-WAY COMMUNICATION SYSTEM SHALL BE A MAXIMUM OF 48 IN FROM THE FLOOR OF THE CAR. IT SHALL BE IDENTIFIED BY A RAISED OR RECESSED SYMBOL AND LETTERING AND LOCATED ADJACENT TO THE DEVICE. IF THE SYSTEM USES A HANDSET, THEN THE LENGTH OF THE CORD FROM THE PANEL TO THE HANDSET SHALL BE AT LEAST 29 IN. IF THE SYSTEM IS LOCATED IN A CLOSED COMPARTMENT, THE COMPARTMENT DOOR HARDWARE SHALL CONFORM TO 4.27, CONTROLS AND OPERATING MECHANISMS. THE EMERGENCY INTERCOMMUNICATION SYSTEM SHALL NOT REQUIRE VOICE COMMUNICATION.

43. DOORWAYS SHALL HAVE A MINIMUM CLEAR OPENING OF 32 IN WITH THE DOOR OPEN 90 DEGREES, MEASURED BETWEEN THE FACE OF THE DOOR AND THE

STOP

44. MINIMUM MANEUVERING CLEARANCES AT DOORS THAT ARE NOT AUTOMATIC OR POWER-ASSISTED SHALL BE AS SHOWN ON SHEET G-006. THE FLOOR OR GROUND AREA WITHIN THE REQUIRED CLEARANCES SHALL BE LEVEL AND CLEAR. 45. THE MINIMUM SPACE BETWEEN TWO HINGED OR PIVOTED DOORS IN SERIES SHALL BE 48 IN PLUS THE WIDTH OF ANY DOOR SWINGING INTO THE SPACE. DOORS IN SERIES SHALL SWING EITHER IN THE SAME DIRECTION OR AWAY FROM

46. THRESHOLDS AT DOORWAYS SHALL NOT EXCEED 3/4 IN. IN HEIGHT FOR EXTERIOR SLIDING DOORS OR 1/2 IN. FOR OTHER TYPES OF DOORS, RAISED THRESHOLDS AND FLOOR LEVEL CHANGES AT ACCESSIBLE DOORWAYS SHALL BE BEVELED WITH A SLOPE NO GREATER THAN 1:2.

THE SPACE BETWEEN THE DOORS.

47. IF A DOOR HAS A CLOSER, THEN THE SWEEP PERIOD OF THE CLOSER SHALL BE ADJUSTED SO THAT FROM AN OPEN POSITION OF 70 DEGREES, THE DOOR WILL TAKE AT LEAST 3 SECONDS TO MOVE TO A POINT 3 IN FROM THE LATCH, MEASURED TO THE LEADING EDGE OF THE DOOR.

48. THE MAXIMUM FORCE FOR PUSHING OR PULLING OPEN A DOOR SHALL BE AS

(A) FIRE DOORS SHALL HAVE THE MINIMUM OPENING FORCE ALLOWABLE BY THE APPROPRIATE ADMINISTRATIVE AUTHORITY. THESE FORCES DO NOT APPLY TO THE FORCE REQUIRED TO RETRACT LATCH BOLTS OR DISENGAGE OTHER DEVICES THAT MAY HOLD THE DOOR IN A CLOSED POSITION.

49. IF AN AUTOMATIC DOOR IS USED, THEN IT SHALL COMPLY WITH AMERICAN NATIONAL STANDARD FOR POWER-OPERATED DOORS, ANSI A156.10-1979. SLOWLY OPENING, LOW-POWERED, AUTOMATIC DOORS SHALL BE CONSIDERED A TYPE OF CUSTOM DESIGN INSTALLATION AS DESCRIBED IN PARAGRAPH 1.1.1 OF ANSI A156.10-1979. SUCH DOORS SHALL NOT OPEN TO BACK CHECK FASTER THAN 3 SECONDS AND SHALL REQUIRE NO MORE THAN 15 LBF TO STOP DOOR MOVEMENT. IF A POWER-ASSISTED DOOR IS USED. ITS CLOSING SHALL CONFORM TO THE REQUIREMENTS IN SECTION 10 OF ANSI A156.10-1979.

50. ENTRANCES REQUIRED TO BE ACCESSIBLE SHALL BE CONNECTED BY AN ACCESSIBLE ROUTE TO PUBLIC TRANSPORTATION STOPS, TO ACCESSIBLE PARKING AND PASSENGER LOADING ZONES, AND TO PUBLIC STREETS OR SIDEWALKS IF AVAILABLE. THEY SHALL ALSO BE CONNECTED BY AN ACCESSIBLE ROUTE TO ALL ACCESSIBLE SPACES OR ELEMENTS WITHIN THE BUILDING OR FACILITY.

51. DRINKING FOUNTAIN SPOUTS SHALL BE NO HIGHER THAN 36 IN MEASURED FROM THE FLOOR OR GROUND SURFACES TO THE SPOUT OUTLET.

52. THE SPOUTS OF DRINKING FOUNTAINS AND WATER COOLERS SHALL BE AT THE FRONT OF THE UNIT AND SHALL DIRECT THE WATER FLOW IN A TRAJECTORY THAT IS PARALLEL OR NEARLY PARALLEL TO THE FRONT OF THE UNIT. THE SPOUT SHALL PROVIDE A FLOW OF WATER AT LEAST 4 IN HIGH SO AS TO ALLOW THE INSERTION OF A CUP OR GLASS UNDER THE FLOW OF WATER.

53. DRINKING FOUNTAIN UNIT CONTROLS SHALL BE FRONT MOUNTED OR SIDE MOUNTED NEAR THE FRONT EDGE.

54. CLEARANCES AT DRINKING FOUNTAINS SHALL BE AS FOLLOWS. (A) WALL- AND POST-MOUNTED CANTILEVERED UNITS SHALL HAVE A CLEAR KNEE SPACE BETWEEN THE BOTTOM OF THE APRON AND THE FLOOR OR GROUND AT LEAST 27 IN HIGH, 30 IN WIDE, AND 17 IN TO 19 IN DEEP. SUCH UNITS SHALL ALSO HAVE A MINIMUM CLEAR FLOOR SPACE 30 IN BY 48 IN TO ALLOW A PERSON IN A WHEELCHAIR TO APPROACH THE UNIT FACING FORWARD. (B) FREE STANDING OR BUILT-IN UNITS NOT HAVING A CLEAR SPACE UNDER THEM SHALL HAVE A CLEAR FLOOR SPACE AT LEAST 30 IN BY 48 IN THAT ALLOWS A PERSON IN A WHEELCHAIR TO MAKE A PARALLEL APPROACH TO THE UNIT.

55. THE HEIGHT OF WATER CLOSETS SHALL BE 17 IN TO 19 IN. MEASURED TO THE TOP OF THE TOILET SEAT. SEATS SHALL NOT BE SPRUNG TO RETURN TO A LIFTED POSITION.

56. FLUSH CONTROLS SHALL BE HAND OPERATED OR AUTOMATIC. CONTROLS FOR FLUSH VALVES SHALL BE MOUNTED ON THE WIDE SIDE OF TOILET AREAS NO MORE THAN 44 IN ABOVE THE FLOOR.

57. TOILET PAPER DISPENSERS SHALL BE INSTALLED WITHIN REACH. DISPENSERS THAT CONTROL DELIVERY, OR THAT DO NOT PERMIT CONTINUOUS PAPER FLOW, SHALL NOT BE USED.

58. TOILET STALLS WITH A MINIMUM DEPTH OF 56 IN SHALL HAVE WALL-MOUNTED WATER CLOSETS. IF THE DEPTH OF TOILET STALLS IS INCREASED AT LEAST 3 IN. THEN A FLOOR-MOUNTED WATER CLOSET MAY BE USED

59. IN STANDARD STALLS, THE FRONT PARTITION AND AT LEAST ONE SIDE PARTITION SHALL PROVIDE A TOE CLEARANCE OF AT LEAST 9 IN ABOVE THE FLOOR. IF THE DEPTH OF THE STALL IS GREATER THAN 60 IN, THEN THE TOE CLEARANCE IS NOT REQUIRED.

60. IF TOILET STALL APPROACH IS FROM THE LATCH SIDE OF THE STALL DOOR, CLEARANCE BETWEEN THE DOOR SIDE OF THE STALL AND ANY OBSTRUCTION MAY BE REDUCED TO A MINIMUM OF 42 IN.

61. URINALS SHALL BE STALL-TYPE OR WALL-HUNG WITH AN ELONGATED RIM AT A MAXIMUM OF 17 IN ABOVE THE FLOOR. 62. A CLEAR FLOOR SPACE 30 IN BY 48 IN SHALL BE PROVIDED IN FRONT OF

EMERG. SHOWER / EYE WASH

24" TOWEL BAR W/ REINF. STRIP

ROUGHING OR BY A MATERIAL APPLIED TO THE CONTACT SURFACE. SUCH TEXTURED SURFACES SHALL NOT BE PROVIDED FOR EMERGENCY EXIT DOORS OR ANY DOORS OTHER THAN THOSE TO HAZARDOUS AREAS. 75. FOOD SERVICE LINES SHALL HAVE A MINIMUM CLEAR WIDTH OF 36 IN. WITH A PREFERRED CLEAR WIDTH OF 42 IN WHERE PASSAGE OF STOPPED WHEELCHAIRS BY PEDESTRIANS IS DESIRED. TRAY SLIDES SHALL BE MOUNTED NO HIGHER THAN

DOOR AND EXIT. 69. THE ACCESSIBLE FIXTURES AND CONTROLS SHALL BE ON AN ACCESSIBLE ROUTE. AN UNOBSTRUCTED TURNING SPACE OF 60 IN SHALL BE PROVIDED WITHIN AN ACCESSIBLE TOILET ROOM. THE CLEAR FLOOR SPACE AT FIXTURES AND CONTROLS, THE ACCESSIBLE ROUTE, AND THE TURNING SPACE MAY OVERI AP. 70. SINKS SHALL BE MOUNTED WITH THE COUNTER OR RIM NO HIGHER THAN 34 IN FROM THE FLOOR.

71. KNEE CLEARANCE THAT IS AT LEAST 27 IN HIGH, 30 IN WIDE, AND 19 IN DEEP

73. A CLEAR FLOOR SPACE AT LEAST 30 IN BY 48 IN SHALL BE PROVIDED IN FRONT

OF A SINK TO ALLOW FORWARD APPROACH. THE CLEAR FLOOR SPACE SHALL BE

74. DOORS THAT LEAD TO AREAS THAT MIGHT PROVE DANGEROUS TO A BLIND

TEXTURED SURFACE ON THE DOOR HANDLE, KNOB, PULL OR OTHER OPERATING

PERSON (FOR EXAMPLE, DOORS TO LOADING PLATFORMS, BOILER ROOMS,

STAGES, AND THE LIKE) SHALL BE MADE IDENTIFIABLE TO THE TOUCH BY A

HARDWARE. THIS TEXTURED SURFACE MAY BE MADE BY KNURLING OR

ON AN ACCESSIBLE ROUTE AND SHALL EXTEND A MAXIMUM OF 19 IN

SHALL BE PROVIDED UNDERNEATH SINKS.

UNDERNEATH THE LAVATORY OR SINK.

34 IN ABOVE THE FLOOR.

72. EACH SINK SHALL BE A MAXIMUM OF 6-1/2 IN DEEP.

OTHERWISE COVERED. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER LAVATORIES. 67. MIRRORS SHALL BE MOUNTED WITH THE BOTTOM EDGE OF THE REFLECTING SURFACE NO HIGHER THAN 40 IN FROM THE FLOOR. 68. ALL DOORS TO ACCESSIBLE TOILET ROOMS SHALL BE ACCESSIBLE. DOORS SHALL NOT SWING INTO THE CLEAR FLOOR SPACE REQUIRED FOR ANY FIXTURE EXCEPT DOORS MAY SWING INTO THE CLEAR FLOOR PACE REQUIRED FOR ANY FIXTURES ONLY IN A TOILET OR BATHROOM FOR INDIVIDUAL USE THAT PROVIDES

SUFFICIENT MANEUVERING SPACE WITHIN THE ROOM FOR A PERSON USING A

WHEELCHAIR TO ENTER AND CLOSE THE DOOR, USE THE FIXTURES, REOPEN THE

65. A CLEAR FLOOR SPACE 30 IN BY 48 IN SHALL BE PROVIDED IN FRONT OF A LAVATORY TO ALLOW FORWARD APPROACH. SUCH CLEAR FLOOR SPACE SHALL ADJOIN OR OVERLAP AN ACCESSIBLE ROUTE AND SHALL EXTEND A MAXIMUM OF 19 IN UNDERNEATH THE LAVATORY. 66. HOT WATER AND DRAIN PIPES UNDER LAVATORIES SHALL BE INSULATED OR

HIGHER THAN 34 IN ABOVE THE FINISHED FLOOR. PROVIDE A CLEARANCE OF AT LEAST 29 IN FROM THE FLOOR TO THE BOTTOM OF THE APRON.

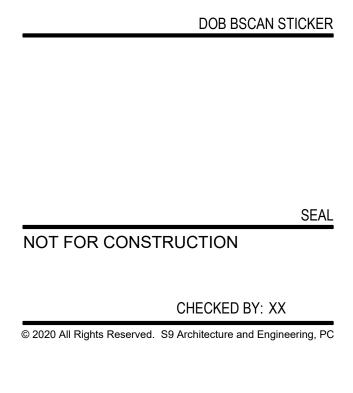
MOUNTED NO MORE THAN 44 IN. ABOVE THE FLOOR. 64. LAVATORIES SHALL BE MOUNTED WITH THE RIM OR COUNTER SURFACE NO

CLEARANCE BETWEEN THEM.

URINALS TO ALLOW FORWARD APPROACH. THIS CLEAR SPACE SHALL ADJOIN OR OVERLAP AN ACCESSIBLE ROUTE. URINAL SHIELDS THAT DO NOT EXTEND

BEYOND THE FRONT EDGE OF THE URINAL RIM MAY BE PROVIDED WITH 29 IN

63. FLUSH CONTROLS SHALL BE HAND OPERATED OR AUTOMATIC, AND SHALL BE



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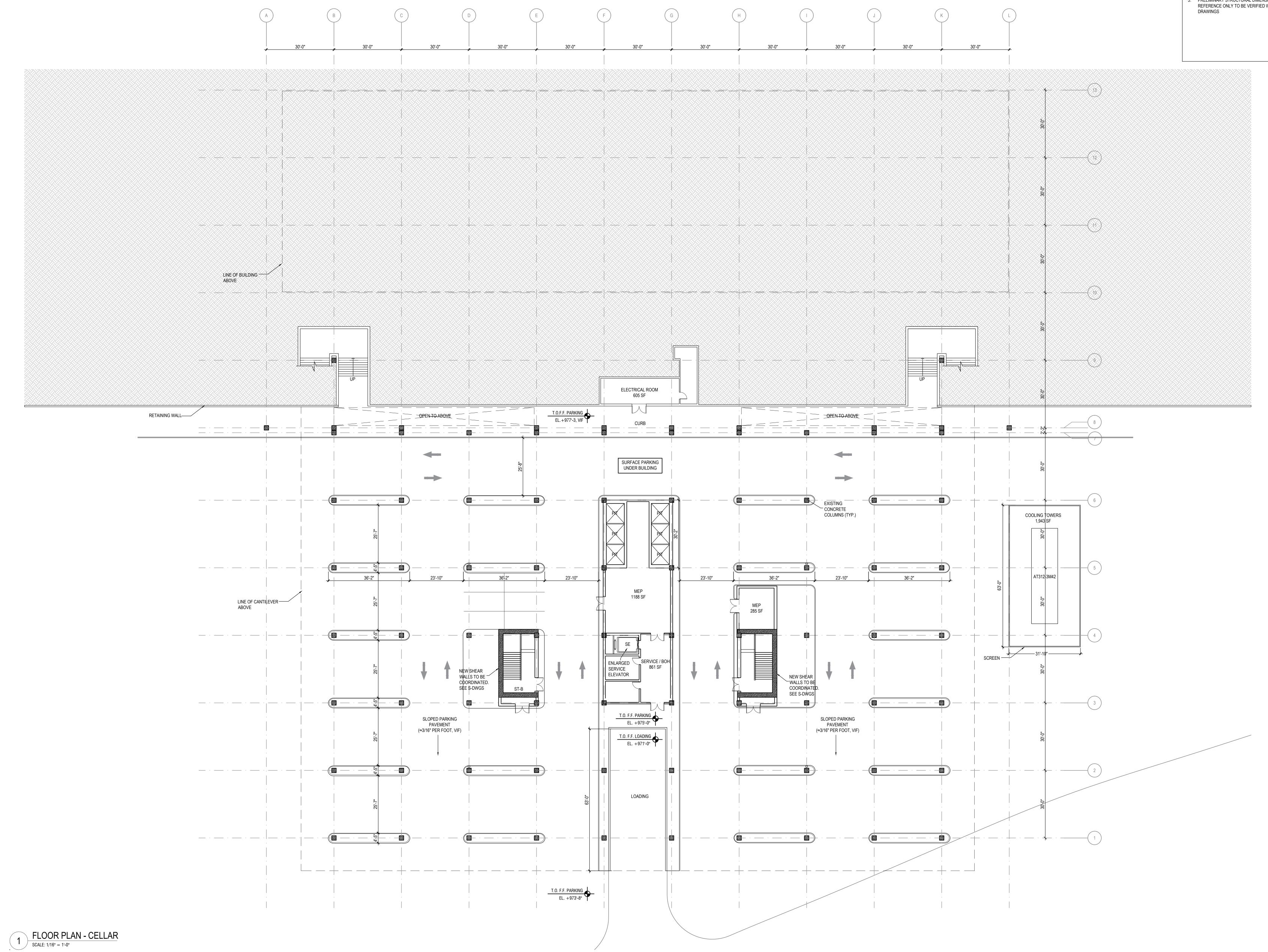
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322 8TH AVENUE

NEW YORK, NY 10001 T 212.457.4077

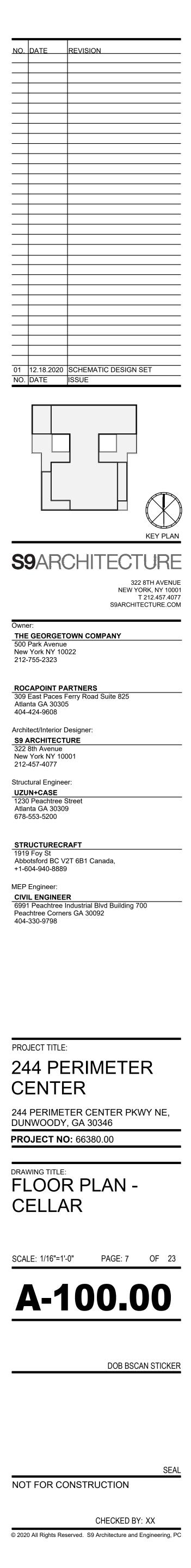
S9ARCHITECTURE.COM

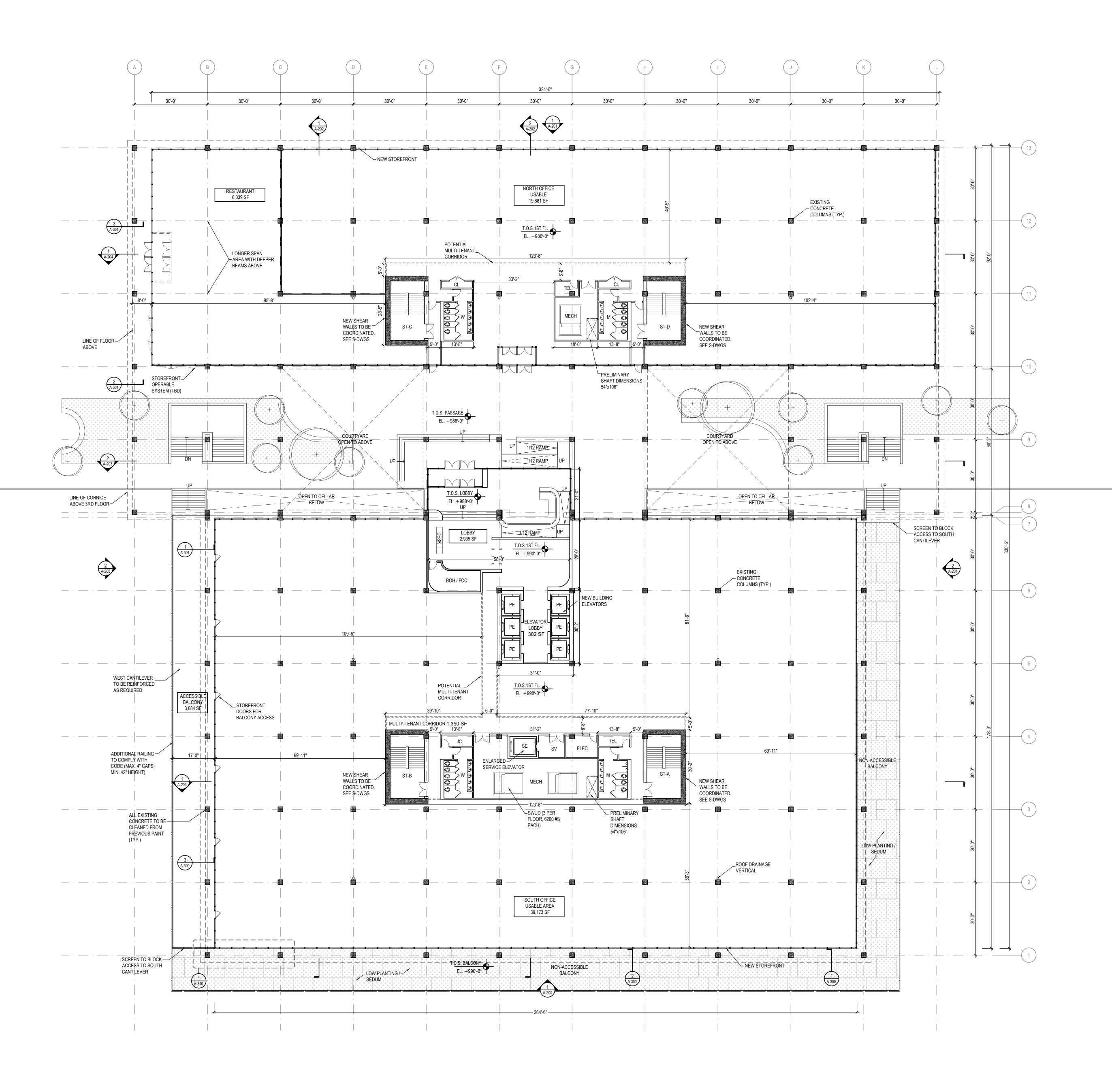
NO. DATE ISSUE



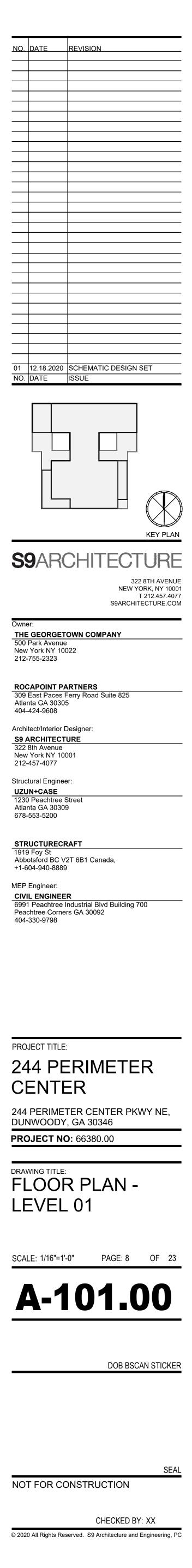


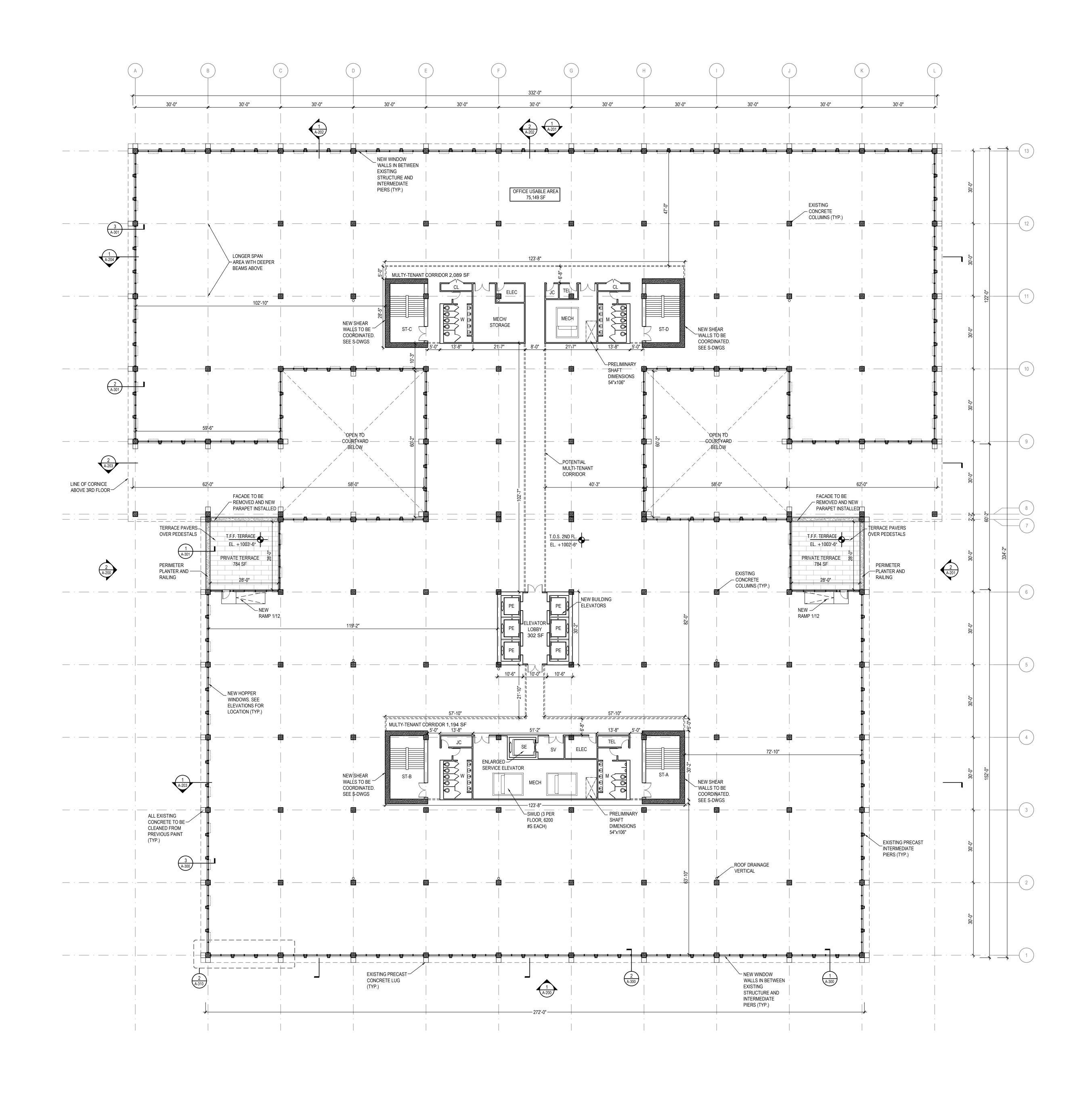
- SEE STRUCTURAL DRAWINGS FOR DISTINCTION BETWEEN EXISTING AND PROPOSED STRUCTURAL ELEMENT
- ALL DIMENSIONS TO EXISTING ELEMENTS TO BE VERIFIED IN FIELD.
- PRELIMINARY STRUCTURAL DIMENSIONS FOR REFERENCE ONLY TO BE VERIFIED WITH STRUCTURAL DRAWINGS



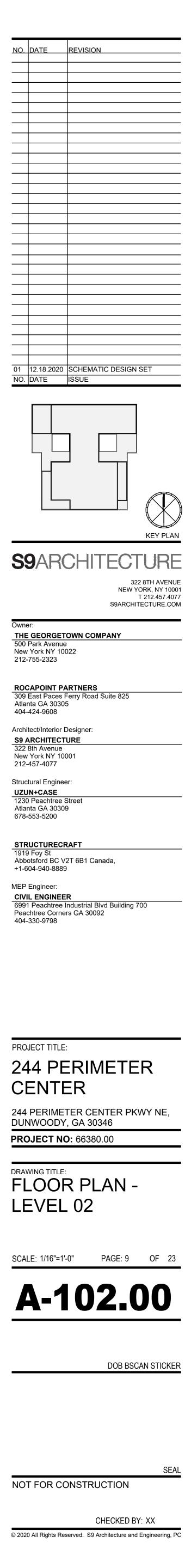


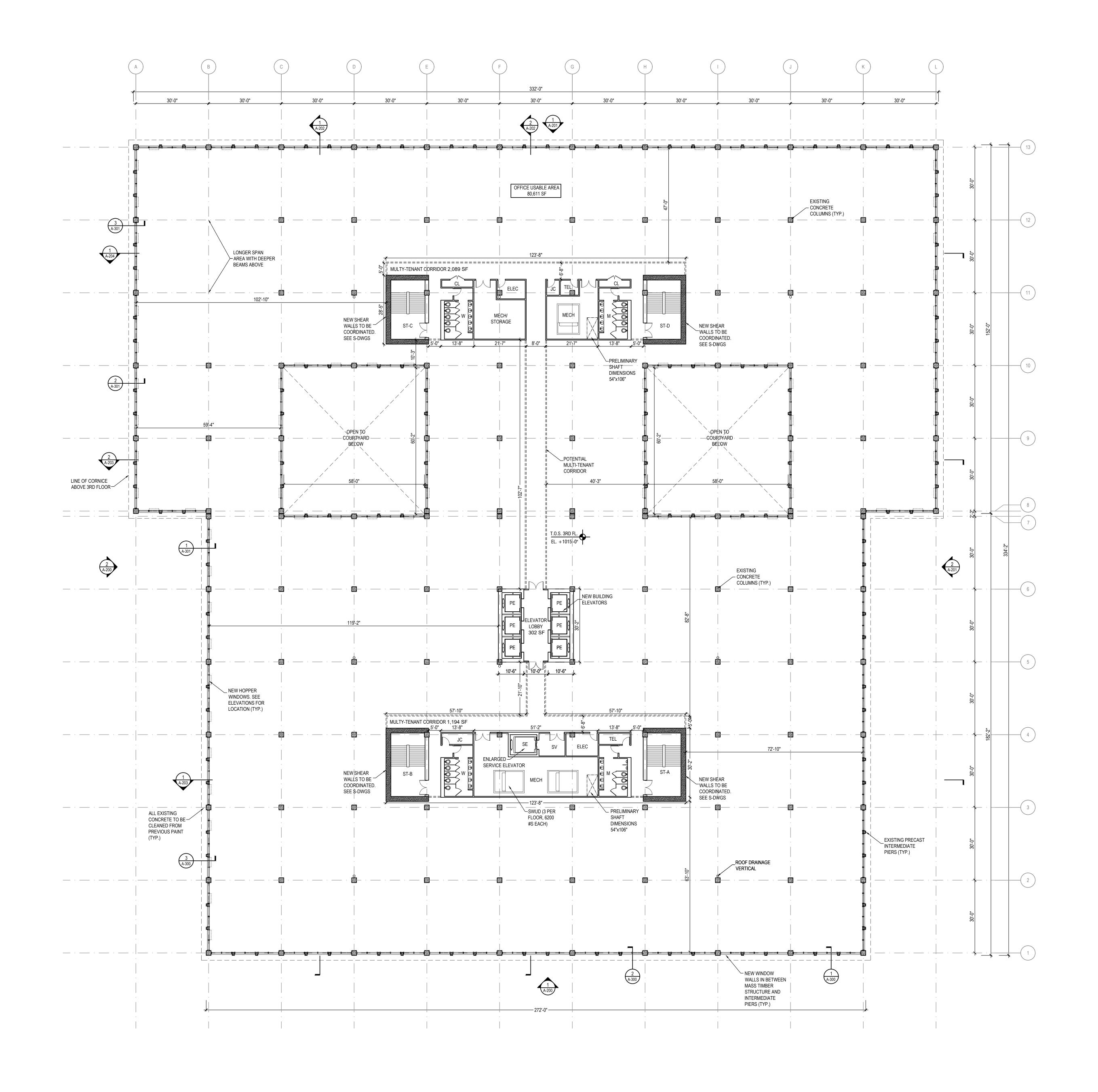
- 1. SEE STRUCTURAL DRAWINGS FOR DISTINCTION BETWEEN EXISTING AND PROPOSED STRUCTURAL ELEMENT
- . 2. ALL DIMENSIONS TO EXISTING ELEMENTS TO BE VERIFIED IN FIELD.
- B. PRELIMINARY STRUCTURAL DIMENSIONS FOR REFERENCE ONLY TO BE VERIFIED WITH STRUCTURAL DRAWINGS



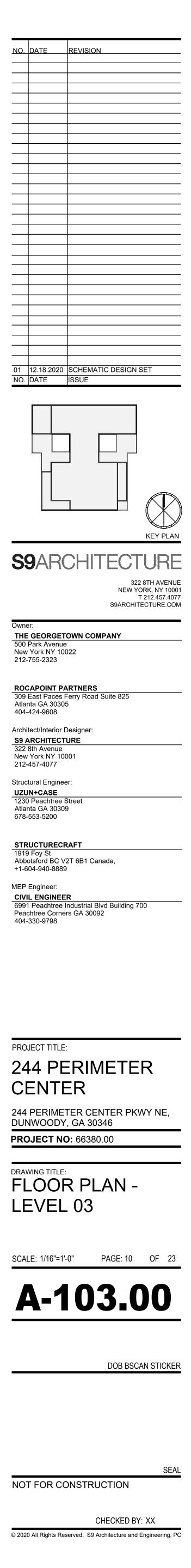


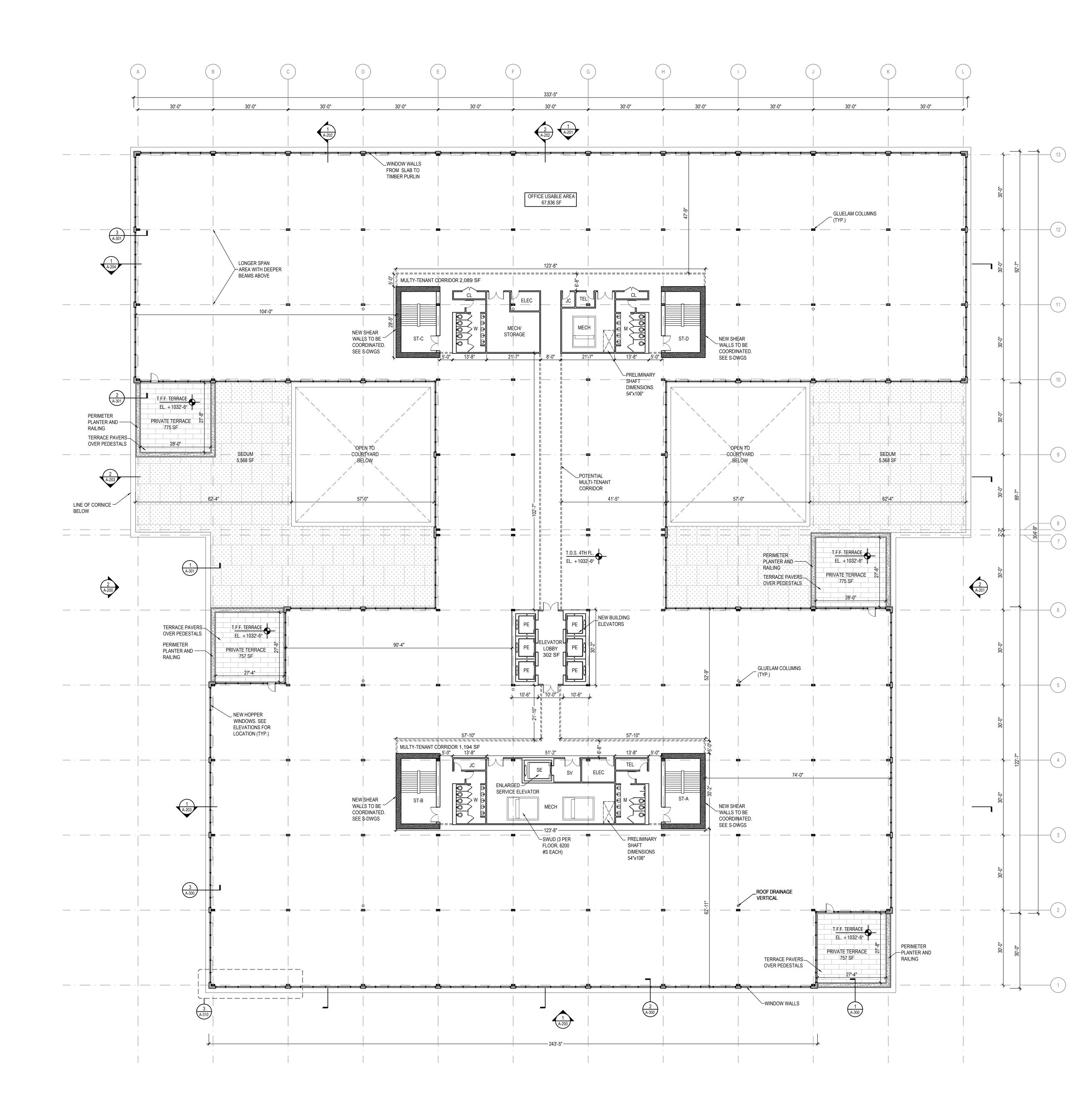
- 1. SEE STRUCTURAL DRAWINGS FOR DISTINCTION BETWEEN EXISTING AND PROPOSED STRUCTURAL ELEMENT
- 2. ALL DIMENSIONS TO EXISTING ELEMENTS TO BE VERIFIED IN FIELD.
- B. PRELIMINARY STRUCTURAL DIMENSIONS FOR REFERENCE ONLY TO BE VERIFIED WITH STRUCTURAL DRAWINGS



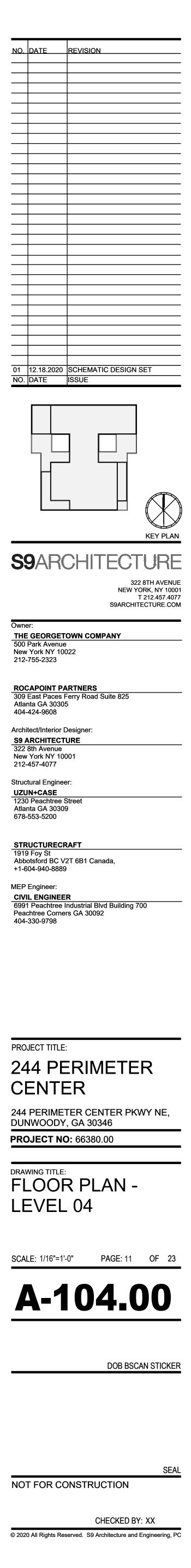


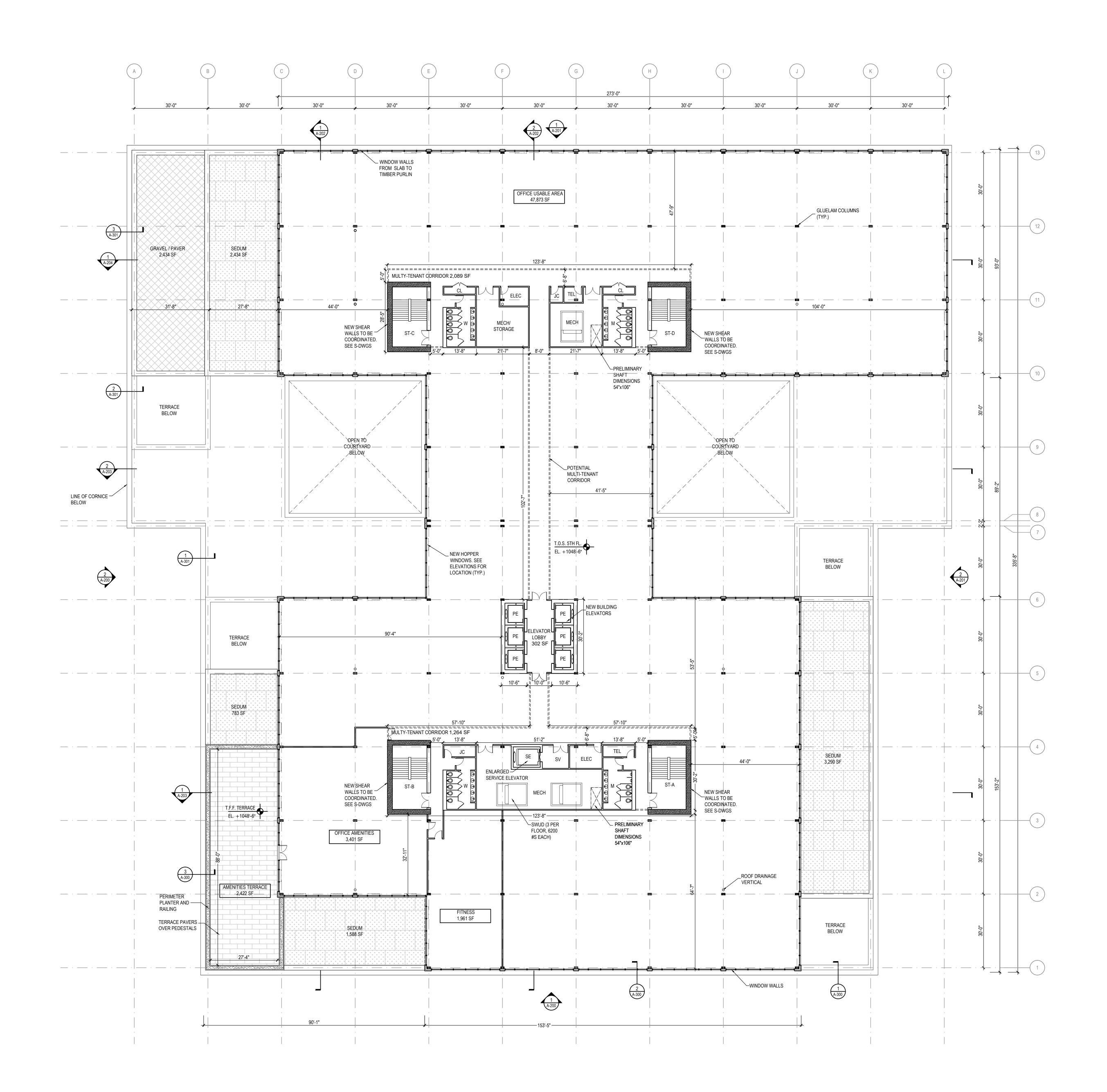
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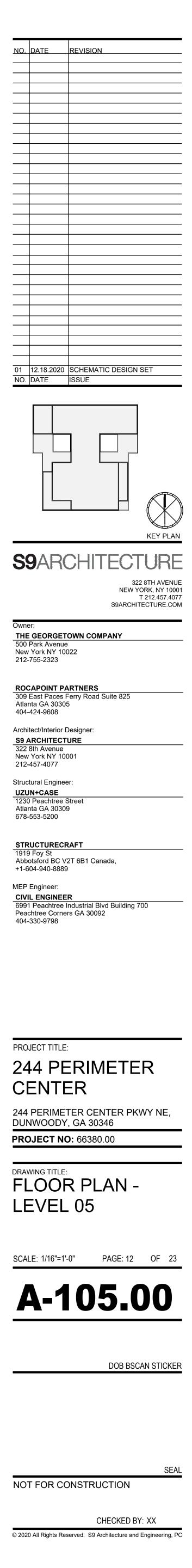


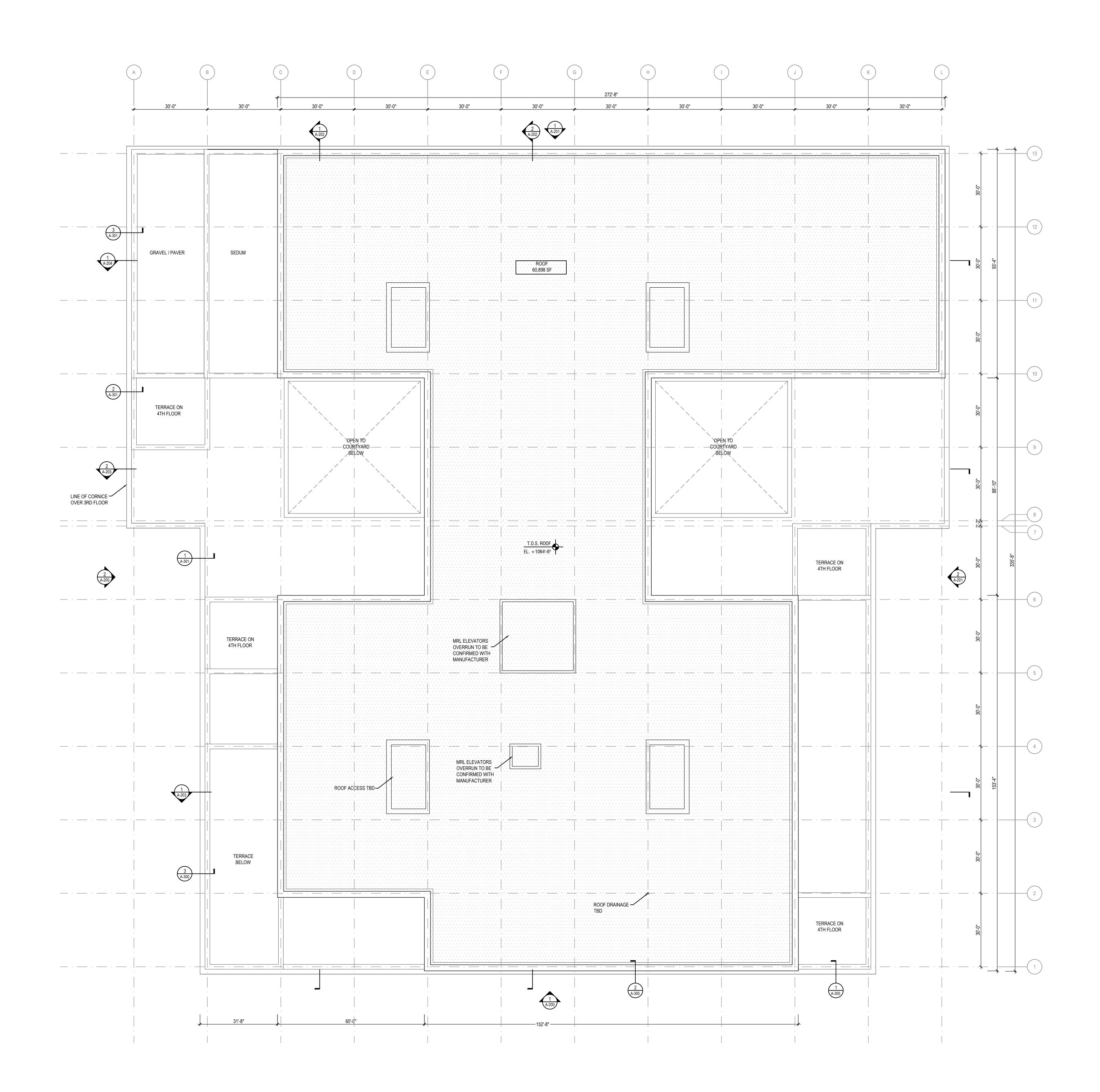
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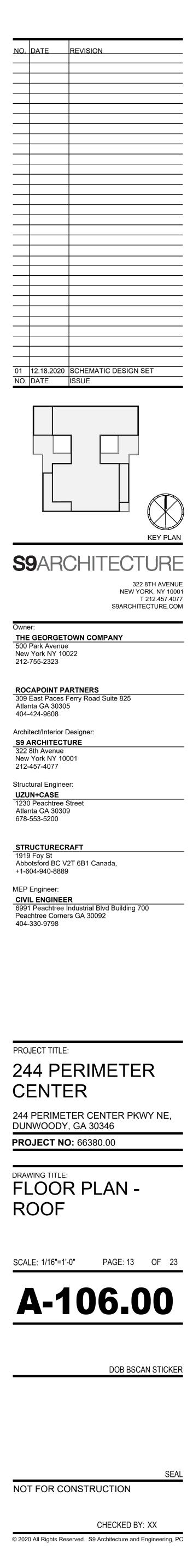


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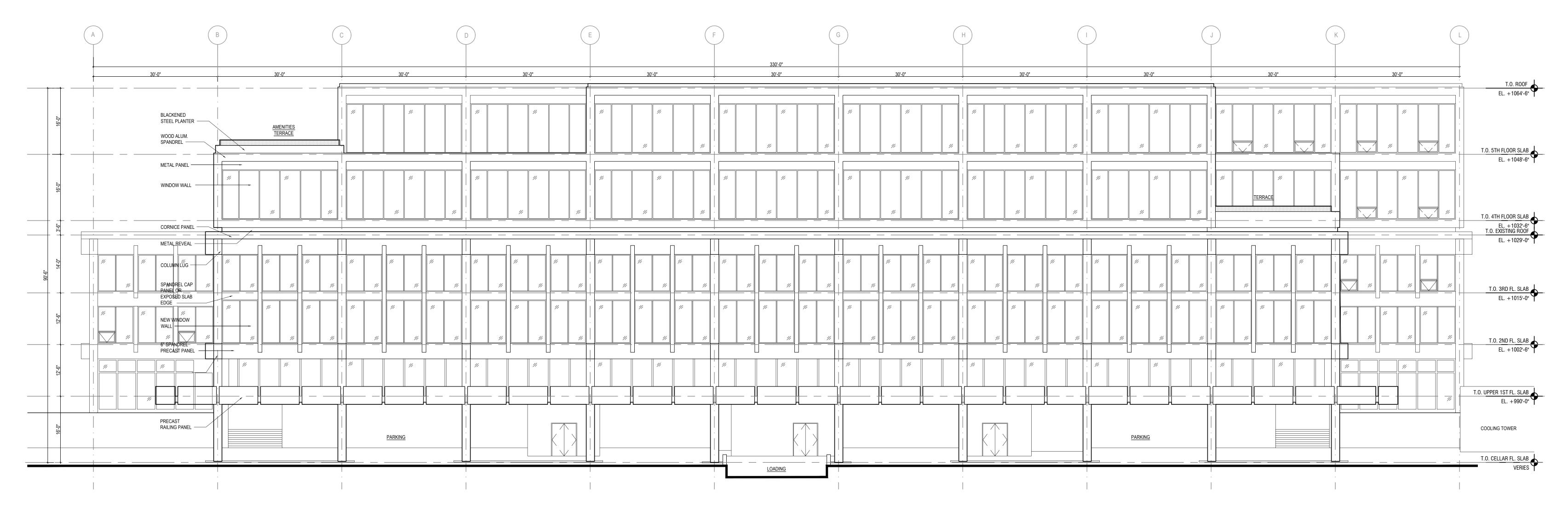




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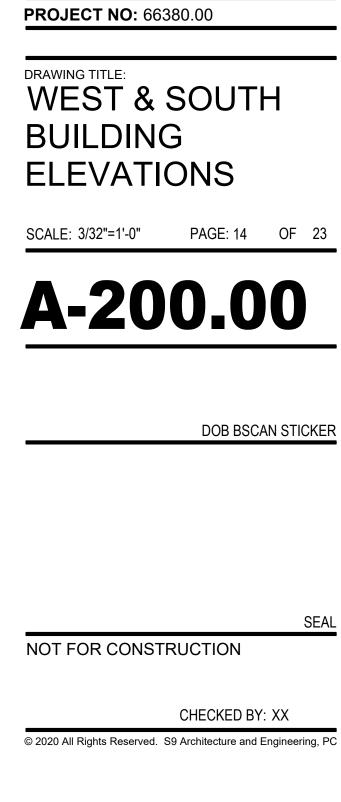


## 1 SOUTH BUILDING ELEVATION SCALE: 3/32" = 1'-0"





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**S9**ARCHITECTURE 322 8TH AVENUE NEW YORK, NY 10001 T 212.457.4077 S9ARCHITECTURE.COM Owner THE GEORGETOWN COMPANY 500 Park Avenue New York NY 10022 212-755-2323 ROCAPOINT PARTNERS 309 East Paces Ferry Road Suite 825 Atlanta GA 30305 404-424-9608 Architect/Interior Designer: S9 ARCHITECTURE 322 8th Avenue New York NY 10001 212-457-4077 Structural Engineer: UZUN+CASE 1230 Peachtree Street Atlanta GA 30309 678-553-5200 STRUCTURECRAFT 1919 Foy St Abbotsford BC V2T 6B1 Canada, +1-604-940-8889 MEP Engineer: **CIVIL ENGINEER** 6991 Peachtree Industrial Blvd Building 700 Peachtree Corners GA 30092 404-330-9798

01 NO.	12.18.2020 DATE	SCHEMATIC DESIGN SET
2 🌶		<pre> I  KEY PLAN </pre>

NO. DATE

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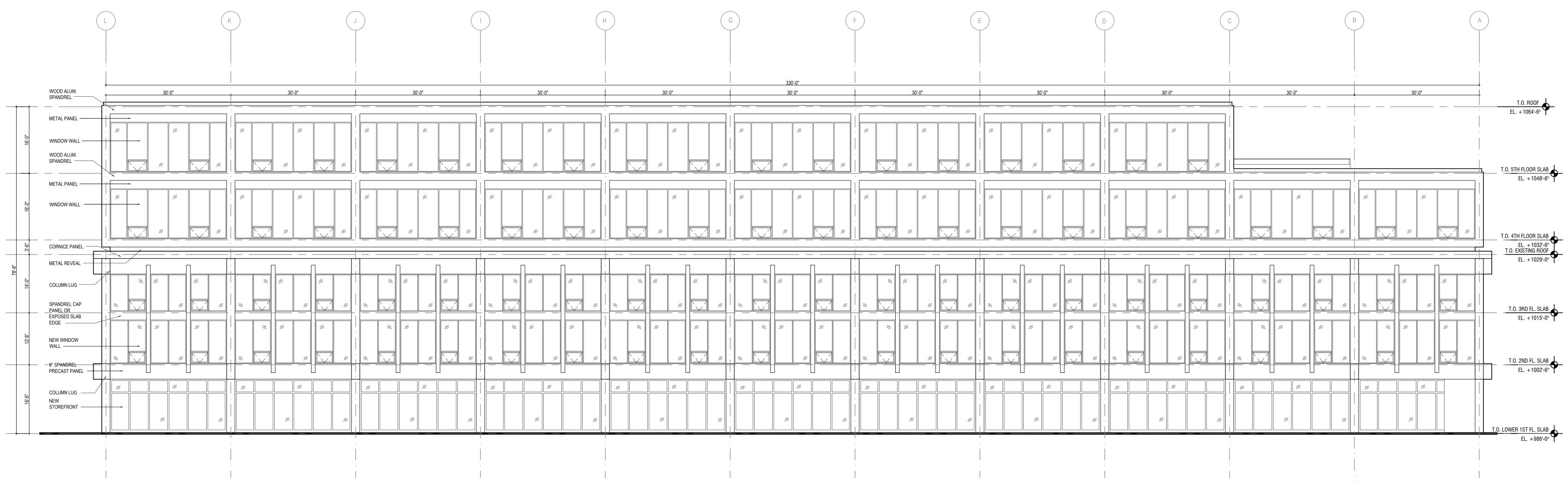
PROJECT TITLE:

CENTER

244 PERIMETER

244 PERIMETER CENTER PKWY NE, DUNWOODY, GA 30346

#### 1 NORTH BUILDING ELEVATION SCALE: 3/32" = 1'-0"





SUALE. 5/32 = 1-0

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NOT FOR CONSTRUCTION

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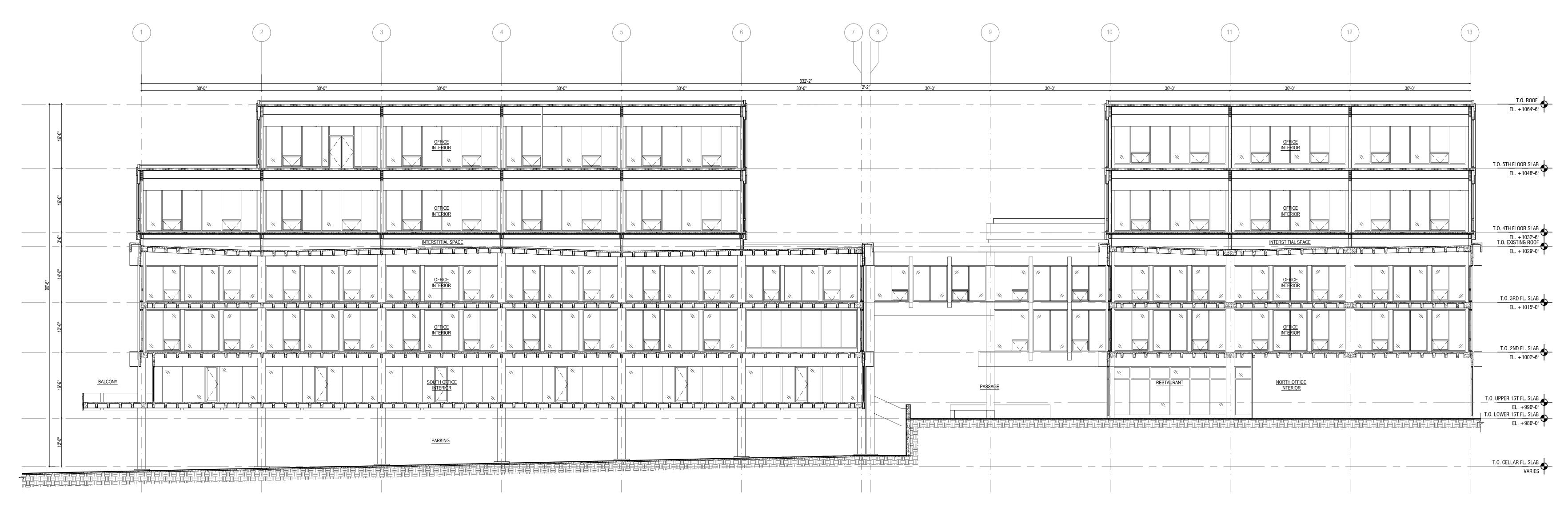
\_\_\_\_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ 01 12.18.2020 SCHEMATIC DESIGN SET NO. DATE ISSUE KEY PLAN **S9**ARCHITECTURE 322 8TH AVENUE NEW YORK, NY 10001 T 212.457.4077 S9ARCHITECTURE.COM THE GEORGETOWN COMPANY 500 Park Avenue New York NY 10022 212-755-2323 ROCAPOINT PARTNERS 309 East Paces Ferry Road Suite 825 Atlanta GA 30305 404-424-9608 Architect/Interior Designer: **S9 ARCHITECTURE** 322 8th Avenue New York NY 10001 212-457-4077 Structural Engineer: UZUN+CASE 1230 Peachtree Street Atlanta GA 30309 678-553-5200 STRUCTURECRAFT 1919 Foy St Abbotsford BC V2T 6B1 Canada, +1-604-940-8889 MEP Engineer: **CIVIL ENGINEER** 6991 Peachtree Industrial Blvd Building 700 Peachtree Corners GA 30092 404-330-9798 PROJECT TITLE: 244 PERIMETER CENTER 244 PERIMETER CENTER PKWY NE, DUNWOODY, GA 30346 PROJECT NO: 66380.00 DRAWING TITLE: EAST & NORTH BUILDING ELEVATIONS SCALE: 3/32"=1'-0" PAGE: 15 OF 23 A-201.00 DOB BSCAN STICKER

NO. DATE

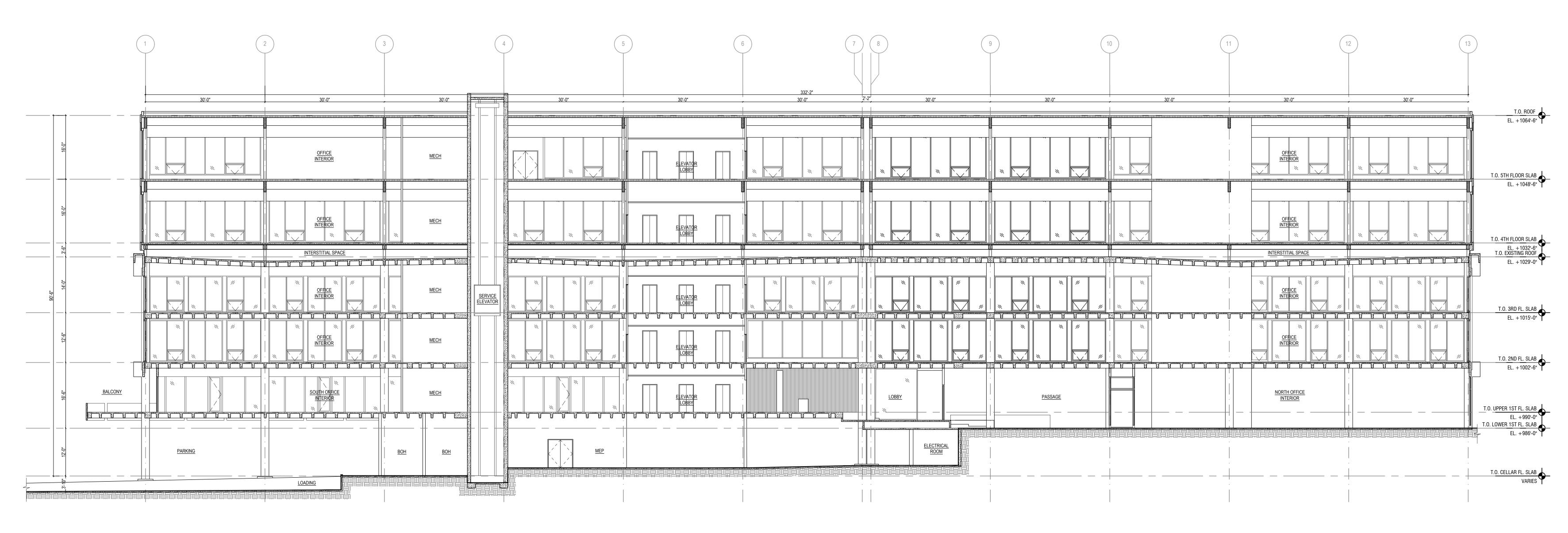
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#### 1 OVERALL BUILDING SECTION SCALE: 3/32" = 1'-0"

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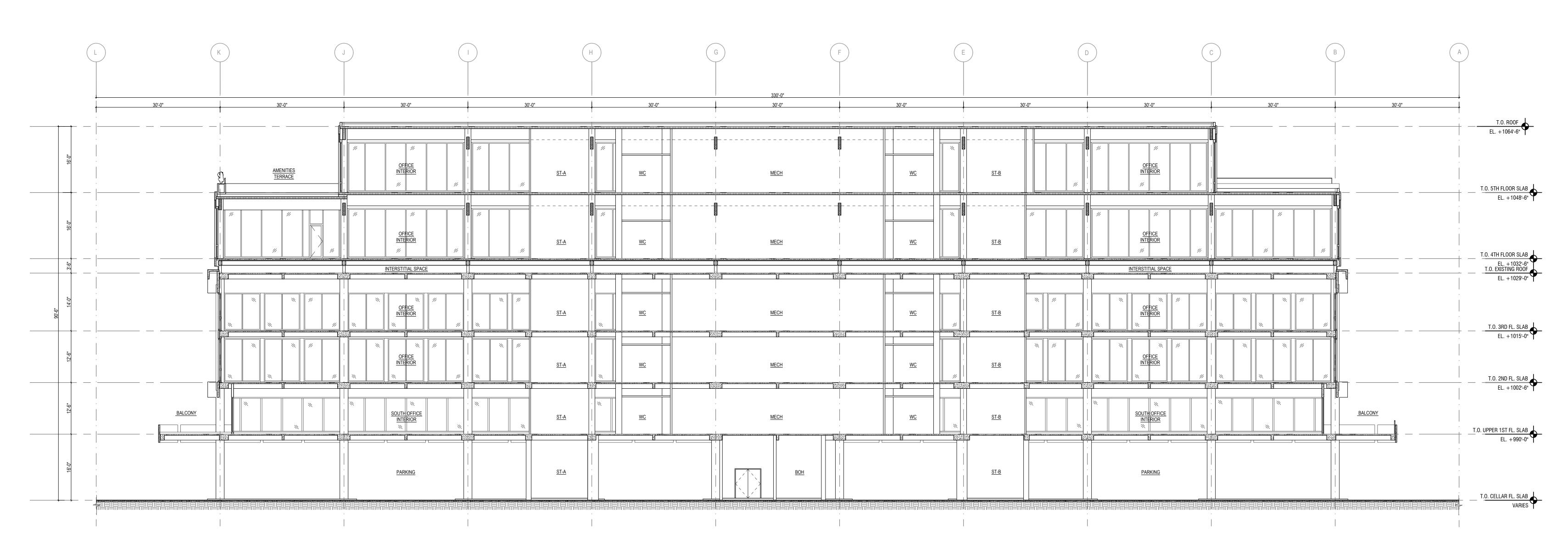


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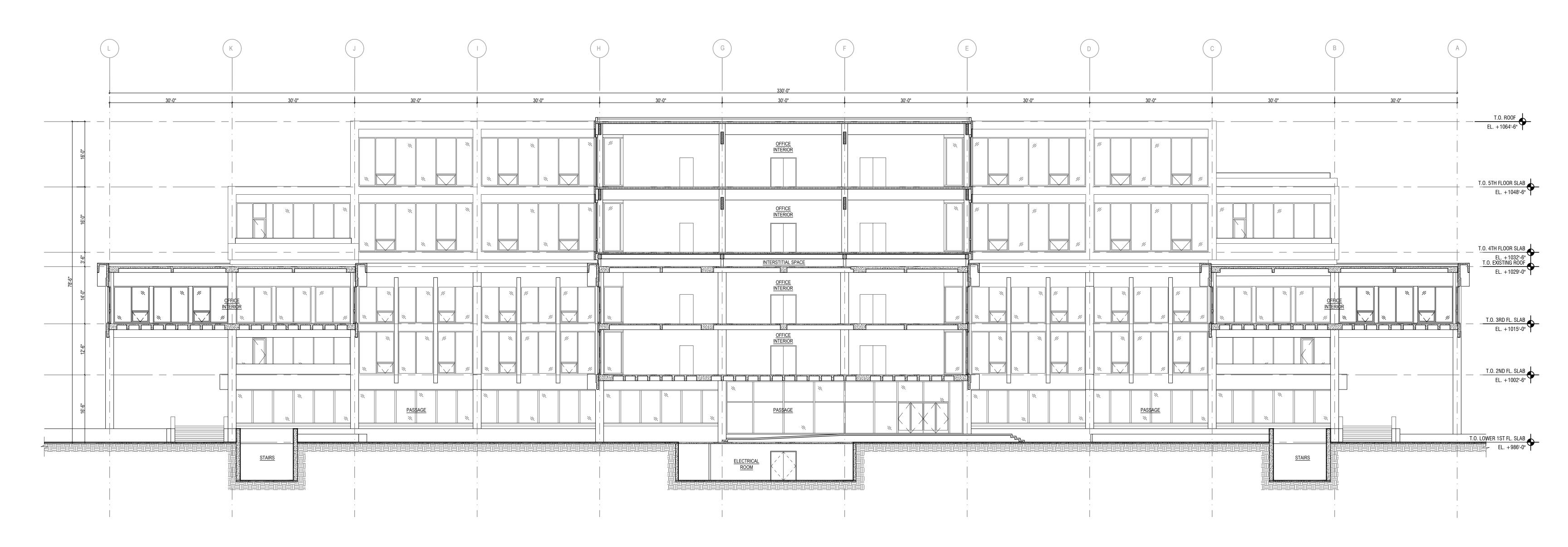




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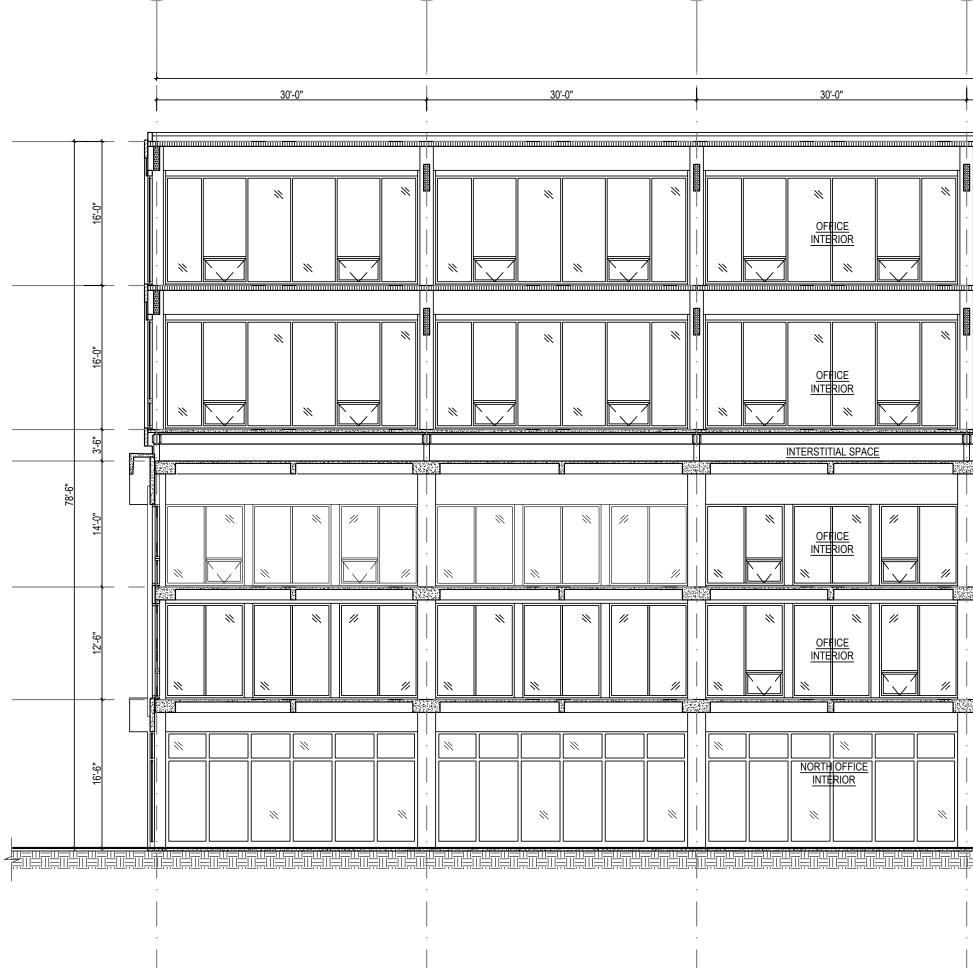
#### 2 OVERALL BUILDING SECTION SCALE: 3/32" = 1'-0"





#### 1 OVERALL BUILDING SECTION SCALE: 3/32" = 1'-0"

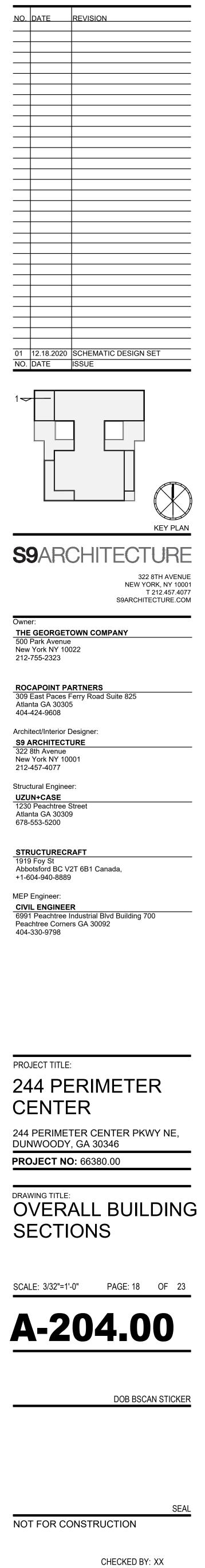
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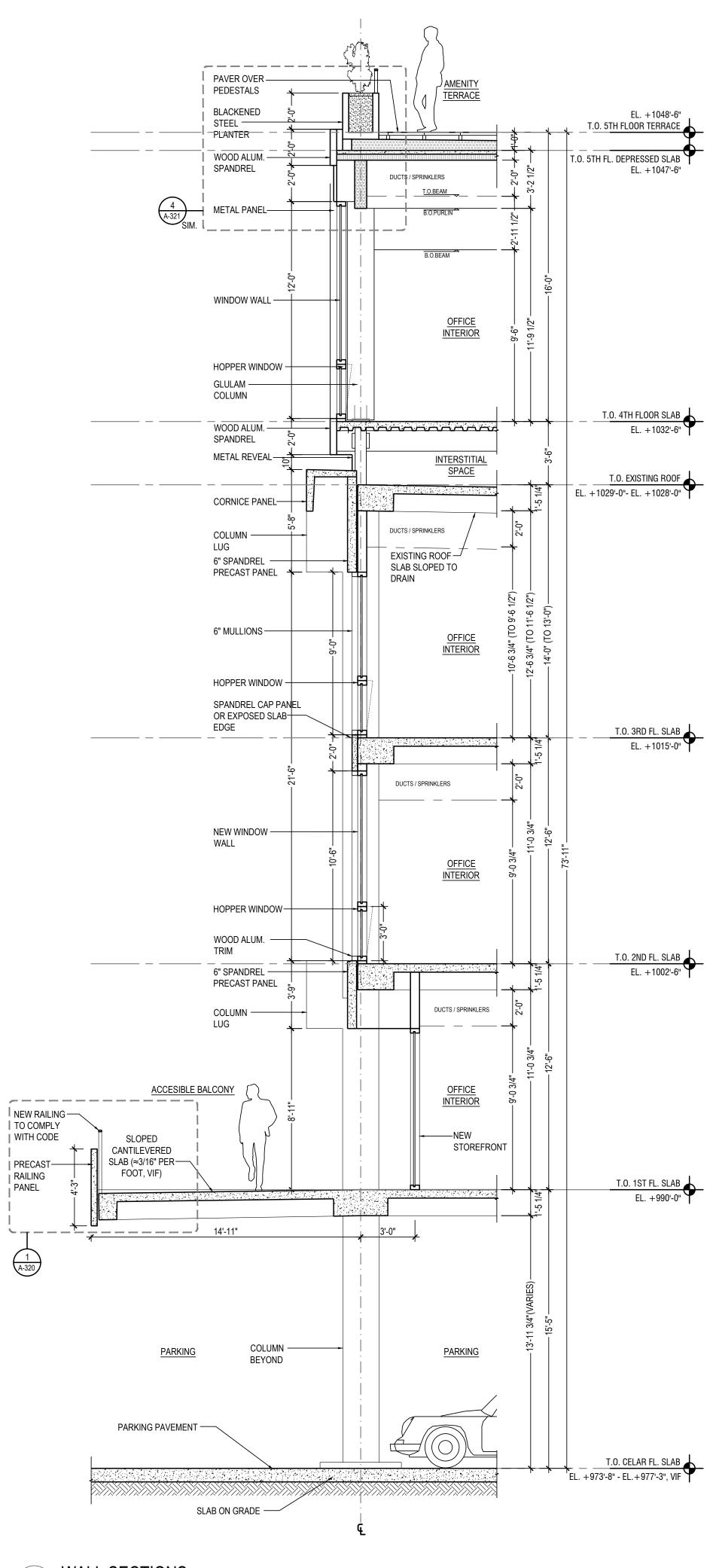
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		G		F	E
	30'-0"	30'-0"	<u>330'-0"</u> 30'-0"	30'-0"	
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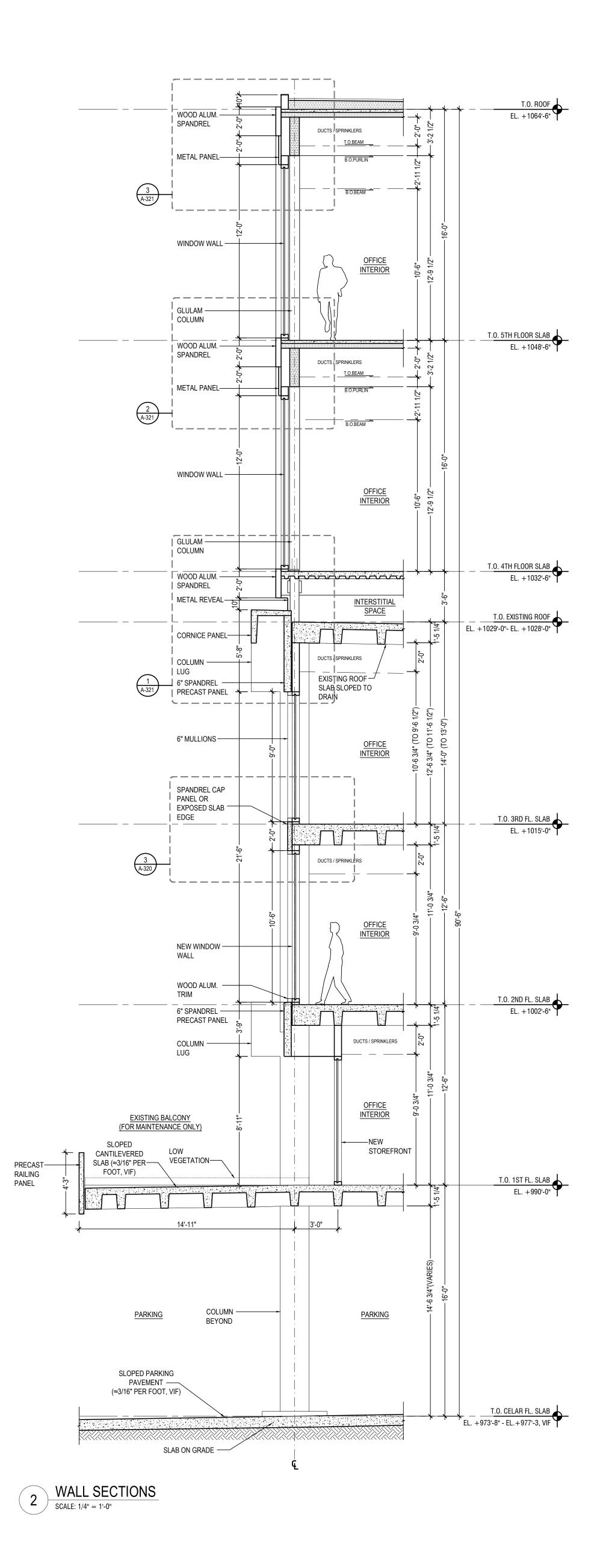




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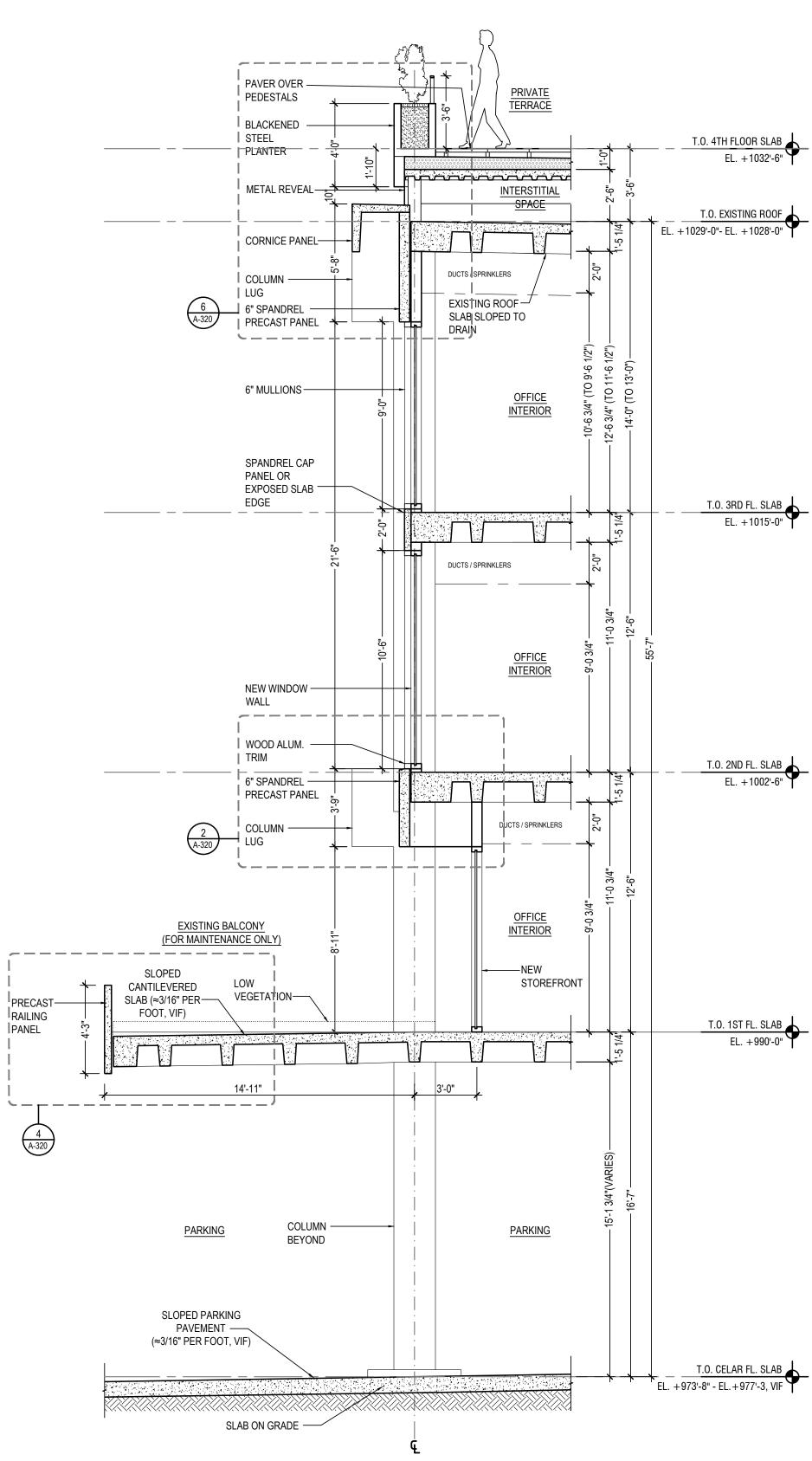


3 WALL SECTIONS SCALE: 1/4" = 1'-0"

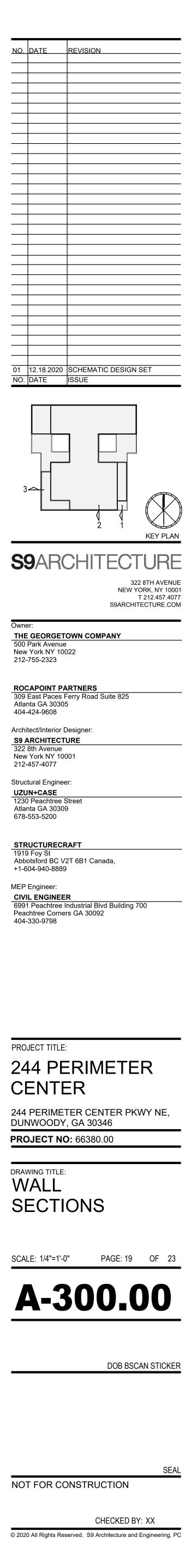


#### GENERAL NOTES

- . SEE STRUCTURAL DRAWINGS FOR DISTINCTION
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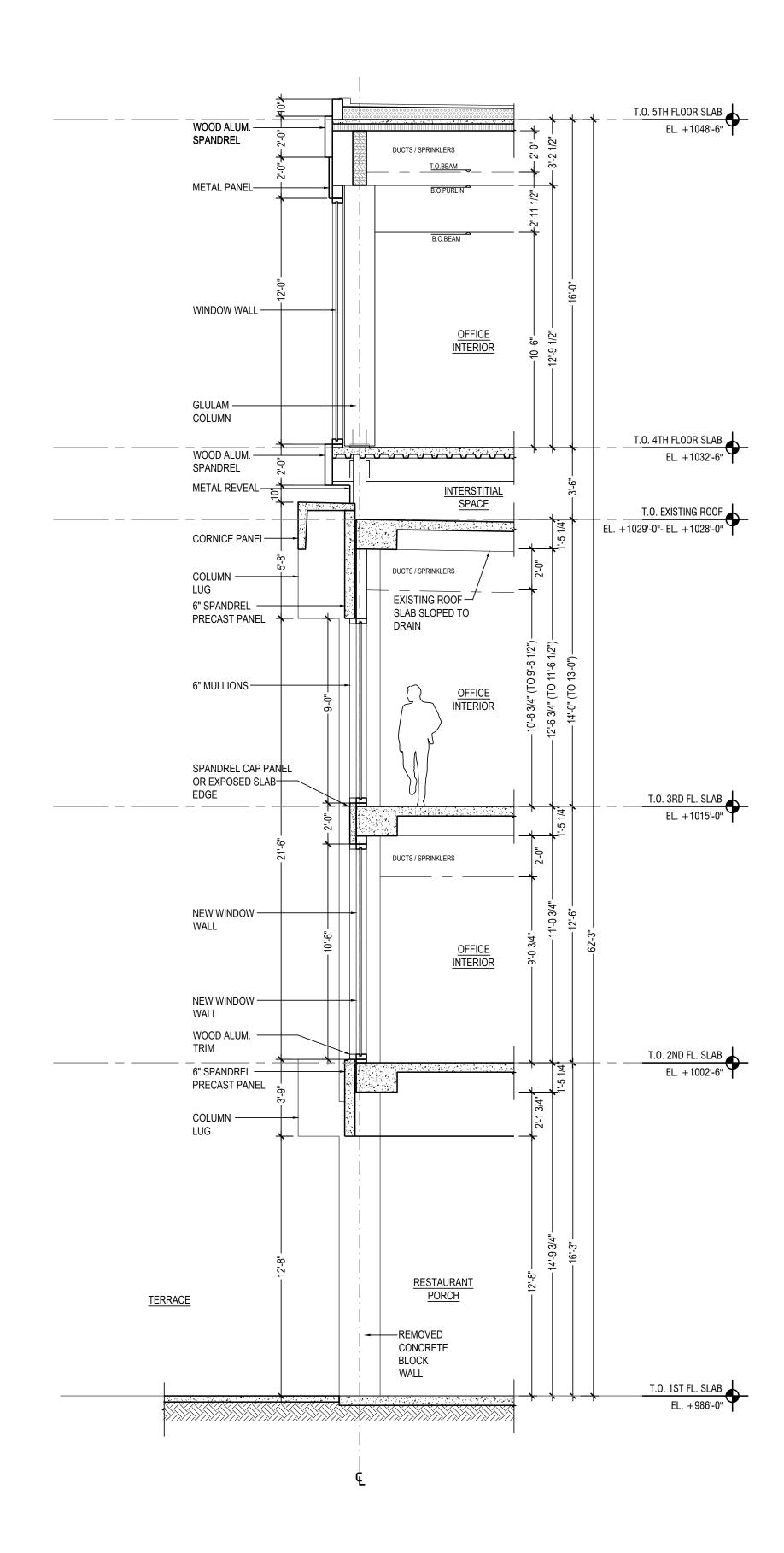


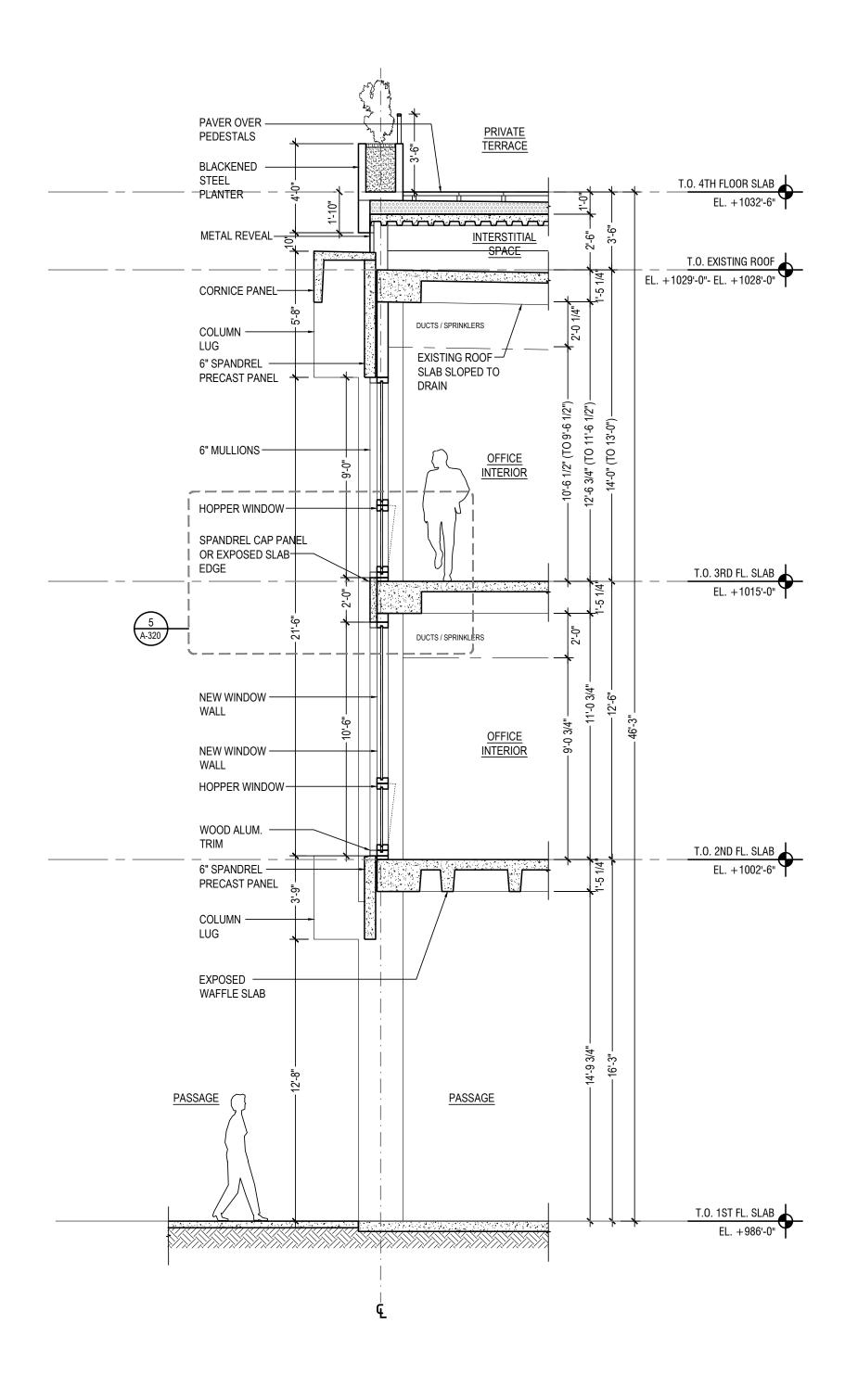
1 WALL SECTIONS SCALE: 1/4" = 1'-0"





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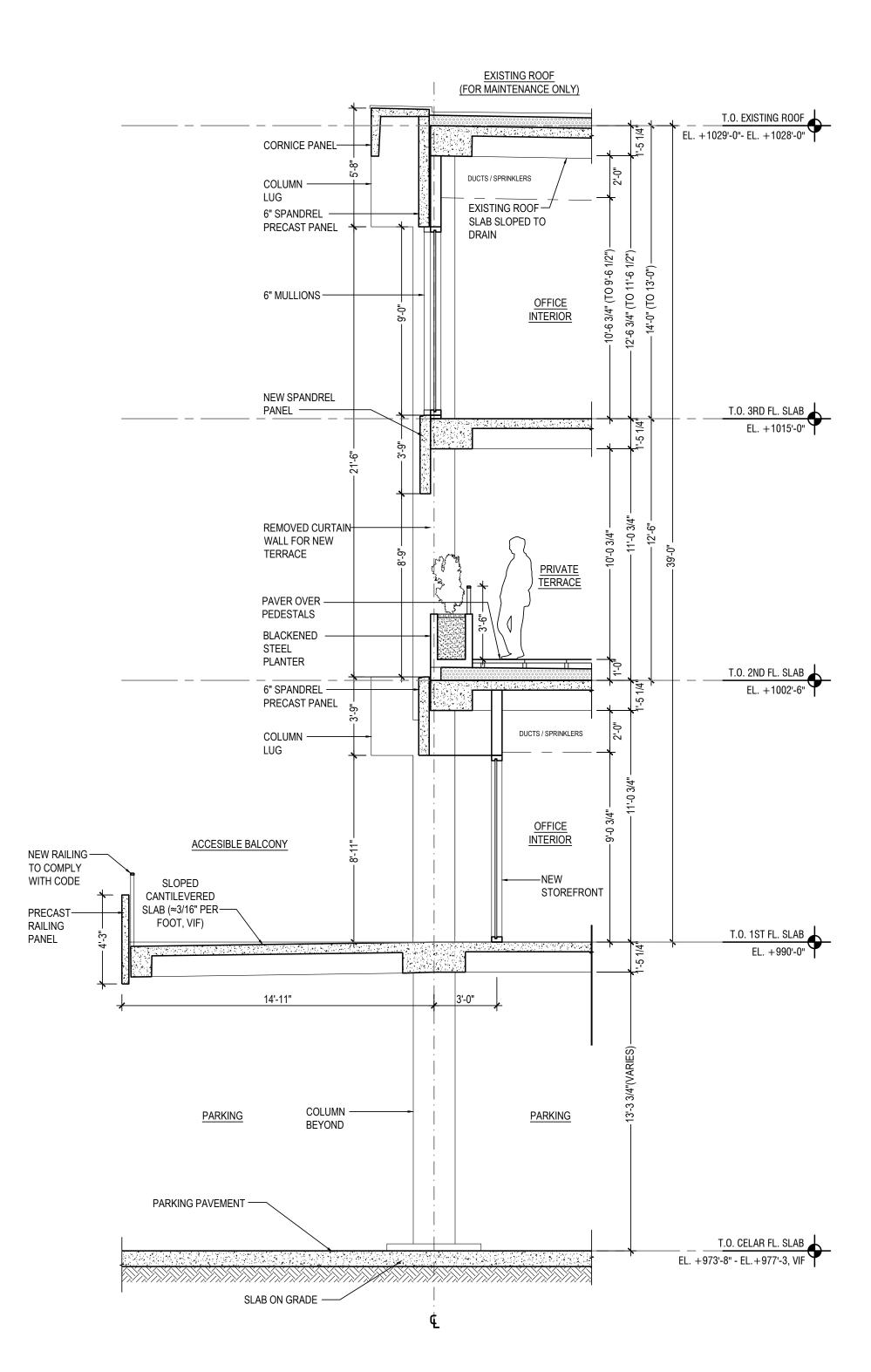




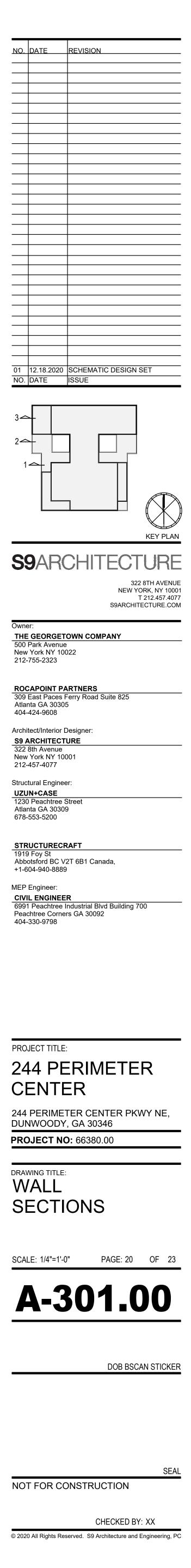
2 WALL SECTIONS SCALE: 1/4" = 1'-0"

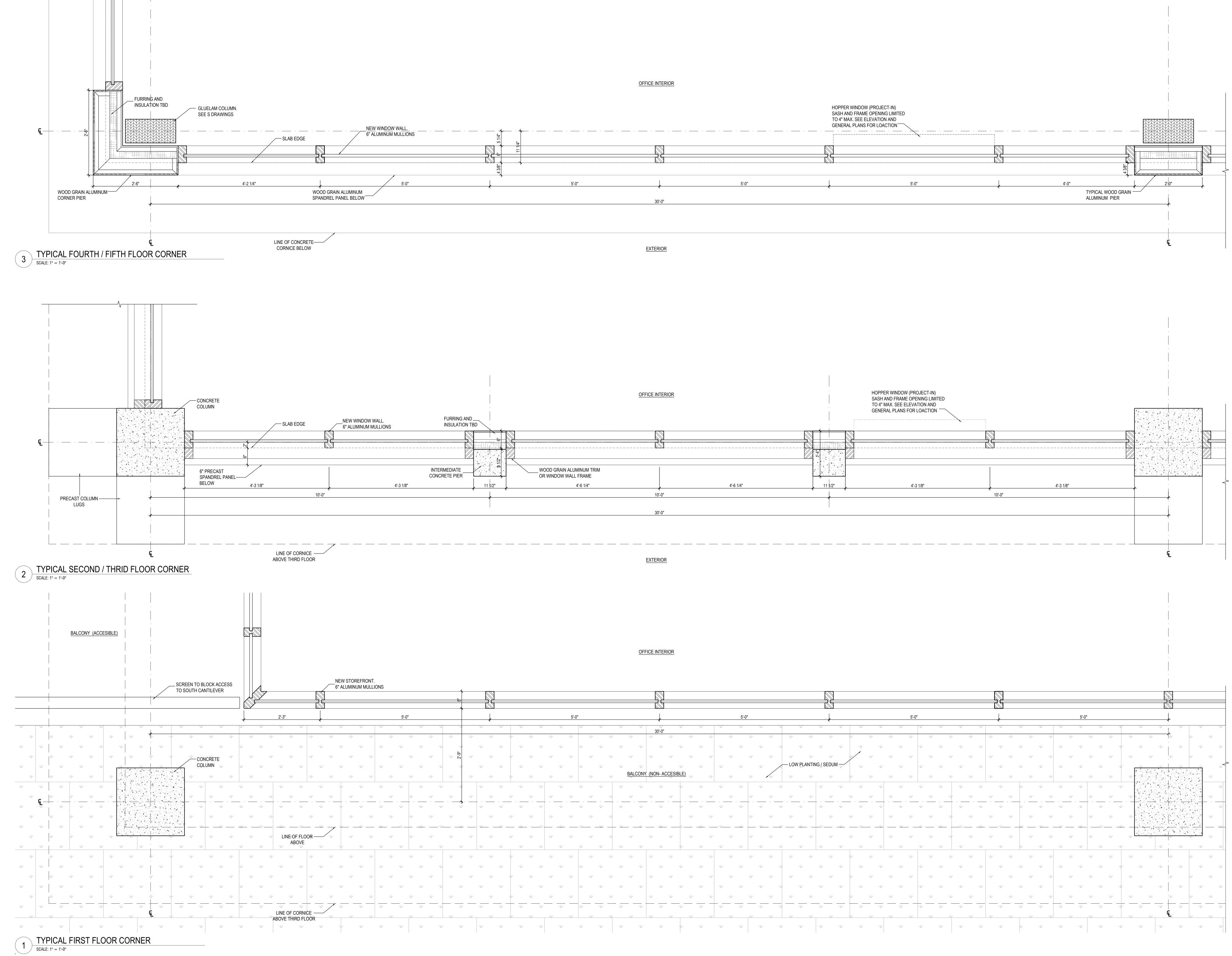
#### GENERAL NOTES

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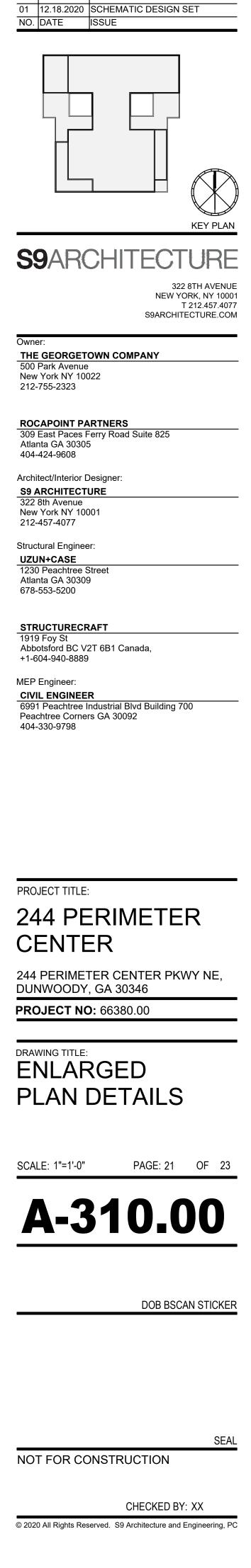






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		SASH TO 4'	PER WINDOW (PROJECT-IN) H AND FRAME OPENING LIMITED " MAX. SEE ELEVATION AND ERAL PLANS FOR LOACTION					
*	11 1/2"	1	4'-3 1/8"	10'-0"	 4'-3 1/8"		<u>.</u>	
	 				 		<u>-</u>	_
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NO. DATE

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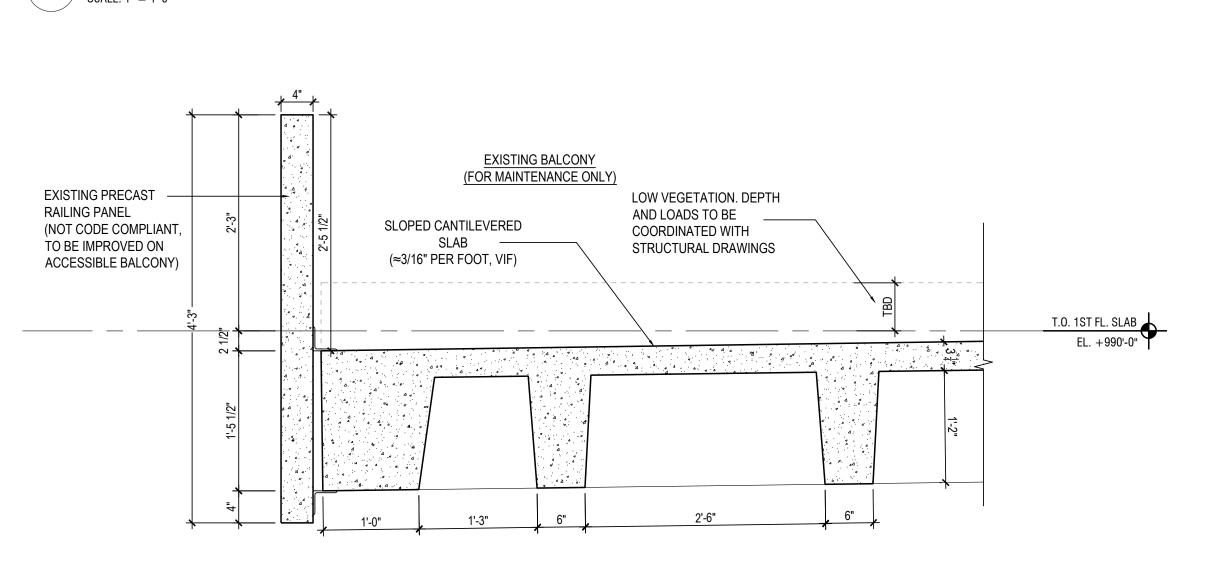
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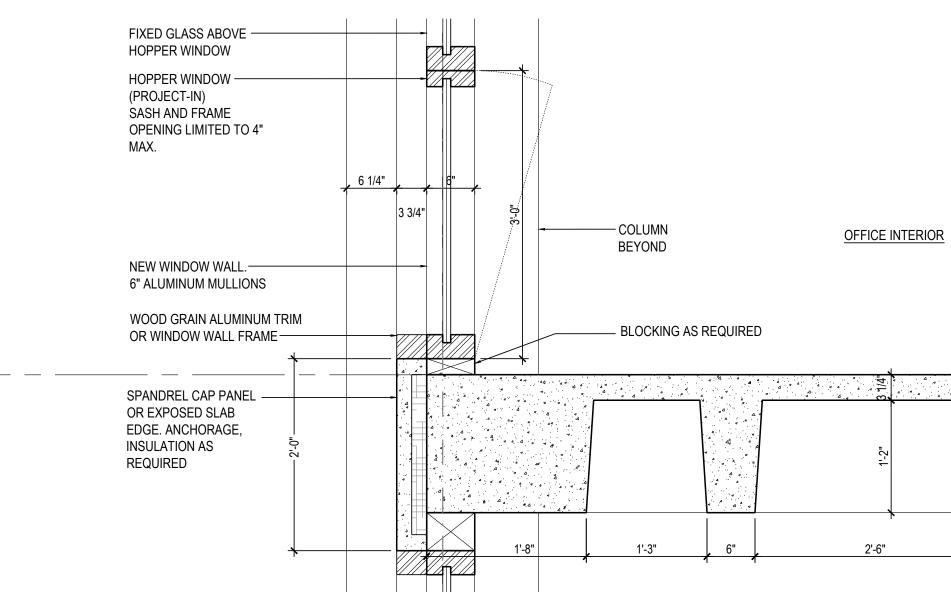
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4 TYPICAL FIRST FLOOR SOUTH CANTILEVER RAILING SCALE: 1" = 1'-0"

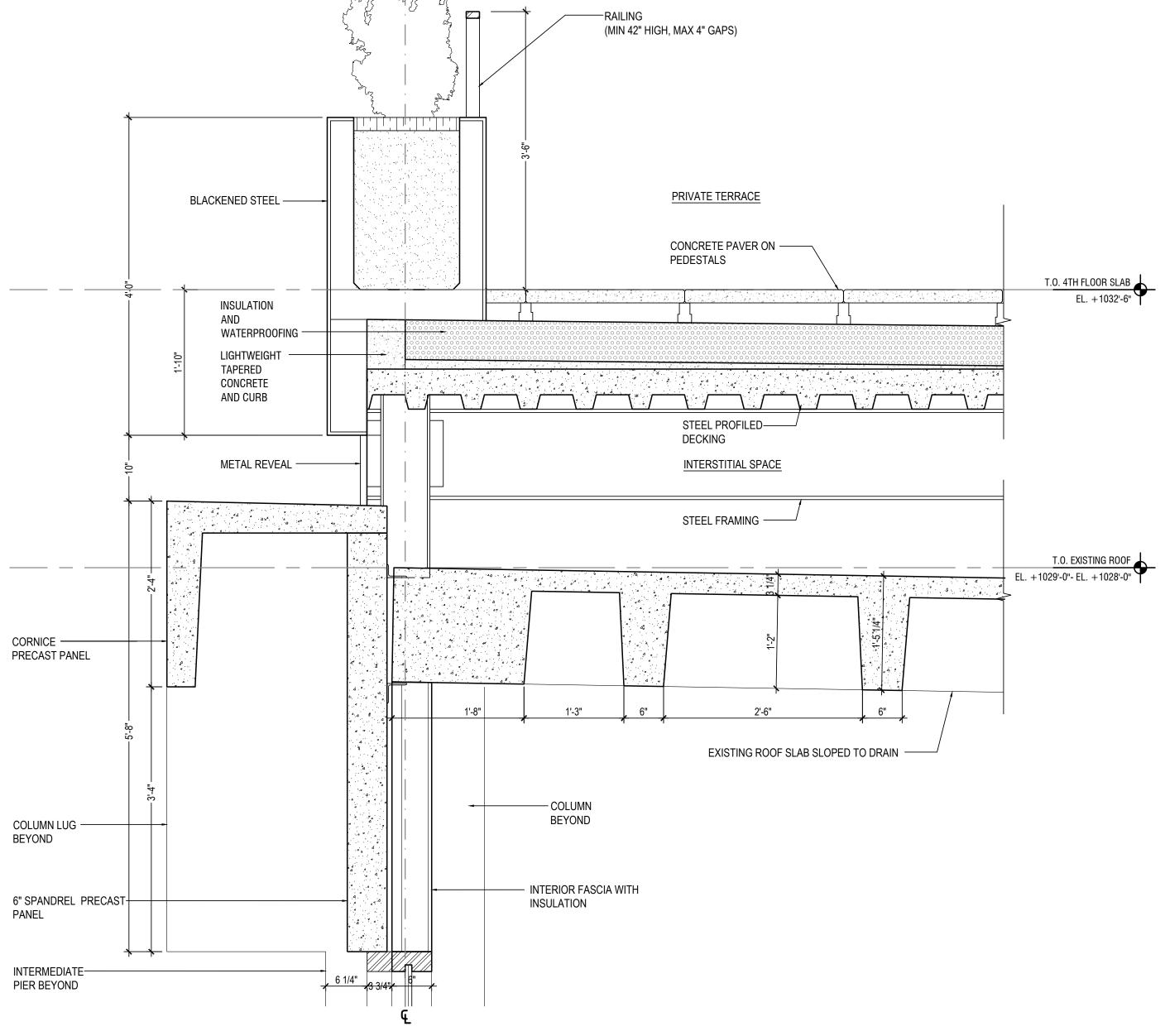




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## 6 TYPICAL CONCRETE CORNICE WITH TERRACE SCALE: 1" = 1'-0"



#### INTERMEDIATE -PIER BEYOND

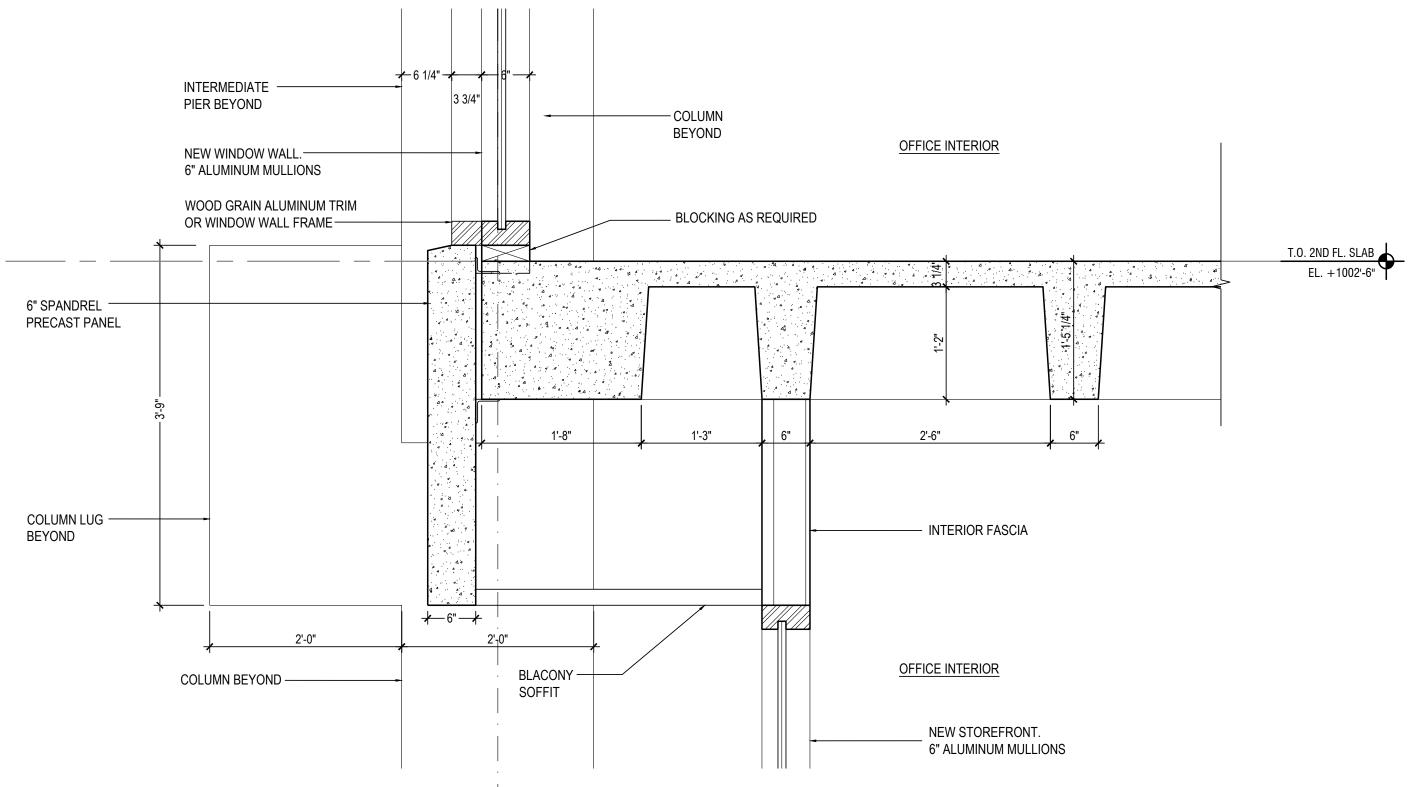
NEW WINDOW WALL.-**6" ALUMINUM MULLIONS** WOOD GRAIN ALUMINUM TRIM

SPANDREL CAP PANEL -OR EXPOSED SLAB EDGE. ANCHORAGE, INSULATION AS REQUIRED

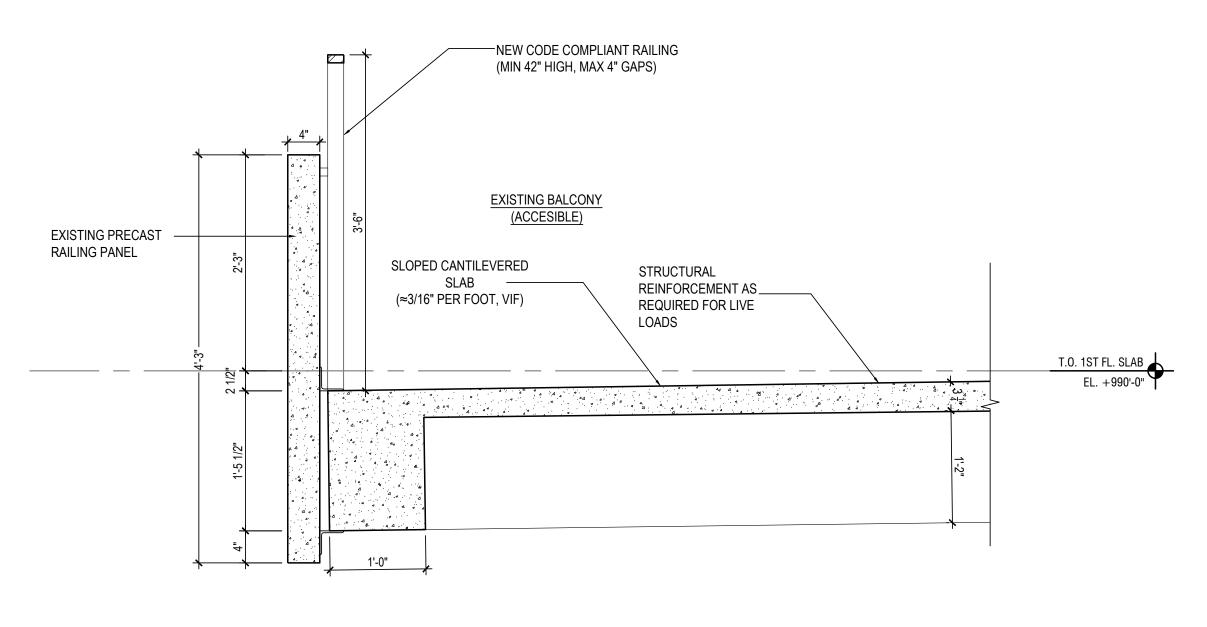
COLUMN BEYOND — NEW WINDOW WALL.-

#### TYPICAL SECOND FLOOR SPANDREL 3 / SCALE: 1" = 1'-0"

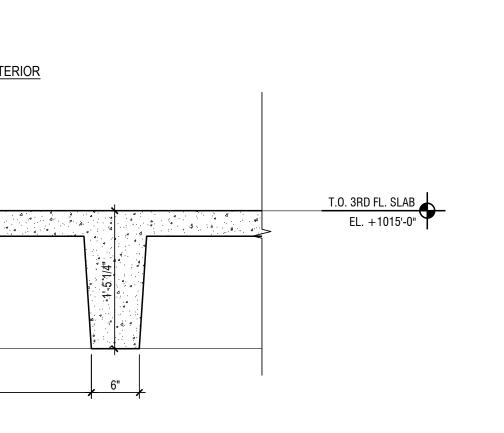
INTERMEDIATE -PIER BEYOND



## 2 TYPICAL SECOND FLOOR SPANDREL SCALE: 1" = 1'-0"



1 TYPICAL FIRST FLOOR WEST CANTILEVER RAILING SCALE: 1" = 1'-0"



#### GENERAL NOTES

SEE STRUCTURAL DRAWINGS FOR DISTINCTION BETWEEN EXISTING AND PROPOSED STRUCTURAL NO. DATE

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01 12.18.2020 SCHEMATIC DESIGN SET

**S9**ARCHITECTURE

THE GEORGETOWN COMPANY 500 Park Avenue

ROCAPOINT PARTNERS 309 East Paces Ferry Road Suite 825 Atlanta GA 30305 404-424-9608

New York NY 10022

Architect/Interior Designer:

S9 ARCHITECTURE 322 8th Avenue

New York NY 10001 212-457-4077

Structural Engineer:

Atlanta GA 30309 678-553-5200

UZUN+CASE 1230 Peachtree Street

STRUCTURECRAFT 1919 Foy St

MEP Engineer:

PROJECT TITLE:

CENTER

PROJECT NO: 66380.00

DRAWING TITLE: ENLARGED

Abbotsford BC V2T 6B1 Canada, +1-604-940-8889

Peachtree Corners GA 30092 404-330-9798

**CIVIL ENGINEER** 6991 Peachtree Industrial Blvd Building 700

244 PERIMETER

244 PERIMETER CENTER PKWY NE, DUNWOODY, GA 30346

SECTION DETAILS

SCALE: 1"=1'-0" PAGE: 22 OF 23

A-320.00

DOB BSCAN STICKER

212-755-2323

KEY PLAN

322 8TH AVENUE NEW YORK, NY 10001

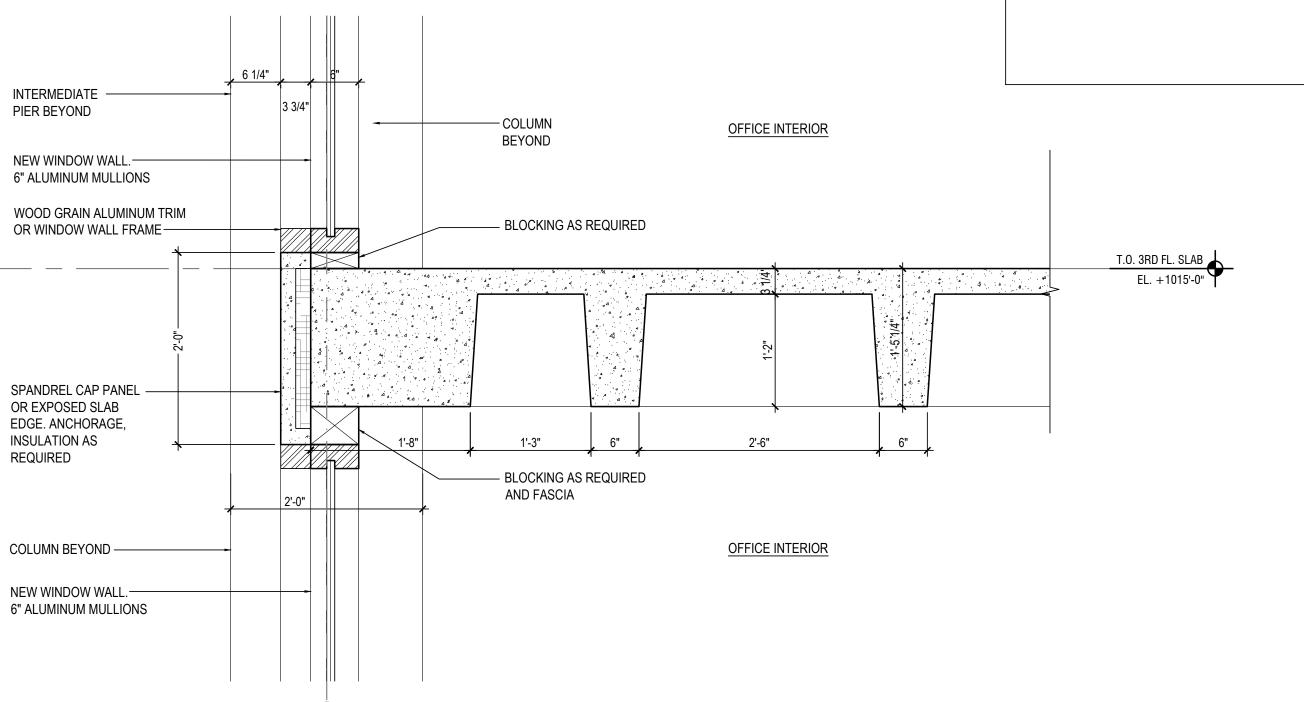
T 212.457.4077 S9ARCHITECTURE.COM

NO. DATE ISSUE

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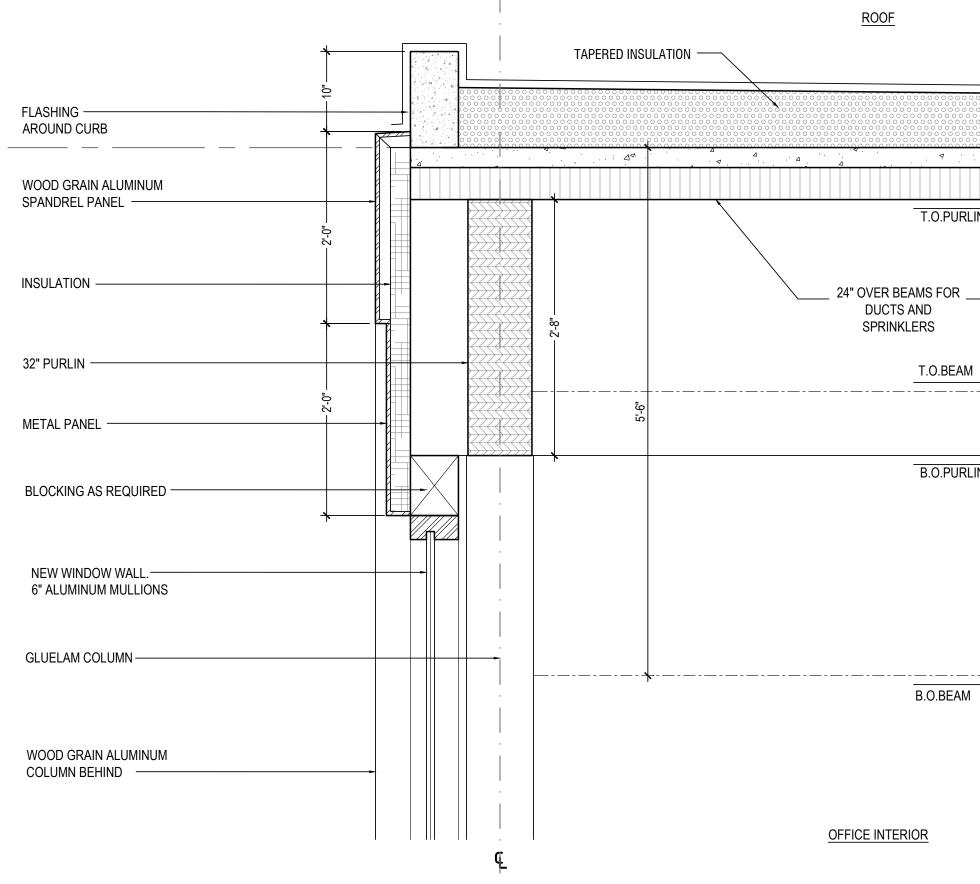
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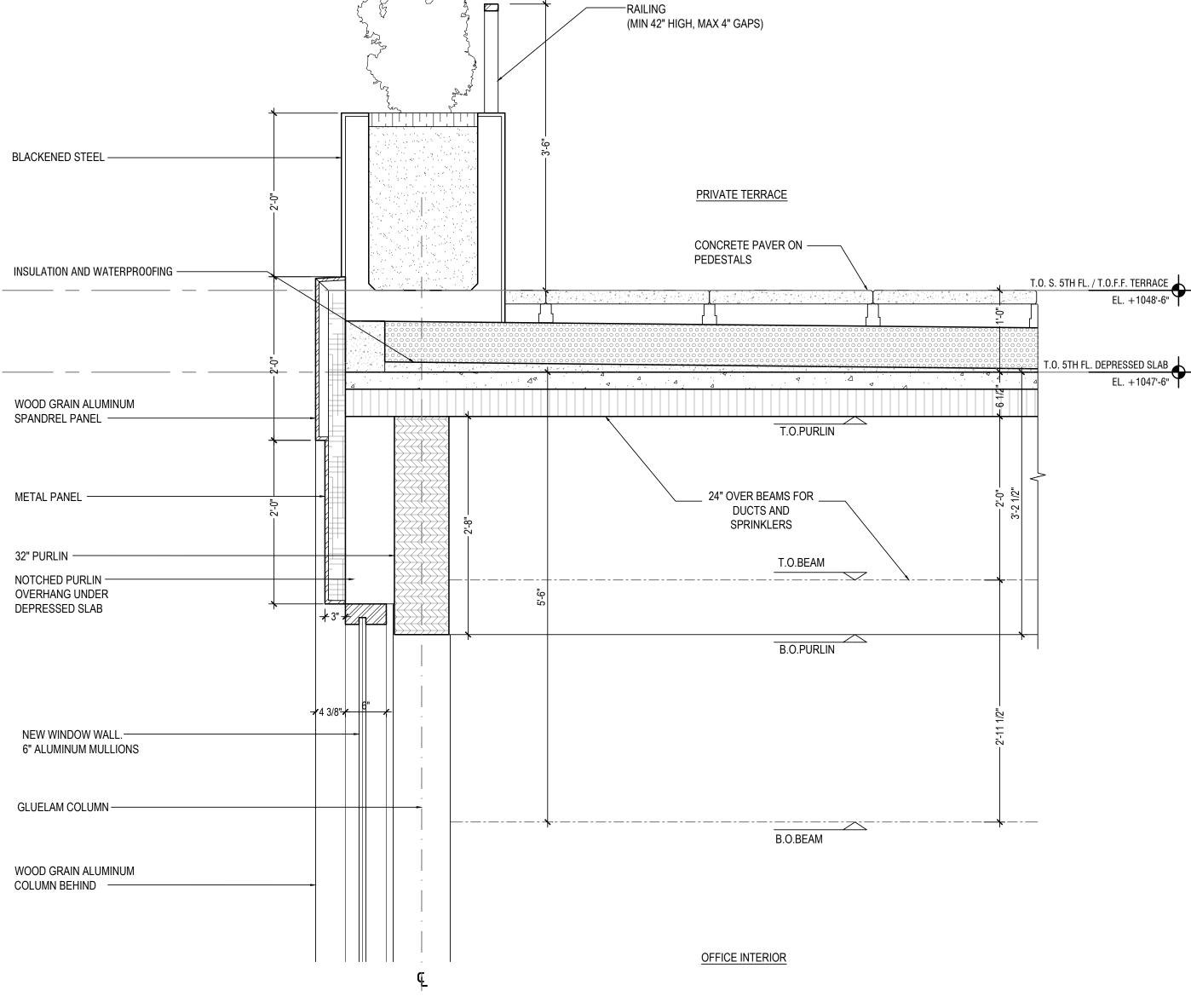
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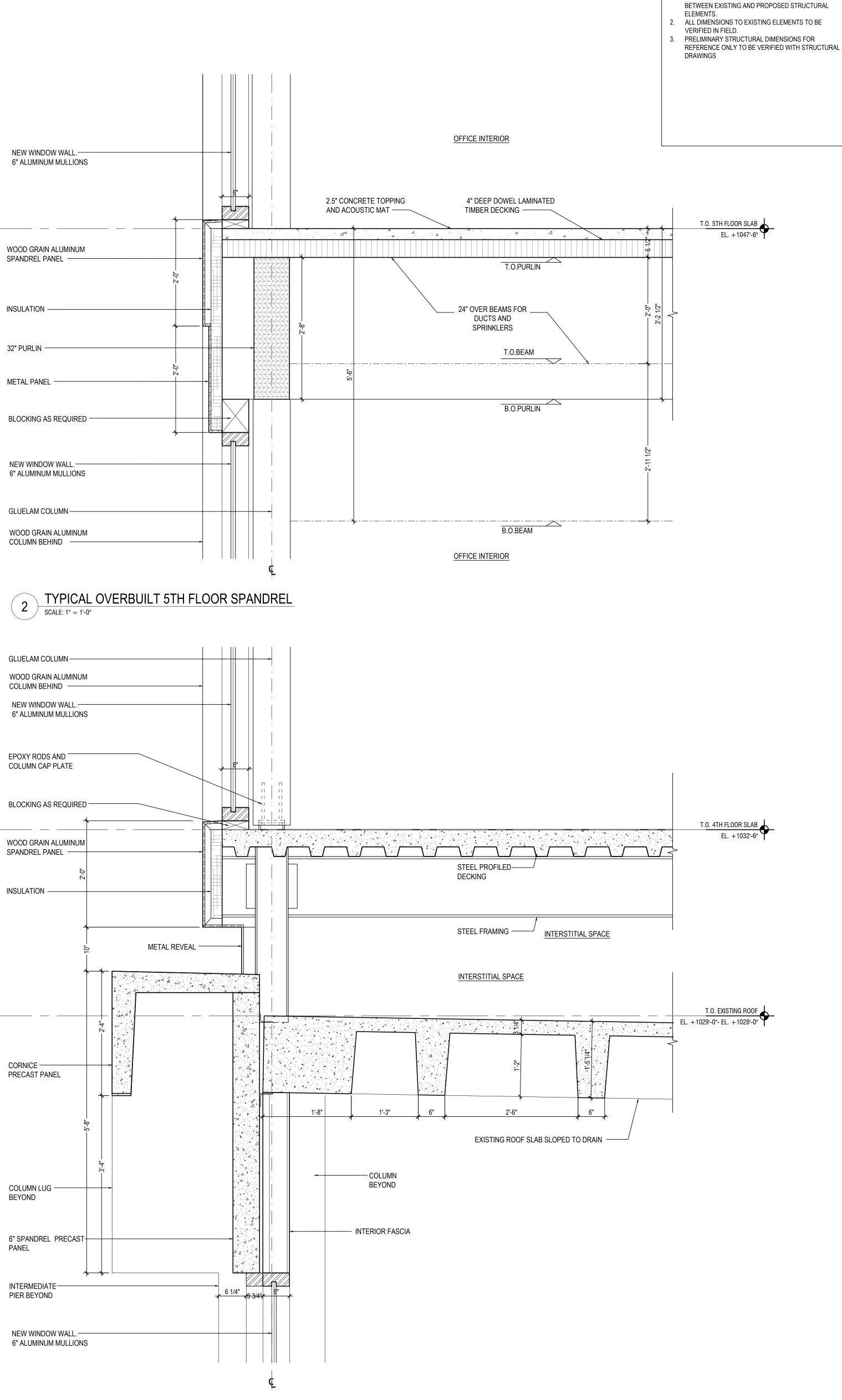




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OOD GRAIN ALUMINUM PANDREL PANEL		
SULATION	"0 .c	0- 7
2" PURLIN	•	
ETAL PANEL	"0 ic	- 7 <u>-</u>
LOCKING AS REQUIRED	-	_
IEW WINDOW WALL. " ALUMINUM MULLIONS		
VOOD GRAIN ALUMINUM		

## SCALE: 1" = 1'-0"

GLUELAM COLUMN	
Wood Grain Aluminum Column Behind	
NEW WINDOW WALL.	
6" ALUMINUM MULLIONS	



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FOR	2.0"	
D.BEAM	<b>_</b>	
D.PURLIN		
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© 2020 All Rights Reserved. S9 Architecture and Engineer	ring, PC

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01 12.18.2020 SCHEMATIC DESIGN SET

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New York NY 10001 212-457-4077

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Atlanta GA 30309 678-553-5200

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PROJECT TITLE:

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**PROJECT NO:** 66380.00

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Abbotsford BC V2T 6B1 Canada,

Peachtree Corners GA 30092 404-330-9798

**CIVIL ENGINEER** 6991 Peachtree Industrial Blvd Building 700

244 PERIMETER

244 PERIMETER CENTER PKWY NE, DUNWOODY, GA 30346

SECTION DETAILS

SCALE: 1"=1'-0" PAGE: 23 OF 23

A-321.00

DOB BSCAN STICKER

KEY PLAN

322 8TH AVENUE NEW YORK, NY 10001

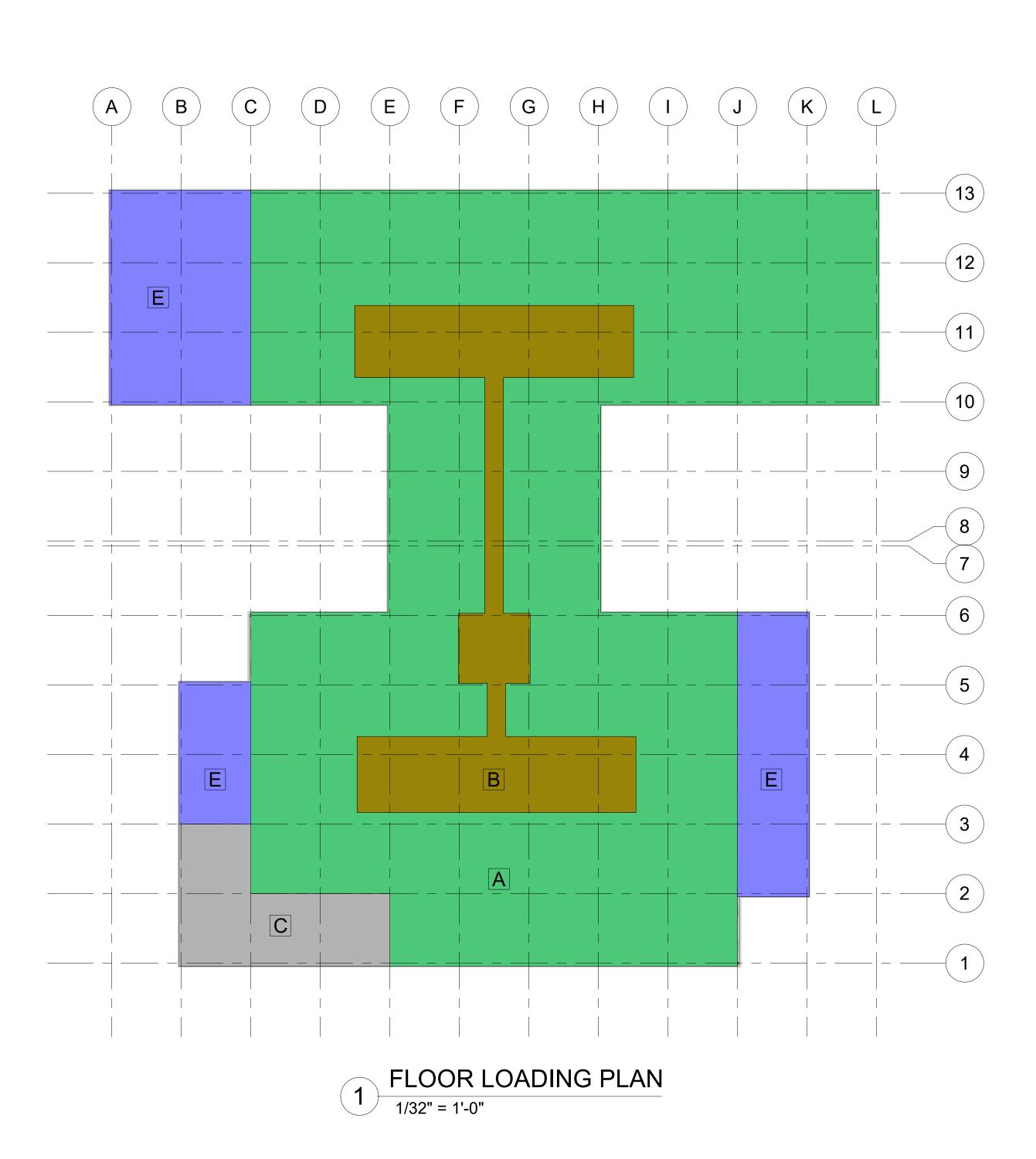
T 212.457.4077 S9ARCHITECTURE.COM

NO. DATE ISSUE

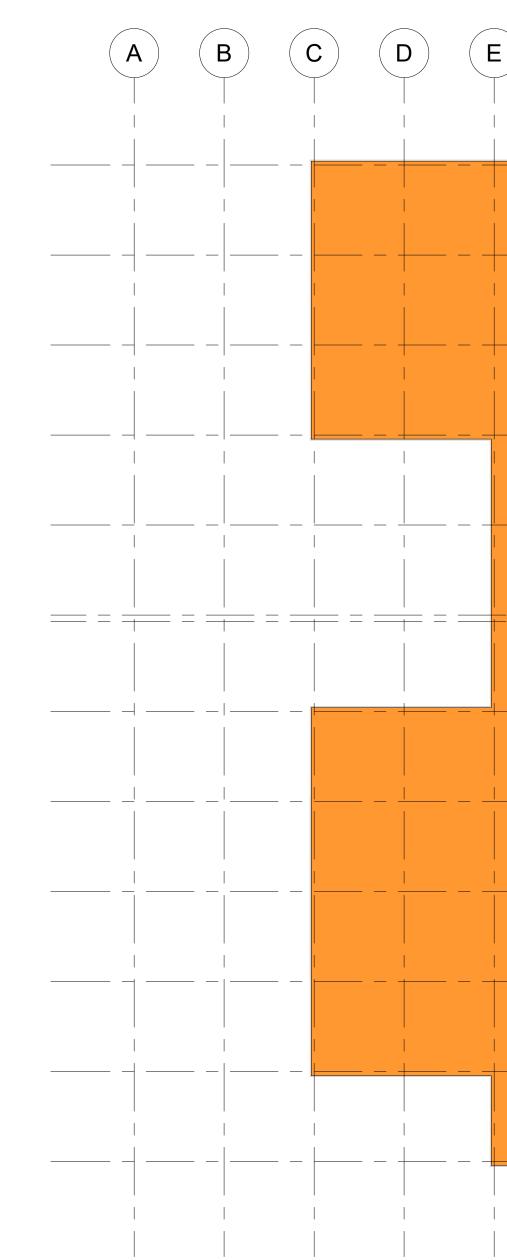
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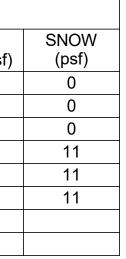
GENERAL NOTES

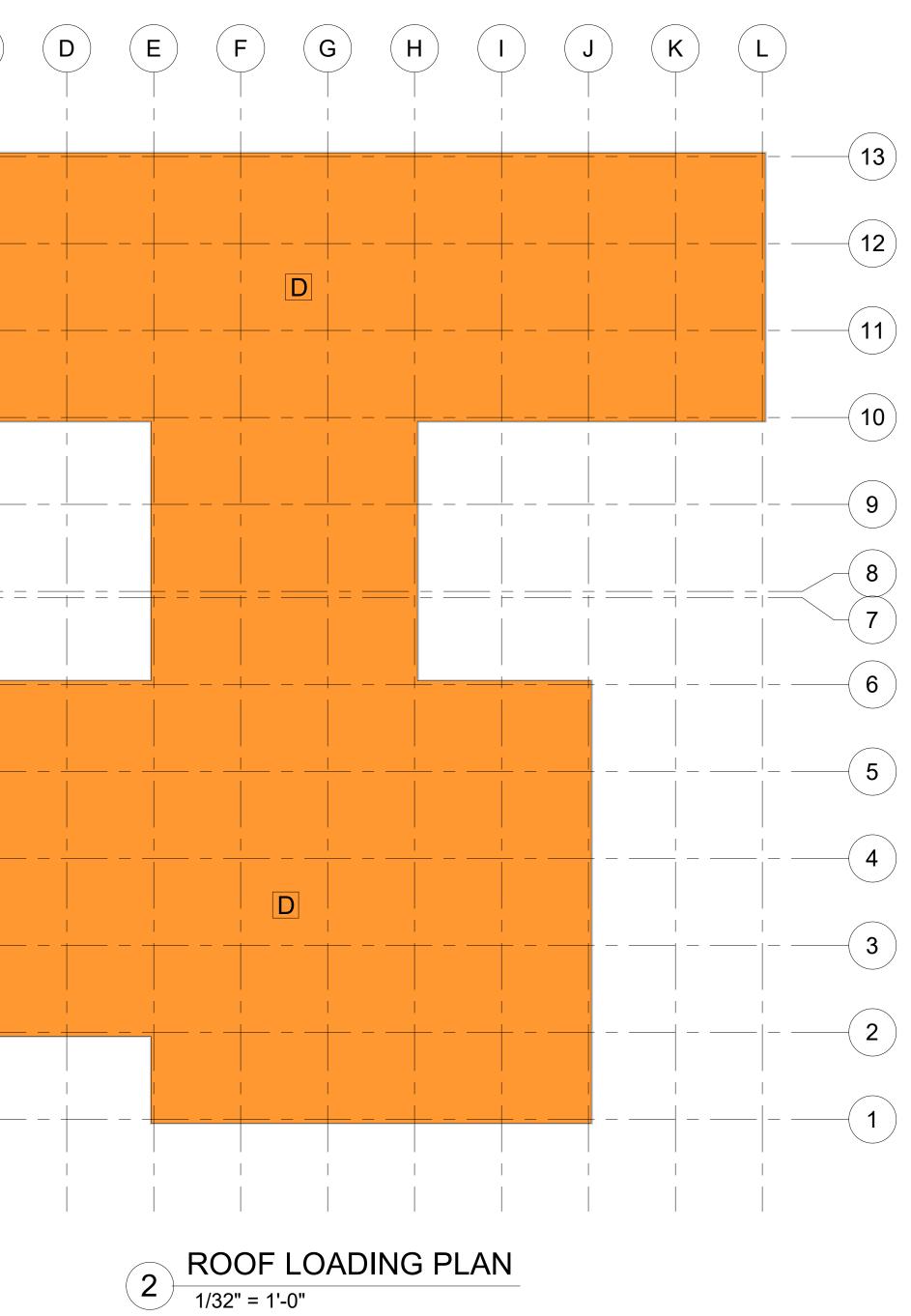
SEE STRUCTURAL DRAWINGS FOR DISTINCTION



LOAD SCHEDULE					
MARK	LOAD SOURCE	SDL (psf)	DL (psf)	Σ DEAD (psf)	LIVE (pst
Α	OFFICE	8	55	63	65
В	CORRIDOR	8	55	63	80
С	TERRACE	30	17	47	75
D	ROOF	10	17	27	20
E	GREEN ROOF	30	17	47	20
F	MECH ROOF	130	97	227	140.0

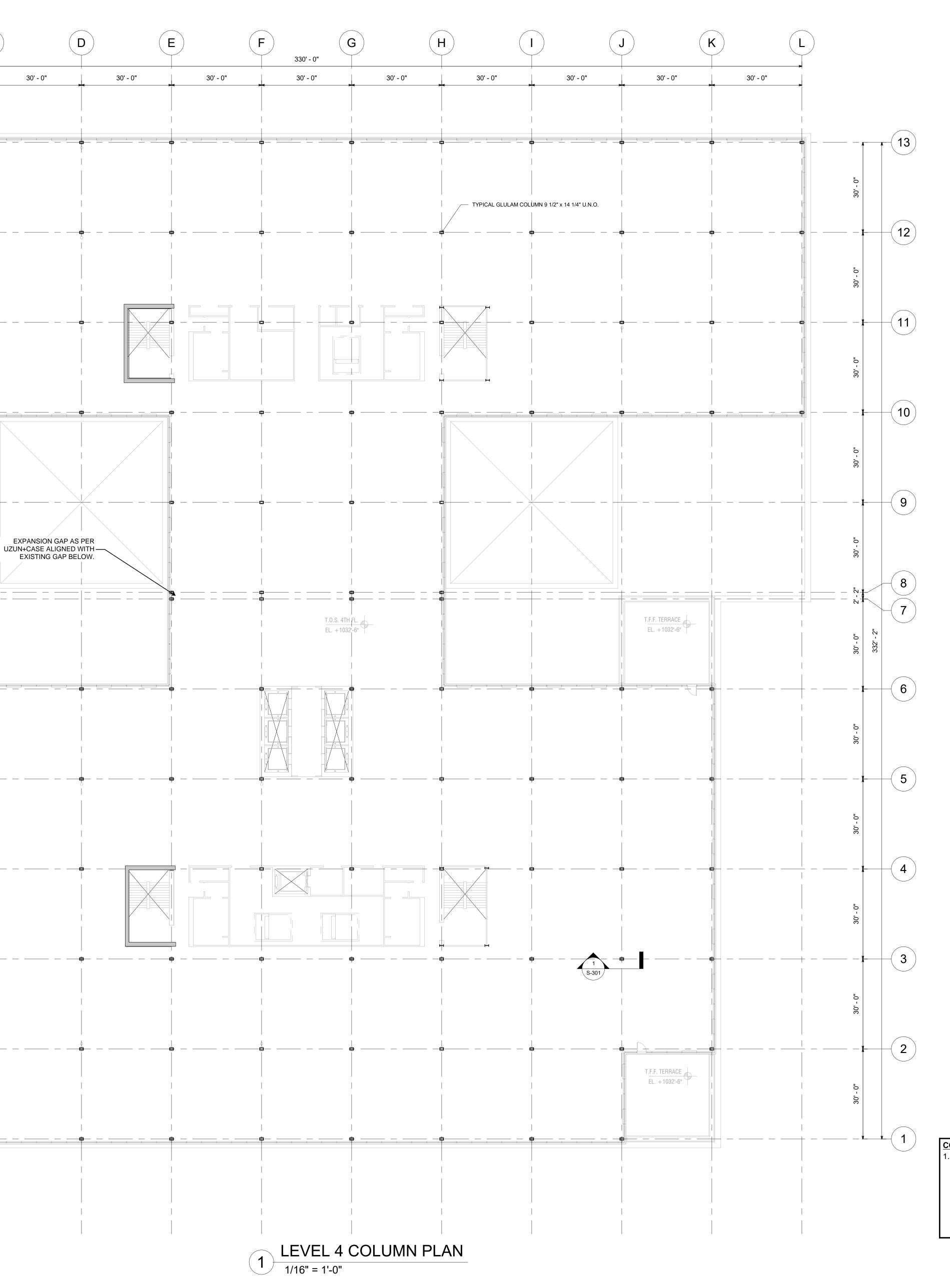






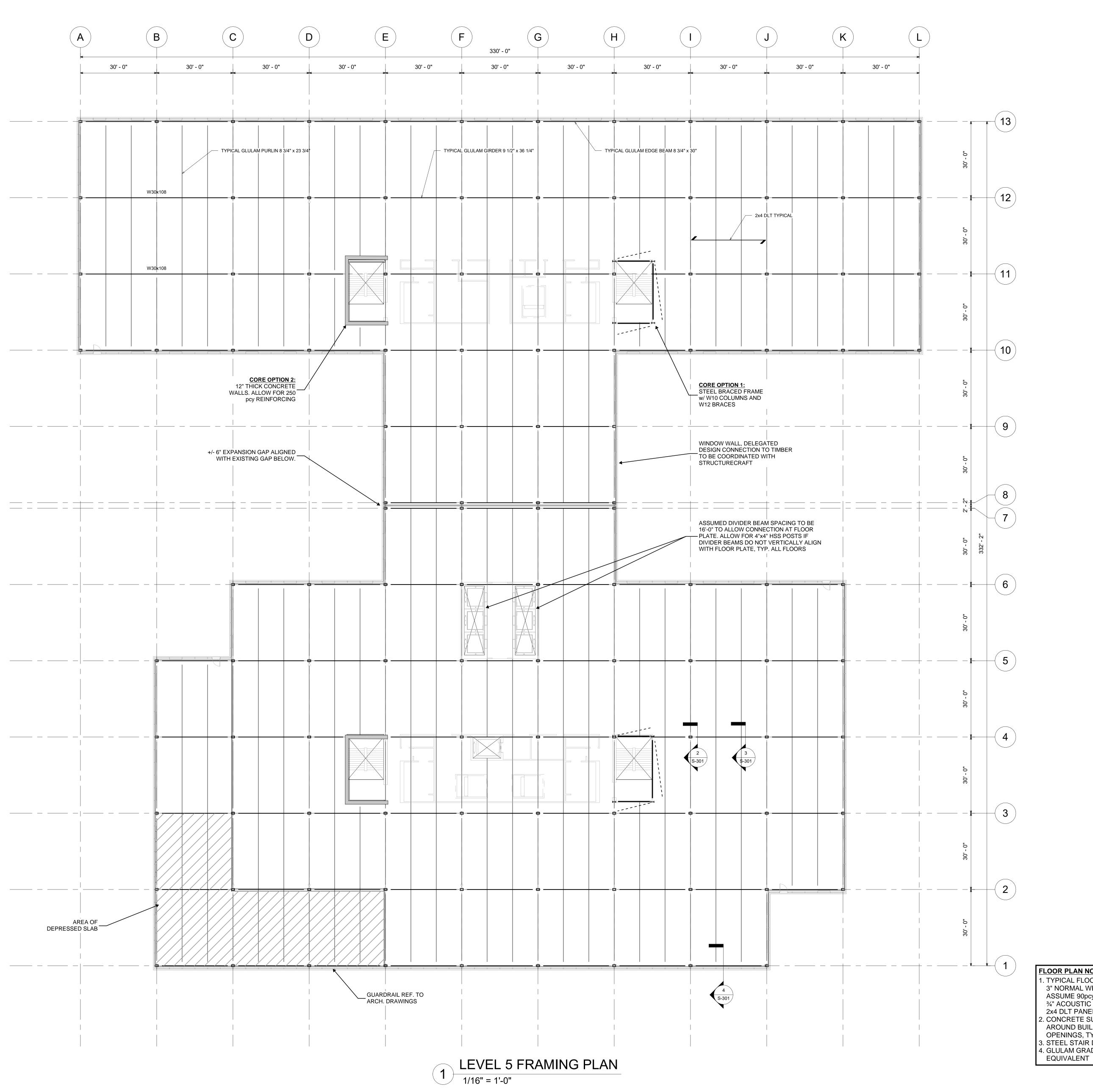
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	raft
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ARCHITECT S9 ARCHITECTURE	
CLIENT THE GEORGETOWN COMPANY ROCAPOINT PARTNERS PROJECT	
244 PERIMETER OFFICE	
LOADING PLAN	
CHECKED: EW	379
DATE: 20-12-18	
S-010	

A	B	С
<b>30' - 0</b> "	30' - 0"	
		- <b>-</b> -
T.F.F. TERRACE EL. +1032'-6"		
		EXI UZUN- E
	T.F.F. TERRACE EL. +1032'-6"	
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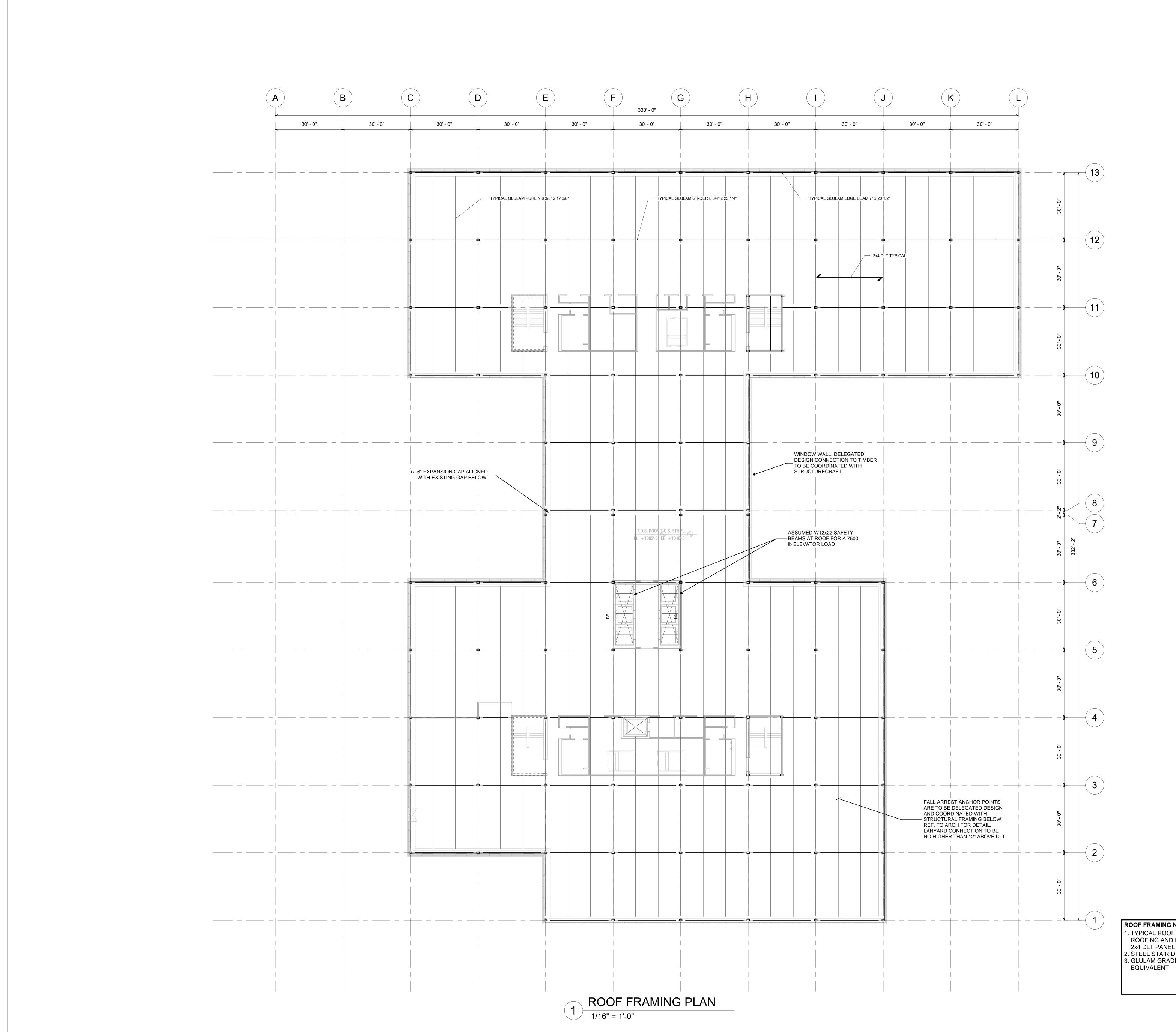
<u>COLUMN NOTES:</u> 1. GLULAM GRADE TO BE 24F-V8, DF L2 OR APPROVED EQUIVALENT

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S9 ARCHITECTURE	_
ROCAPOINT PARTNERS PROJECT 244 PERIMETER	_
OFFICE	
TITLE	
LEVEL 4 COLUMN PLAN	
DRAWN: BOG JOB No. S379 CHECKED: EW	_
DATE: 20-12-18	_
S-104	



FLOOR PLAN NOTES:
1. TYPICAL FLOOR BUILDUP:
3" NORMAL WEIGHT CONCRETE TOPPING OVER
ASSUME 90pcy OF REINFORCING
3/4" ACOUSTIC MAT
2x4 DLT PANEL w/ 1/2" SHEATHING
2. CONCRETE SUPPLIER TO PROVIDE LIGHT GAUGE POUR STOP
AROUND BUILDING PERIMETER AND AROUND INTERIOR
OPENINGS, TYP.
3. STEEL STAIR DELEGATED DESIGN TYP. ALL FLOORS
4. GLULAM GRADE TO BE 24F-V8, DF L2 OR APPROVED

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Do not scale from drawings; always read written dimensions.          ARCHITECT         S9 ARCHITECTURE         CLIENT         THE GEORGETOWN COMPANY         ROCAPOINT PARTNERS         PROJECT         244 PERIMETER         OFFICE         TITLE         LEVEL 5 FRAMING
Do not scale from drawings; always read written dimensions.          ARCHITECT         S9 ARCHITECTURE         CLIENT         THE GEORGETOWN COMPANY         ROCAPOINT PARTNERS         PROJECT         244 PERIMETER         OFFICE         TITLE
Do not scale from drawings; always read written dimensions.          ARCHITECT         S9 ARCHITECTURE         CLIENT         THE GEORGETOWN COMPANY         ROCAPOINT PARTNERS         PROJECT         244 PERIMETER         OFFICE         TITLE         LEVEL 5 FRAMING
ARCHITECT S9 ARCHITECTURE CLIENT THE GEORGETOWN COMPANY ROCAPOINT PARTNERS PROJECT 244 PERIMETER OFFICE TITLE LEVEL 5 FRAMING PLAN
Do not scale from drawings; always read written dimensions.          ARCHITECT         S9 ARCHITECTURE         CLIENT         THE GEORGETOWN COMPANY ROCAPOINT PARTNERS         PROJECT         244 PERIMETER OFFICE         TITLE         LEVEL 5 FRAMING PLAN         DRAWN:       BOG         LOB NO.       \$379         CHECKED:       EW
Do not scale from drawings; always read written dimensions.
Do not scale from drawings; always read written dimensions.          ARCHITECT         S9 ARCHITECTURE         CLIENT         THE GEORGETOWN COMPANY ROCAPOINT PARTNERS         PROJECT         244 PERIMETER OFFICE         TITLE         LEVEL 5 FRAMING PLAN         DRAWN:       BOG         LOB NO.       \$379         CHECKED:       EW

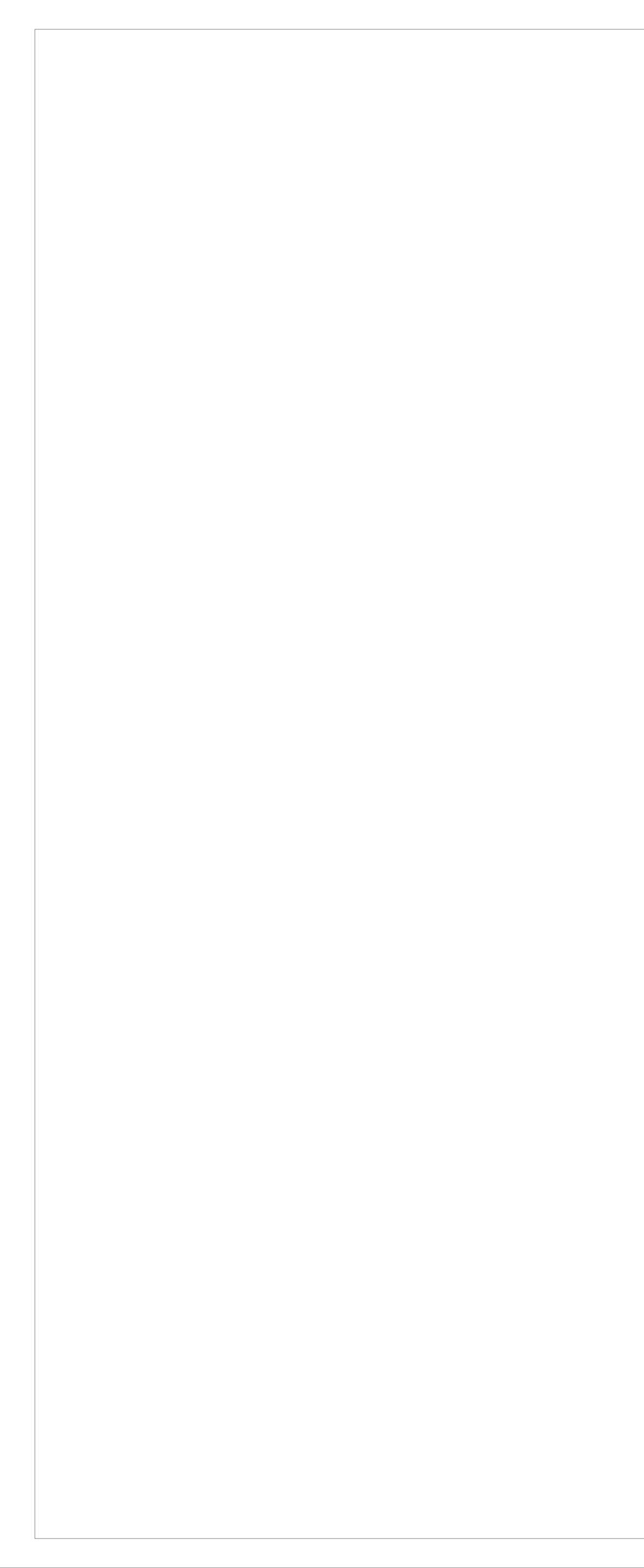


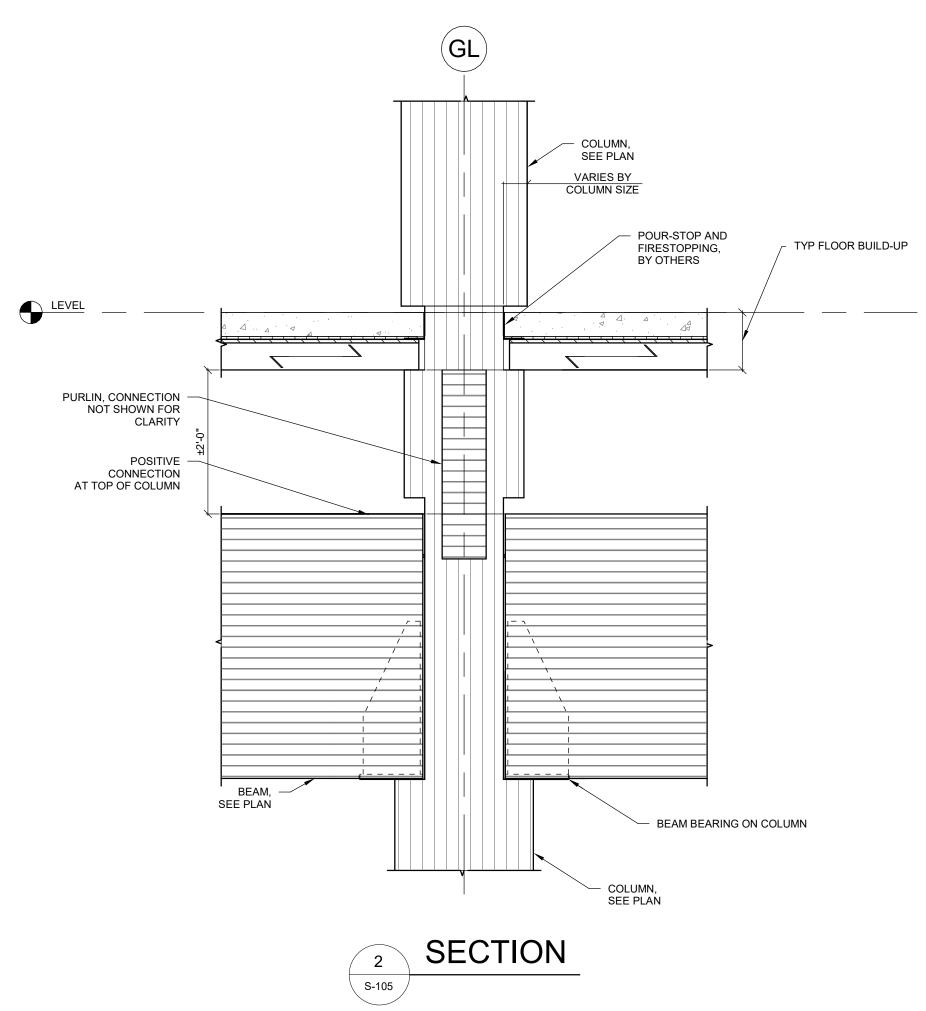
**ROOF FRAMING NOTES:** 

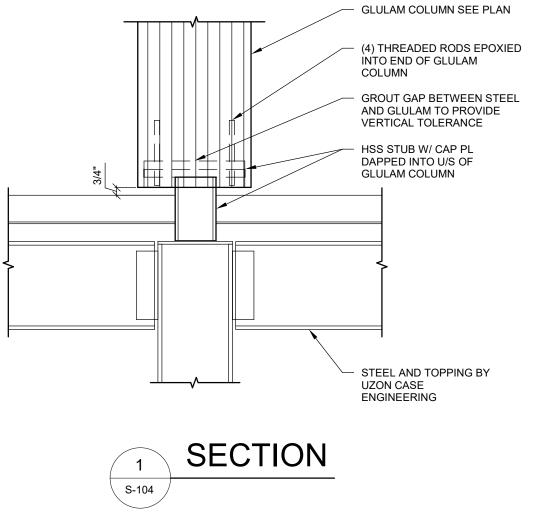
1. TYPICAL ROOF BUILDUP: ROOFING AND INSULATION AS PER ARCH

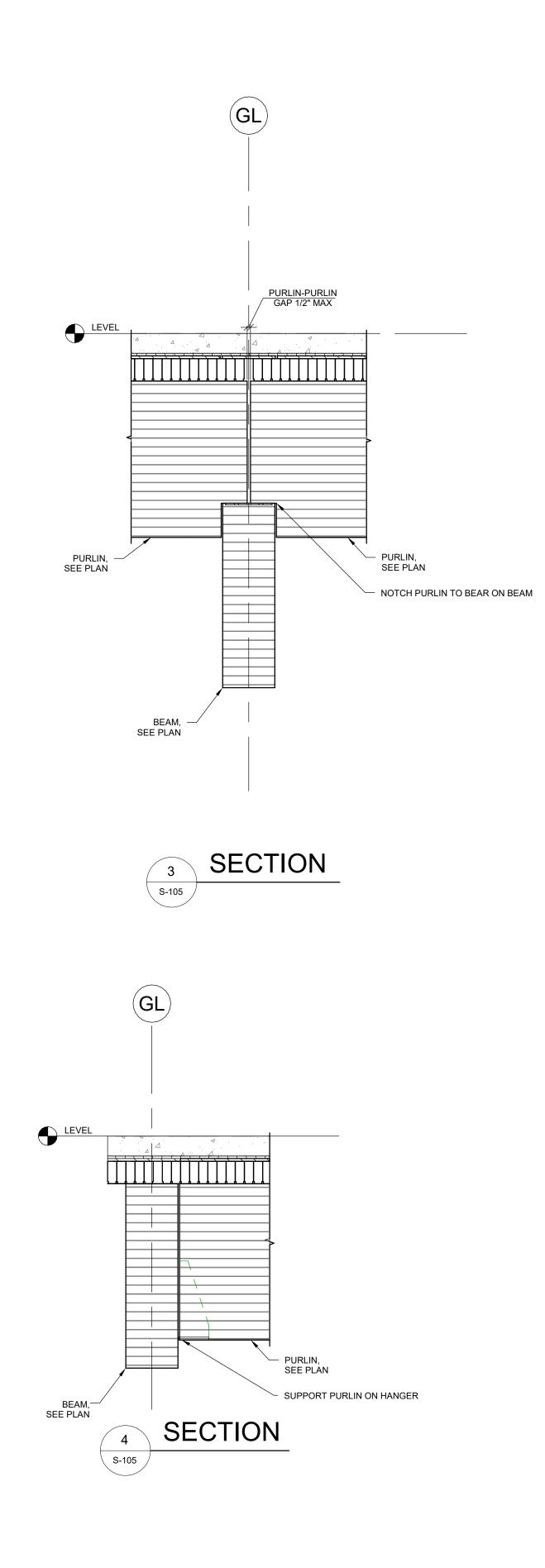
2x4 DLT PANEL w/ 1/2" SHEATHING 2. STEEL STAIR DELEGATED DESIGN TYP. ALL FLOORS 3. GLULAM GRADE TO BE 24F-V8, DF L2 OR APPROVED

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PROJECT 244 PERIMETER
OFFICE
DRAWN: BOG JOB No. 0070
DRAWN:     BOG     JOB No.     S379       CHECKED:     EW     REVISION       DATE:     20-12-18
S-301

ſ				1.00	GENERAL
	٨	ABBREVI ADD'L	ATIONS ADDITIONAL	1.01	ALL CONSTRUCTION SHALL CONFORM TO THE 2018 INTERNATIONAL BUILDING CODE
	~	ADJ. A/E	ADJACENT ARCHITECT/ENGINEER		WITH THE CURRENT GEORGIA STATE AMENDMENTS. REFERENCE TO OTHER STANDARD SPECIFICATIONS OR CODES SHALL MEAN THE BUILDING CODE ADOPTED EDITION OR THE
		ALT. ARCH.	ALTERNATE ARCHITECTURAL	1.02	NOTED EDITION, IF NOT BUILDING CODE ADOPTED. MODIFICATIONS TO EXISTING STRUCTURE SHALL BE IN COMPLIANCE WITH IBC CHAPTER
		AFF	ABOVE FINISHED FLOOR	1.03	34 OR IEBC. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, AND ELEVATIONS AFFECTING NEW
	в	BM	BEAM		CONSTRUCTION BEFORE STARTING WORK. NOTIFY THE ARCHITECT OF ANY DISCREPANCY.
	D	BRG BOT.	BEARING BOTTOM	1.04	NOTIFY THE ARCHITECT IN WRITING OF CONDITIONS ENCOUNTERED IN THE FIELD CONTRADICTORY TO THOSE SHOWN IN THE CONTRACT DOCUMENTS.
		B/, BO	BOTTOM OF	1.05	THE CONTRACTOR IS SOLELY RESPONSIBLE FOR THE DESIGN, ADEQUACY, AND SAFETY
-	0	BLDG			OF ERECTION BRACING, SHORING, TEMPORARY SUPPORTS, ETC. THE STRUCTURAL SYSTEM AND ITS ELEMENTS SHALL NOT BE CONSIDERED STABLE UNTIL THE STRUCTURE
	С	CFMF	COLD FORMED METAL FRAMING	1.06	IS COMPLETE. COORDINATE STRUCTURAL CONTRACT DOCUMENTS WITH ARCHITECTURAL,
		CGS CJ	CENTER OF GRAVITY OF STRAND CONTRACTION JOINT		MECHANICAL, ELECTRICAL, PLUMBING, AND CIVIL DOCUMENTS. NOTIFY THE ARCHITECT OF ANY CONFLICT AND/OR OMISSION.
		CJ CL CLR	CENTERLINE CLEAR	1.07	COORDINATE AND VERIFY FLOOR AND ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. FOR
		CMU	CONCRETE MASONRY UNIT		ADDITIONAL OPENINGS, INSERTS, SLEEVES, CURBS, PADS, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS SEE ARCHITECTURAL, MECHANICAL, PLUMBING, AND
		COL. COORD.	COLUMN COORDINATE	1.08	ELECTRICAL DRAWINGS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF DIMENSIONS SHOWN ON
╢		CONC. CONN.	CONCRETE		THE STRUCTURAL AND ARCHITECTURAL DRAWINGS. NOTIFY THE ARCHITECT OF ANY DISCREPANCY BEFORE STARTING SHOP DRAWINGS OR ANY WORK. FOR DIMENSIONS
		CONT.	CONTINUOUS	1.09	NOT SHOWN, SEE ARCHITECTURAL DRAWINGS. REVIEW OF SHOP DRAWINGS AND OTHER SUBMITTALS BY THE ARCHITECT DOES NOT
	D	DBA	DEFORMED BAR ANCHOR		RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE
		DIM. DWL	DIMENSION		FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, AND DIMENSIONS SPECIFIED
	Е	EA.	EACH		IN THE CONTRACT DOCUMENTS. CONTRACTOR IS ALSO RESPONSIBLE FOR MEANS, METHODS, TECHNIQUES, SEQUENCES, AND PROCEDURES OF CONSTRUCTION.
		EE EF	EACH END EACH FACE	1.10	CONSTRUCTION SHALL BE TESTED AND INSPECTED BY A QUALIFIED AGENCY IN ACCORDANCE WITH CHAPTER 17 OF THE BUILDING CODE. SEE SPECIFICATIONS FOR
		ES EW	EACH SIDE EACH WAY		THE SCHEDULE OF SPECIAL INSPECTIONS AND ADDITIONAL REQUIREMENTS.
		ELEV. EMBED.	ELEVATION, ELEVATOR EMBEDMENT		STRUCTURAL LOAD CRITERIA RISK CATEGORY II
		EQ. EXIST.	EQUAL EXISTING		SUPERIMPOSED DEAD LOADS:
		EXP. JT.	EXPANSION JOINT		ROOFING 10 PSF     MISCELLANEOUS ROOF LOAD 20 PSF
	F	FTG FND	FOOTING FOUNDATION		MISCELLANEOUS FLOOR LOAD
-	G	GALV.	GALVANIZED	2.03	LIVE LOADS*:
		GC	GENERAL CONTRACTOR		OFFICE
	• -	GR.	GRADE (MATERIAL)		PLANTED AREA 100 PSF
	Н	HK HORIZ.	HOOK HORIZONTAL		STAIRS 100 PSF     MECHANICAL ROOMS 150 PSF**     ELEVATOR MACHINE ROOM 100 PSF**
	I	IF	INSIDE FACE		ELEVATOR MACHINE ROOM
		INFO. JT			** OR ACTUAL WEIGHT OF EQUIPMENT, WHICHEVER IS GREATER. ***OR ACTUAL WEIGHT OF SOIL AND PLANTING.
	_	К	JOINT KIPS	2.04	SNOW LOAD CRITERIA:
	K	KSF	KIPS KIPS PER SQUARE FOOT	2.05	GROUND SNOW LOAD Pg = 5 PSF WIND LOAD CRITERIA:
	I	LLH	LONG LEG		BASIC DESIGN WIND SPEEDV = 105 MPH     WIND EXPOSURE C
		LLV	HORIZONTAL	2.06	WIND EXPOSUREC     INTERNAL PRESSURE COEFFICIENTGcpi = ± 0.18 SEISMIC LOAD CRITERIA:
		LSH	LONG SIDE HORIZONTAL	2.00	• SEISMIC IMPORTANCE FACTOR $I_e = 1.0$
┨		LSV	LONG SIDE VERTICAL		<ul> <li>0.2 SECOND SPECTRAL RESPONSE ACCELERATION S<sub>s</sub> = 0.187g*</li> <li>1.0 SECOND SPECTRAL RESPONSE ACCELERATION S<sub>1</sub> = 0.083g*</li> </ul>
	М	MANUF. MAX.	MANUFACTURER MAXIMUM		SEISMIC LOAD CRITERIA:       Ie = 1.0         SEISMIC IMPORTANCE FACTOR       Ie = 1.0         0.2 SECOND SPECTRAL RESPONSE ACCELERATION       Ss = 0.187g*         1.0 SECOND SPECTRAL RESPONSE ACCELERATION       S1 = 0.083g*         SITE CLASS       D         SITE COEFFICIENT       Fa = 1.6         SITE COEFFICIENT       Fv = 2.4         0.2 SECOND SPECTRAL       SDS = 0.200g*         1.0 SECOND SPECTRAL       SDS = 0.200g*         BIGN RESPONSE ACCELERATION       SD1 = 0.133g*         SEISMIC DESIGN CATEGORY       B         BASIC SEISMIC FORCE RESISTING SYSTEM – OFFICE:       ORDIANARY REINFORCED CONCRETE SHEARWALL
		MECH. MEP	MECHANICAL MECHANICAL,		<ul> <li>SITE COEFFICIENT F<sub>v</sub> = 2.4</li> <li>0.2 SECOND SPECTRAL</li> </ul>
			ELECTRICAL, PLUMBING		<ul> <li>DESIGN RESPONSE ACCELERATION S<sub>DS</sub> = 0.200g*</li> <li>1.0 SECOND SPECTRAL</li> </ul>
		MIN. MISC.	MINIMUM MISCELLANEOUS		<ul> <li>DESIGN RESPONSE ACCELERATION S<sub>D1</sub> = 0.133g*</li> <li>SEISMIC DESIGN CATEGORY B</li> </ul>
	0	OC	ON CENTER		<ul> <li>BASIC SEISMIC FORCE RESISTING SYSTEM – OFFICE: ORDIANARY REINFORCED CONCRETE SHEARWALL</li> </ul>
		OPNG OPP.	OPENING OPPOSITE		• BASIC SEISMIC FORCE RESISTING SYSTEM – OFFICE: ORDIANARY REINFORCED CONCRETE SHEARWALL RESPONSE MODIFICATION COEFFICIENT R = 5 SYSTEM OVERSTRENGTH FACTOR $\Omega_0$ = 2.5 DEFLECTION AMPLIFICATION FACTOR $C_d$ = 4.5 DESIGN BASE SHEAR XXX KIPS SEISMIC RESPONSE COEFFICIENT C <sub>s</sub> = 0.XX • ANALYSIS PER EQUIVALENT LATERAL FORCE PROCEDURE *DEDUCED VALUES ARE ASSUMED DENDING DESULTS OF SITE SPECIFIC SEISMIC
			OPPOSITE HAND OUTSIDE FACE		DEFLECTION AMPLIFICATION FACTOR Cd = 4.5
		OSB	ORIENTED STRAND BOARD		DESIGN BASE SHEARXXX KIPSSEISMIC RESPONSE COEFFICIENT $C_s = 0.XX$
	Ρ	PL. PT	PLATE POST-TENSIONED		REDUCED VALUES ARE ASSUMED PENDING RESULTS OF SITE-SPECIFIC SEISMIC
		PCF	POUNDS PER CUBIC FOOT	2.00	HAZARD ANALYSIS.
-		PCY	POUNDS PER CUBIC YARD	I —	FOUNDATIONS AND SLAB-ON-GROUND THE DESIGN OF FOUNDATIONS, RETAINING WALLS, AND SLABS-ON-GROUND IS BASED ON
		PSF	POUNDS PER SQUARE		THE CRITERIA ESTABLISHED IN THE GEOTECHNICAL REPORT BY NOVA ENGINEERING AND ENVIRONMENTAL, LLC (NOVA) DATED AUGUST 5, 2020. FOUNDATION SIZES AND
		PSI	POUNDS PER SQUARE		EVALUATION OF EXISTING FOUNDATIONS ARE BASED ON AN ALLOWABLE BEARING PRESSURE OF 50 KSF FOR EXISTING DRILLED PIERS, AND 3000 PSF FOR SPREAD
	R	REINF.	REINFORCEMENT,		FOOTINGS. DESIGN OF NEW FOUNDATION ON MICROPILES IS BASED ON 8" DIAMETER, 150 TONS COMPRESSION, TO BE CONFIRMED BY MICROPILE SPECIALITY SUBCONTRACTOR
	IX.	REQ'D	REINFORCING	3.02	AND LOAD TESTS. RETAINING WALLS ARE BASED ON THE FOLLOWING CRITERIA:
	s	SCHED.	SCHEDULE		EQUIVALENT LATERAL FLUID PRESSURE, ACTIVEXX PCF     EQUIVALENT LATERAL FLUID PRESSURE, AT REST XX PCF
	~	SIM. SOG	SIMILAR SLAB ON GROUND		EQUIVALENT LATERAL FLUID PRESSURE, AT RESTX PCF     EQUIVALENT LATERAL FLUID PRESSURE, PASSIVEXXX PCF     COEFFICIENT OF FRICTION 0.XX
		STIFF. SYM.	STIFFENER SYMMETRIC	3.03	THE SPECIAL INSPECTOR SHALL INSPECT THE CONDITION AND ASSURE THE ADEQUACY
	т	TEMP.	TEMPERATURE		OF ALL SUBGRADES, FILLS, AND BACKFILLS BEFORE PLACEMENT OF FOUNDATIONS, FOOTINGS, SLABS, AND WALLS AND SHALL SUBMIT REPORTS TO THE ARCHITECT DESCRIBING THE FINDINGS INCLUDING ANY NON-CONFORMING WORK.
		T&B T/, TO	TOP AND BOTTOM TOP OF	3.04	SIDES OF FOUNDATIONS SHALL BE FORMED UNLESS CONDITIONS PERMIT EARTH
	• -	TYP.			FORMING. FOUNDATIONS POURED AGAINST THE EARTH REQUIRE THE FOLLOWING PRECAUTIONS: SLOPE SIDES OF EXCAVATIONS AS APPROVED BY THE ARCHITECT AND CLEAN UP SLOUGHING BEFORE AND DURING CONCRETE PLACEMENT.
-	U	UNO	UNLESS NOTED OTHERWISE	3.05	WHERE FOOTING STEPS ARE NECESSARY, THEY SHALL BE NO STEEPER THAN ONE
	V	VERT.	VERTICAL	3.06	VERTICAL TO TWO HORIZONTAL. BASEMENT WALLS (WALLS SUPPORTED AT THE TOP AND BOTTOM BY SLABS) REQUIRE
	W	WF			THE FOLLOWING PRECAUTION: DO NOT BACKFILL AGAINST THE WALL BEFORE THE SLABS AT THE TOP AND BOTTOM ARE CAST AND HAVE REACHED 75% OF DESIGN
		WP WWR	WORK POINT WELDED WIRE BEINEORCEMENT	3.07	STRENGTH. UNLESS NOTED OTHERWISE, SLABS-ON-GROUND SHALL BE MINIMUM 5" THICK, PLACED
		W/	REINFORCEMENT WITH		ON PREPARED SUBGRADE. REINFORCED WITH 6x6-W2.9xW2.9 WWF IN FLAT SHEETS (ROLLS NOT PERMITTED). SUPPORT MESH WITH PRECAST CONCRETE SUPPORTS AT 4'-0"
					EACH WAY 1 ½" CLEAR FROM TOP OF SLAB. LAP MASH 2 SQUARES AT SPLICES. STAGGER SPLICES. PROVIDE 15 MIL MINIMUM POLYETHYLENE VAPOR BARRIER BENEATH THE
					FLOOR SLAB WITH JOINTS LAPPED NOT LESS THAN 6 INCHES AND TAPED. PLACE CONTRACTION JOINTS WHERE SHOWN ON PLANS, AT COLUMN LINES, AND AT
					INTERMEDIATE LINES SUCH THAT SPACING BETWEEN JOINTS DOES NOT EXCEED 15'-0" AND ASPECT RATIO OF PANELS DOES NOT EXCEED 1.5. LOCATE CONSTRUCTION JOINTS
					AT CONTRACTION JOINTS. PROVIDE A 4" SUBBASE LAYER OF COMPACTED GRADED AGGREGATE BENEATH THE VAPOR RETARDER.
				<u>4.00</u>	REINFORCED CONCRETE
				4.01	ALL CONCRETE WORK SHALL CONFORM TO ACI 301-16, "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS". DESIGN IS BASED ON ACI 318, "BUILDING
-				4.02	CODE REQUIREMENTS FOR STRUCTURAL CONCRETE". UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL BE NORMALWEIGHT AND HAVE THE
					FOLLOWING MINIMUM 28-DAY STRENGTHS: • FOUNDATIONS 4000 PSI
					FOUNDATION WALLS & RETAINING WALLS 4000 PSI     SLABS-ON-GROUND 4000 PSI
					EXISTING BEAMS, GIRDERS, AND FORMED SLABS (120 PCF) 4000 PSI     SLABS ON STEEL DECK (115 PCF MAXIMUM) 3000 PSI
					ALL OTHER CONCRETE NOT NOTED
					EXISTING COLUMNS4000 PSI
1				4.00	COLUMN ENLARGEMENT 4000 PSI
					CONCRETE MIX DESIGNS, IN ACCORDANCE WITH ACI 301-16 SECTION 4.2.2, SHALL BE SUBMITTED TO THE TESTING AGENCY AND THE ARCHITECT.
				4.04	THE PROPOSED MATERIALS AND MIX DESIGN SHALL BE FULLY DOCUMENTED AND REVIEWED BY THE SPECIAL INSPECTOR. RESPONSIBILITY FOR OBTAINING THE RECURRED DESIGN STRENGTH IS THE CONTRACTOR'S
				4.05	REQUIRED DESIGN STRENGTH IS THE CONTRACTOR'S. USE OF CALCIUM CHLORIDE, CHLORIDE IONS, OR OTHER SALTS IN CONCRETE IS NOT
				l	PERMITTED.
Ľ				<u> </u>	

4.06	THE AIR CONTENT IN ALL CONCRETE EXPOSED TO WEATHER AT POINT OF PLACEMENT SHALL BE PROPORTIONED ACCORDING TO ACI 301-16 TABLE 4.2.2.7(b)1 FOR EXPOSURE CLASS F1. PROVIDE AIR CONTENT THAT COMPLIES WITH FLOOR SYSTEM FIRE RATING	5.10	FLOOR AND ROOF LEVEL. SE WITH 2#5 AT EACH FLOOR AN
4.07	REQUIREMENTS. UNLESS NOTED OTHERWISE, SAMPLES FOR STRENGTH TESTS OF EACH CLASS OF	5.11	
	CONCRETE PLACED EACH DAY SHALL BE TAKEN BY THE TESTING AGENCY NOT LESS THAN ONCE A DAY, NOR LESS THAN ONCE FOR EACH 100 CUBIC YARDS OF CONCRETE,		SHOW WALL STEEL IN ELEVA FABRICATION SHALL BEGIN U
	NOR LESS THAN ONCE FOR EACH 5000 SQUARE FEET OF SURFACE AREA FOR SLABS AND WALLS. WHEN FREQUENCY OF TESTING WILL PROVIDE FEWER THAN FIVE TESTS FOR EACH CONCRETE MIX, TEST SAMPLES SHALL BE MADE FROM AT LEAST FIVE	5.12	UNLESS NOTED OTHERWISE, FOLLOWING CRITERIA:
	RANDOMLY SELECTED BATCHES OR FROM EACH BATCH IF FEWER THAN FIVE BATCHES ARE USED. SAMPLE CONCRETE IN ACCORDANCE WITH ASTM C172. PERFORM THE		<ul> <li>AT 25'-0" ON CENTER M</li> <li>AT 12'-0" MAXIMUM FR</li> </ul>
	FOLLOWING TESTS IN ACCORDANCE WITH THE INDICATED STANDARD:     SLUMP ASTM C143		<ul> <li>NO CLOSER THAN 1'-4</li> <li>NO CLOSER THAN 1'-4</li> <li>DISCONTINUE HORIZO</li> </ul>
	AIR CONTENT:     NORMAL WEIGHT CONCRETE ASTM C231		FIELD OF THE WALL, B CMU LINTELS, MASON
	<ul> <li>LIGHTWEIGHT CONCRETE ASTM C173</li> <li>COMPRESSIVE STRENGTH: ASTM C39, WITH (1) CYLINDER AT 7 DAYS, (2) 6"X12" CYLINDERS OR (3) 4"X8"</li> </ul>		OTHER STRUCTURAL CONTROL JOINTS. PR
	<ul><li>CYLINDERS AT 28 DAYS, AND (1) SPECIMEN HELD IN RESERVE</li><li>UNIT WEIGHT:</li></ul>	5.13	CONTROL JOINT. UNLESS NOTED OTHERWISE, • EXTERIOR WALLS
4.08	LIGHTWEIGHT CONCRETE ASTM C567 BEGIN CURING SLABS IMMEDIATELY AFTER FINISHING CONCRETE PER PROJECT SPECIFICATIONS. CURING METHOD SHALL BE COMPATIBLE WITH ADHERED FINISHES. A	5.14	INTERIOR WALLS     SAMPLE AND TEST GROUT IN
	MOIST CURE OR DISSIPATING CURING COMPOUND SHALL BE USED AT CEMENTITIOUS FINISHES. SUBMIT PRODUCT DATA TO THE ARCHITECT FOR REVIEW.	0.11	CURE ONE SET OF FIVE CUB SURFACE. TEST ONE CUBE DAYS FOR A STRENGTH TES
4.09	HORIZONTAL CONSTRUCTION JOINTS ARE PERMITTED ONLY WHERE INDICATED. THE ARCHITECT SHALL APPROVE THE LOCATION OF VERTICAL CONSTRUCTION JOINTS. CONSTRUCTION JOINTS SHALL BE THOROUGHLY ROUGHENED BY MECHANICAL MEANS	<u>6.00</u>	STRUCTURAL STEEL
4.10	AND CLEANED. UNLESS NOTED OTHERWISE, PLACE VERTICAL CONTRACTION JOINTS IN CONCRETE		ALL STRUCTURAL STEEL COL "SPECIFICATION FOR STRUC
	WALLS AT 20'-0" (MAXIMUM), NOT TO EXCEED THREE TIMES THE HEIGHT OF THE WALL, 3/4" DEEP, V-CHAMFERED ON BOTH FACES. CONSTRUCTION JOINTS SHALL FALL ON CONTRACTION JOINTS AND SHALL BE KEYED. STOP 50 PERCENT OF THE SPECIFIED	6.02	SHOP DRAWINGS PREPARED "DETAILING FOR STEEL CONS APPROVAL. NO FABRICATION
4.11	LONGITUDINAL REINFORCING AT THE CONTRACTION/CONSTRUCTION JOINT. CHAMFER OR ROUND ALL EXPOSED CORNERS MINIMUM 3/4".	6.03	
4.12	DETAIL CONCRETE REINFORCING AND ACCESSORIES IN ACCORDANCE WITH ACI "DETAILING MANUAL", SP-66(04). SUBMIT SHOP DRAWINGS FOR APPROVAL, SHOWING		CONFORM TO ASTM A992, GF SECTIONS SHALL CONFORM ASTM A53, GRADE B; AND AL
	ALL FABRICATION DIMENSIONS AND LOCATIONS FOR PLACING REINFORCING STEEL AND ACCESSORIES. DO NOT BEGIN FABRICATION UNTIL SHOP DRAWINGS ARE COMPLETED AND REVIEWED.	6.04	A36 OR A572, GRADE 50. STEEL FRAMING CONNECTIO
4.13	DETAIL ALL CONCRETE WALLS AND BEAMS IN ELEVATION UNLESS SPECIFICALLY APPROVED OTHERWISE. CUT SECTIONS SHOWING BAR LOCATIONS AND CONCRETE		<ul> <li>BOLTED JOINTS SHAL JOINTS USING HIGH-S</li> </ul>
4.14	COVER. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, UNLESS NOTED		AND SHALL BE MINIMU SHALL BE CONSIDERE OTHERWISE NOTED.
4.15	OTHERWISE. WELDED WIRE REINFORCEMENT (MESH) SHALL CONFORM TO ASTM A1064 AND SHALL BE PROVIDED IN FLAT SHEETS (ROLLS NOT PERMITTED). LAP TWO SQUARES AT SPLICES.		WASHERS) IN ACCORI (TWIST OFF BOLTS) IN
4.16	TIE ALL REINFORCING STEEL AND EMBEDMENTS SECURELY IN PLACE PRIOR TO PLACING CONCRETE. PROVIDE SUFFICIENT SUPPORTS TO MAINTAIN THE POSITION OF		<ul> <li>BOLTS.</li> <li>WELDS SHALL CONFO AMERICAN WELDINGS</li> </ul>
A 4-	REINFORCING WITHIN SPECIFIED TOLERANCES DURING ALL CONSTRUCTION ACTIVITIES. "STICKING" DOWELS INTO WET CONCRETE IS NOT PERMITTED.		AMERICAN WELDING S PROCESSES AND OPE "STANDARD QUALIFIC
4.17	PROVIDE CONTINUOUS REINFORCING WHEREVER POSSIBLE; SPLICE ONLY AS SHOWN OR APPROVED; STAGGER SPLICES WHERE POSSIBLE; USE CLASS "B" TENSION SPLICE UNLESS NOTED OTHERWISE. DOWELS SHALL MATCH THE SIZE AND SPACING OF THE	6.05	QUALIFICATIONS ON T ANCHOR RODS SHALL CONF
	SPECIFIED REINFORCING AND SHALL BE LAPPED WITH CLASS "B" TENSION SPLICES. UNLESS NOTED OTHERWISE LAP LENGTHS EXPRESSED IN NUMBER OF BAR DIAMETERS		OTHERWISE NOTED. FOR AN ROD INTENDED TO PROJECT THE GRADE IDENTIFICATION
	SHALL BE AS FOLLOWS:		CUTTING TORCHES FOR COR FRAMING.
	BAR SIZECLASSNORMAL WEIGHT CONCRETE, f'c (PSI)3000400050006000	6.06	IN ACCORDANCE WITH AISC STEEL CONNECTIONS ARE A FOLLOWS:
	#6 OR SMALLER A 44 DIA 38 DIA 34 DIA 31 DIA		CONNECTIONS NOTE:     WHERE COLUMN SPLI
	B         57 DIA         49 DIA         44 DIA         40 DIA           #7 TO #11         A         55 DIA         47 DIA         42 DIA         39 DIA		OPTION 2 CONNECTION 14-3 OF THE AISC "STE
	B 71 DIA 62 DIA 55 DIA 50 DIA		LOAD DATA IS GIVEN AT THE CONNECTION DESIGN. SEE I BEAM END CONNECTIONS SH
	NOTES: 1. VALUES SHOWN ARE FOR GRADE 60 REINFORCING. MULTIPLY VALUES BY 1.25		<ul> <li>MINIMUM 5/16" THICK I</li> </ul>
	FOR GRADE 75 REINFORCING. 2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH		<ul><li>BOLT SPACING = 3", A</li><li>WHERE BEAM REACTION</li></ul>
	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> </ul>		<ul> <li>BOLT SPACING = 3", A</li> <li>WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, ANI</li> </ul>
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4.19 4.20	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE: <ul> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1"</li> <li>BEAM STIRRUPS AND COLUMN TIES1 '#"</li> </ul> </li> <li>DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> </ul>	6.08 6.09	<ul> <li>PLACE NON-SHRINK, HIGH-STAFTER SETTING AND LEVELING</li> <li>AFTER SETTING AND LEVELING</li> <li>STEEL CONSTRUCTION SHALL</li> <li>SCHEDULE OF SPECIAL INSP</li> <li>BOLTED CONNECTION "SPECIFICATION FOR</li> <li>ALL FILLET WELDS SH</li> <li>ALL COMPLETE PENET GREATER SHALL BE URASTM E164.</li> <li>WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S</li> <li>WRITTEN REPORTS SINDICATING ANY NON-</li> <li>RE-INSPECT NON-COM</li> <li>PROVIDE TEMPORARY BRACE BRACING, MOMENT CONNECC COMPLETELY INSTALLED. THE STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROVIDE</li> </ul>
4.19 4.20	FOR GRADE 75 REINFORCING. 2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE). 3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING. 4. THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7. 5. MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE. 6. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE. 7. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER. 8. ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE. REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE: • CONCRETE CAST AGAINST EARTH (NOT FORMED) 3" • FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS2" #5 BARS AND SMALLER1 '# BEAM STIRRUPS AND COLUMN TIES1 '# DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS. DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT. FLOOR FINISH TOLERANCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTME 1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME 1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ARCHITECT.	6.08 6.09 6.10	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH US SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR
4.19 4.20 4.21	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED) 3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 46 THROUGH #18 BARS</li></ul>	6.08 6.09 6.10 6.11	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH U SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-SI AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHALL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENEL GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I
4.19 4.20 4.21	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>4. THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>5. MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>6. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>7. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>8. ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>7. CONCRETE CAST AGAINST EARTH (NOT FORMED) 3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 1''''''''''''''''''''''''''''''''''''</li></ul>	6.08 6.09 6.10 6.11	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, ANI CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH US SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-SI AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHALL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENEL GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COM PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (IS STRUCTURAL STEEL IN ACCOR MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT ALL
4.19 4.20 4.21 4.22 4.23	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>4. THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>5. MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>6. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>7. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>8. ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:</li> <li>CONCRETTE CAST AGAINST EARTH (NOT FORMED) 3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS AND SMALLER 2"</li> <li># KBARS AND SMALLER 2"</li> <li># CONCRETTE NOT EXPOSED TO EARTH OR WEATHER: 5LAB SAND WALLS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>FLOOR FINISH TOLERANCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTING. MINIMUM FLATINESS VALUES SHALL BE IN ACCORDANCE WITH ASTING FORM THE INDICATED PLANE OF 1/4" IN 10'-0".</li> <li>RESHORES SHALL BE PLACED OS ON AS PRACTICABLE AFTER STRIPPING OPERATION SECTION ON STRUCTION SECTION SECTION SECTION SECTION ON SOMON. CHEED OR PROPERLY RESHORED OR ASTRUCTION LOADS UNLESS THEY ARE SHORED OR PROPERLY RESHORED OR ORDITE. INDICATED PLANE OF 1/4" IN 10'-0".</li> <li>RESHORE SHALL BE PLACED AS SOON AS PRACTICABLE AFTER STRIPPING OPERALINGS. MINIMUM VARIATION FROM THE</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH U SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELL PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELU STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE" GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COM PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TI STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOI STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTUREL STEEL IN ACCOR MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT ALL MINIPUA THICKNESS. AT NC CAP PLATES ALL AROUND TO
4.19 4.20 4.21 4.22	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:</li> <li>#6 THROUGH #18 BARS2"</li> <li>#5 BARS AND SMALLER1'2"</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:</li> <li>SLABS AND SMALLER1'2"</li> <li>CONCRETE CAST AGAINST EARTH OR WEATHER:</li> <li>SLABS AND SMALLER1'2"</li> <li>DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR DIRECTED BY THE ARCHITECT.</li> <li>FLOOR FINCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTM ET155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME FILSS.</li> <li>ON THELO R TACK WELD REINFORCING STEEL UNLESS APPROVED ON DIRECTED BY THE ARCHITECT.</li> <li>FLOOR FINISH TOLERANCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTM ET155. MINIMUM PLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME ET155. MINIMUM PLACED CONCRETE STRIPPING OPERATIONS ARE COMPLETE. NEWLY PLACED CONCRETE SHALL BE IN ACCORDANCE WITH ASTME ET155. MINIMUM PLACEDS VALUES SHALL BE IN</li></ul>	6.08 6.09 6.10 6.11 6.12	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH 0 SAME PAINT USED FOR SHOO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELU STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOF ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COM PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TO STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS ( STRUCTURAL STEEL IN ACCOM MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u>	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH U SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELU STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TO STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTURAL STEEL IN ACCO MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRO GALVANIZING IS DAMAGED O ASTM A780.
4.19 4.20 4.21 4.22 4.23	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIC LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADEDD INSPECTED IN ACCOF ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTON STELL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS ( STRUCTURAL STEEL IN ACCO MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRO GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u> 5.01	FOR GRADE 75 REINFORCING. 2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE). 3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING. 4. THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7. 5. MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE. 6. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE. 7. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS V OF THE LARGER BAR, WHICHEVER IS GREATER. 8. ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE. 7. CONCRETE CAST AGAINST EARTH (NOT FORMED) 3" 6. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 7. #6 THROUGH #1# BARS 7. * FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 7. * CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 7. * SLABS AND SMALLER 7. * CONCRETE ENDES DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS 7. * OF NELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED 7. * OT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS 7. * OT MELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED 7. * CONCRETE STOR INTERIOR SLABS SHALL BE MEASURED IN 7. ACCORDANCE WITH SPECIFICATION SECTION 03300. EXTERTOR SLABS SHALL DRAIN 7. RESHORED. 7. ON THE ARCHITECT. 7. FLOAR PIETS. ONE STINCTURES SHALL BE IN 7. ACCORDANCE WITH SEGFIFICATION SCHON SHARL BE PLACED AS SOON AS PRACTICABLE AFTER STIPPING 7. PRACHITECT. 7. FLOAR PIETS. NOT DETAILS ON SCHON ON THE 7. SUBAS ON STEEL DECK SHALL BE PLACED LEVEL, WITH A THICKNESS 7. OLERANCE OF +10?, * 14', AND WITHIN +1· 34', * 0F THE SLAB OR NEALL BE IN 7. ACCORDANCE WITH SECFIFICATION SCHALL SE SHALL BE IN 7. CONCRETE S	6.08 6.09 6.10 6.11 6.12 6.13 6.14	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFC BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELI STEEL CONSTRUCTION SHAI SCHEDULE OF SPECIAL INSF BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOF ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COI PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. T STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS ( STRUCTURAL STEEL IN ACCO MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORROR GALVANIZING IS DAMAGED OF ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u> 5.01	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN IN IN DETAILS OR SCHEDULES NOTED OTHERWISE:</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.13 6.14 6.15	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELN STEEL CONSTRUCTION SHAN SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TO STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOD STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTURAL STEEL IN ACCOR MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRO GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE <b>STEEL DECKING:</b>
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u> 5.01	FOR GRADE 75 REINFORCING. 2. MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE). 3. MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING. 4. THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7. 5. MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE. 6. ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE. 7. IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZE, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER. 8. ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE. 7. FORMED CONCRETE EXPOSED TO EARTH (NOT FORMED) 3" 7. FORMED CONCRETE EAST SHOWN IN DETAILS OR SCHEDULES NOTED OTHERWISE: 7. CONCRETE CAST AGAINST EARTH (NOT FORMED) 3" 7. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 7. #6 THROUGH #18 BARS	6.08 6.09 6.10 6.11 6.12 6.13 6.13 6.14 6.15	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELI STEEL CONSTRUCTION SHAI SCHEDULE OF SPECIAL INSF BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOF ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNECT COMPLETELY INSTALLED. TO STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS ( STRUCTURAL STEEL IN ACCOM MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRO GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DEFORMING TO STEEL DEC GRADE 50 MINIMUM. TYPE B
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u> 5.01	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.5 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR POYY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEM MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: #6 THROUGH #18 BARS1"</li> <li>BEAM STIRRUPS AND COLUMN TIES1"</li> <li>BEAM STIRRUPS AND COLUMN TIES</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON. RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNECT OMPLETELY INSTALLED. TI STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOI STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTURELS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOI STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTURELS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOI STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I STRUCTURAL STEEL IN ACCO MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT ALL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHO HIGH-PERFORMANCE CORRO GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE BROOF DECK SHALL BE GRAD SEE DRAWINGS FOR STEEL DECK
4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.02 5.03 5.04	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZE, PROVIDE SPECIFIC LAP GASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGE BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETALLS OR SCHEDULES FOR MEMBER TYPPS (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: "# 6 THROUGH #18 BARS1"</li> <li>BEAM STIRPLYS AND COLUMN SIES OT CEARTH OR WEATHER: # 51ABOR WALLER1"</li> <li>BEAM STIRPLYS AND COLUMN TIES1"</li> <li>BEAM STIRPLYS AND COLUMN TIES1"</li> <li>BEAM STIRPLYS AND COLUMN TIES1"</li> <li>BEAM STIRPLYS AND COLUMN TIES SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCING SLEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>FLOOR RINIS TOLERANCES FOR INTERIOR SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTMET155. MINIMUM FLATNESS VALUES SABL BE IN ACCORDANCE WITH SPECIFICATION SCIEND ON SPECIFICATION SHOWN AND DETAILED ON THE COME WITH SPECIFICATION STORED ON SPECIFICATION STRUCTURAL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCING SLABS SHALL BE MEASURED IN ACCORDANCE WITH ASTMETISTOR AS PRACTICABLE AFTER STRIPPING OPERATIONS ARE COMPLETE. NEWLY PLACED CONCRETE FLOORS SHALL DRAWING AS PRACTICABES STALL DRAWING AS AND ADD TABLE DATE STRUPING OPERATIONS ARE COMPLETE. NEWLY PLACED CONCRETE FLOORS SHALL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCIN</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.13 6.14 6.15 7.00 7.01	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, ANI CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH US SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELIN STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENET GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS SI INDICATING ANY NON- RE-INSPECT NON-COM PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (IN STRUCTURER. PROVIDE I PROVIDE CAP PLATES AT ALL MINIMUM THICKNESS. AT NC CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORROR GALVANIZING IS DAMAGED CA STRUCTURAL STEEL IN ACCOR MANUFACTURER. PROVIDE I PROVIDE CAP PLATES AT ALL MINIMUM THICKNESS. AT NC CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORROR GALVANIZING IS DAMAGED CA STM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANGE SUBMITTAL SECTION OF THE DEFORMED BAR ANCHORS (IN STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANGE SUBMITTAL SECTION OF THE DEFORMED BAR ANCHORS (IN STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANGE SUBMITTAL SECTION OF THE DEFORMED BAR ANCHORS (IN STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANGE SUBMITTAL SECTION OF THE DEFORMED BAR ANCHORS (IN STEEL DECKING: UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORROR GALVANIZING IS DAMAGED CA STM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANGE SUBMITTAL SECTION OF THE DEFORMED BAR ANCHORS FOR STEEL IN STEEL FLOOR DECK SHALL BE GRAD SEE DRAWINGS FOR STEEL IN STEEL FLOOR DECK SHALL BE FOUD
4.19 4.20 4.21 4.22 4.23 4.24 <u><b>5.00</b></u> 5.01 5.02 5.03	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHEN MULTIPLED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARCRE BAR, WHICHEVER IS ORGATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILER OR SCHEDULES FOR MEMBER TYPPS (E.G. COLMNS) SHALLE PAR SIZE, OR CLASS 'A' OF THE LENGTHS SHOWN HERE.</li> <li>REINFORCING STEEL SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED)3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH O SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELU STEEL CONSTRUCTION SHAI SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNECT STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (( STRUCTURAL STEEL IN ACCOMANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANGRO HIGH-PERFORMANCE CORRECT GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND ANCHOR HIGH-PERFORMANCE CORRECT GRADE 50 MINIMUM. TYPE B ROOF DECK SHALL BE GRAD SEE DRAWINGS FOR STEEL DECK STEEL FLOOR DECK SHALL E PUDDLE WELDS AT THE FOLL AT FRAMING PERPEN EACH EDGE FLUTE OF
4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.02 5.03 5.04	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLUCE).</li> <li>MULTIPLY VALUES BY 1.5 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPCIFED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 'A' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMIS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN HERE.</li> <li>REINFORCING STELE SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED OTHERWISE:</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED) 3"</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 1'S BARS AND SMALLER 1.1'S</li> <li>CONCRETE OXE SPOSED TO EARTH OR WEATHER: 1'S BARS AND SMALLER 1.1'S</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'S BARS AND SMALLER 1.1'S</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'S BLASS AND WALLS</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'S BLASS AND WALLS</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'S BLASS AND WALLS</li> <li>DO NOT PLACE PIPES OR DUCTS EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS WITHIN THE SLAB OR WALL UNLESS SPECIFICALLY SHOWN AND DETAILED ON STRUCTURAL DRAWINGS.</li> <li>DO NOT WELD OR TACK WELD REINFORCING SLEBS SHALL BE MEASURED IN ACCORDANCE WITH ASTME 1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME 1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME 1155. MINIMUM FLATNESS VALUES SHALL BE IN ACCORDANCE WITH ASTME STACK.</li> <li>PROKIDES SHALL BE CLOCED AS SOON AS PRACTICABLE AFTER STRIPPING OPERATIONS SCHOLONS EXCERING CONCRETE BLAST ON THE SUBJECTED TO ANY CONSTRUCTION LOADS UNLESS THEY ARE SHORED OR PROPERLY RESHORED.</li> <li>OCNOR</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH 0 SAME PAINT USED FOR SHOP CONCRETE OR FIREPROOFII SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELID STEEL CONSTRUCTION SHALL SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOP ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COI PROVIDE TEMPORARY BRACD BRACING, MOMENT CONNEC COMPLETELY INSTALLED. TI STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTOR STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (0 STRUCTURAL STEEL IN ACCOM MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT ALL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORROR GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DEFORMENDATIONS OF THE DEFORMING TO STEEL DECK ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DEFORMING TO STEEL DECK ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DEFORMING TO STEEL DECK STEEL FLOOR DECK SHALL BE GRAD SEE DRAWINGS FOR STEEL DECK AT FRAMING PARALLE SIDE LAPS SHALL BE IN AT FRAMING PARALLE SIDE LAPS SHALL BE IN AT FRAMING PARALLE SIDE LAPS SHALL BE IN SIDE LAPS SHALL BE
4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.01 5.02 5.03 5.04 5.05	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>A. MULTPLY VALUES BY 1.3 FOR TOP BARS (WHERE MORE THAN 12' OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTPLY VALUES BY 1.3 FOR LEPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3. WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS Y OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>AND THER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LEWSTHS SHOWN HERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED) 3''''''''''''''''''''''''''''''''''''</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01	BOLT SPACING = 3", A WHERE BEAM REACTI PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, ANI CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH US SAME PAINT USED FOR SHOU CONCRETE OR FIREPROOFING SURFACES RECEIVING WELL PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELING STEEL CONSTRUCTION SHAL SCHEDULE OF SPECIAL INSP ALL FILLET WELDS SH ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE UN ASTM E164. WELDING OF HEADED INSPECTED IN ACCOR ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON. RE-INSPECT NON-CON PROVIDE TEMPORARY BRACE BRACING, MOMENT CONNECT COMPLETELY INSTALLED. TH STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I) STELL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS (I) STRUCTURER. PROVIDE PROVIDE CAP PLATES AT ALL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DEFORMING TO STEEL DEC MANUFACTURER. PROVIDE PLATES, BOLTS, AND ANCHOR STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE DIDLE WELDS AT THE FOLD AT FRAMING PARALLE SIDE LAPS SHALL BE GRAD SEE DRAWINGS FOR STEEL DEC STEEL FLOOR DECK SHALL BE PUDDLE WELDS AT THE FOLD AT FRAMING PARALLE SIDE LAPS SHALL BE IN STEEL ROOF DECK SHALL BE PUDDLE WELDS AT THE FOLD AT FRAMING PARALLE SIDE LAPS SHALL BE IN AT FRAMING PARALLE SIDE LAPS SHALL BE IN AT FRAMING PARALLE SIDE LAPS SHALL BE IN STEEL ROOF DECK SHALL BE PUDDLE WELDS AT THE FOLD STEEL ROOF DECK SHALL BE PUDDLE WELDS AT THE FOLD STEEL ROOF DECK SHALL BE PUDDLE WELDS AT THE FOLD STEEL ROOF DECK SHALL BE STEEL ROOF DECK SHALL BE PUDDLE WELDS AT THE FOLD STEEL ROOF DECK SHALL BE STEEL ROOF DECK SHALL BE
4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.01 5.02 5.03 5.04 5.05	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>MULTIPLY VALUES BY 1.5 FOR TOP BARS (WHERE MORE THAN 12' OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTIPLY VALUES BY 1.5 FOR TOP SAYS (OATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3. WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTIPLY VALUES BY 1.3 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALT FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS 3' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>AND THE SPECIFIC LAPS SHOWN IN DETALS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENGTHS SHOWN WHERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED) 3'</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 1'</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'</li> <li>BARS AND VALLS</li> <li>BARS MULLS</li> <li>CONCRETE NOT EXPOSED TO EARTH OR WEATHER: 1'</li> <li>BARS AND VALLS</li> <li>BOND T PLACE PIPES OR DUCTS EXCEEDING ONE.THIRD THE SLAB OR WALL THICKNESS</li> <li>ON TO PLACE PIPES OR DUCTS EXCEEDING ONE.THIRD THE SLAB OR WALL THICKNESS</li> <li>ON TO TAUGE PIPES OR DUCTS AND COLUMN TIES 1''</li> <li>DO NOT PLACE PIPES OR DUCTS OR STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>DO NOT VELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>DO NOT VELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>DO NOT VELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECT.</li> <li>DO NOT VELD OR TACK WELD REINFORCING STEEL UNLESS APPROVED OR DIRECTED BY THE ARCHITECTION SAGON STRUCTURES SHALL BE IN ACCORDANCE WITH ASTM AST AND APPROVED ON DIRECTED DANY OR THE INDICATED PLANE OF 14'' IN 10-0''.</li> <li>RESHORES SHALL BE PLACED DE SOON AS PRACTICABLE AFTER STRIPPING OPERATIONS ARE COMPLETE. NEWLY PLACED CONCRETE FLOORS SHALL DE IN ACCORDANCE WITH</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01 7.02 7.03	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFG BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELL PAINTED. PLACE NON-SHRINK, HIGH-S AFTER SETTING AND LEVELI STEEL CONSTRUCTION SHAI SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WEITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CON PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNECT STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTO STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THE DEFORMED BAR ANCHORS ( STRUCTURAL STEEL IN ACCOM MANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRIG GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL STEEL IN ACCOM GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRIG GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE PLATES, BOLTS, AND ANCHOR HIGH-PERFORMANCE CORRIG GRADE 50 MINIMUM. TYPE B ROOF DECK SHALL BE GRAD SEE DRAWINGS FOR STEEL STEEL FLOOR DECK SHALL BE PUDDLE WELDS AT THE FOL AT FRAMING PARALLE STEEL FLOOR DECK SHALL BE SPACING OF 2 FEET O STEEL ROOF DECK SHALL BE SPACING OF 2 FEET O
4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.01 5.02 5.03 5.04 5.05	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>A. MULTPLY VALUES BY 13 FOR TOP BARS (WHERE MORE THAN 12' OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTPLY VALUES BY 15 FOR EPOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.7.</li> <li>MULTPLY VALUES BY 13 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>I IL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>IN THE CASE OF SPLICES BETWEEN TWO DIFFERENT BAR SIZES, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS Y OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>AND THER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E.G. COLUMNS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LENTHS SHOWN MERE.</li> <li>REINFORCING STEEL, SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED THE NETWORK STEELS SHALL HAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED THE WORTHS SHOWN MERE.</li> <li>CONCRETE CAST AGAINST EARTH (NOT FORMED) 3''</li> <li>FORMED CONCRETE EXPOSED TO EARTH OR WEATHER: 2''</li> <li>SLABS AND SMALLES</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.14 6.13 6.14 6.15 7.00 7.01 7.02 7.03	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFO BEAM SIZE, SPAN, AN CONNECTIONS FOR 9 PROVIDE A SHOP COAT OF S WITH FINISH COAT. TOUCH I SAME PAINT USED FOR SHO! CONCRETE OR FIREPROOFIN SURFACES RECEIVING WELD PAINTED. PLACE NON-SHRINK, HIGH-S' AFTER SETTING AND LEVELI STEEL CONSTRUCTION SHAI SCHEDULE OF SPECIAL INSP BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE U ASTM E164. WELDING OF HEADED INSPECTED IN ACCOF ALL STUDS AND DBA'S WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-COI PROVIDE TEMPORARY BRAC BRACING, MOMENT CONNECT COMPLETELY INSTALLED. TI STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTO STEEL DECK AS SHOWN ON RECOMMED BAR ANCHORS ( STRUCTURE. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND TO UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHOR GALVANIZING IS DAMAGED C ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE STEEL DECK SHALL BE GRAD SEE DRAWINGS FOR STELL DECK ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE STEEL DECK SHALL BE GRAD SEE DRAWINGS FOR STELL DE CONFORMING TO STEEL DECK ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE STEEL FLOOR DECK SHALL BE PUDDLE WELDS AT THE FOLL • AT FRAMING PERPEN EACH EDGE FLUTE OF • AT FRAMING PERPEN EACH EDGE FLUTE OF • AT FRAMING PERPEN EACH EDGE FLUTE OF • AT FRAMING PERPEN BACING OF 2 FEET OF
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4.19 4.20 4.21 4.22 4.23 4.24 5.01 5.02 5.03 5.04 5.05 5.06	<ul> <li>FOR GRADE 75 REINFORCING.</li> <li>AUTPLY VALUES 87 1.3 FOR TOP BARS (WHERE MORE THAN 12" OF FRESH CONCRETE IS CAST BELOW THE SPLICE).</li> <li>MULTPLY VALUES 87 1.5 FOR EDOXY COATED REINFORCING.</li> <li>THE FACTORS FROM NOTES 2 AND 3, WHEN MULTIPLIED, NEED NOT BE GREATER THAN 1.</li> <li>MULTPLY VALUES 87 1.5 FOR LIGHTWEIGHT AND SEMI-LIGHTWEIGHT CONCRETE.</li> <li>ALL FACTORS ARE CUMULATIVE AND NOT MUTUALLY EXCLUSIVE.</li> <li>THE CASE OF SPLICES BETWEEN TWO DIFFERENT TARS JESSE, PROVIDE SPECIFIED LAP BASED ON THE SMALLER BAR SIZE, OR CLASS X' OF THE LARGER BAR, WHICHEVER IS GREATER.</li> <li>ANY OTHER SPECIFIC LAPS SHOWN IN DETAILS OR SCHEDULES FOR MEMBER TYPES (E. C.CLUMINS) SHALL BE PERMITTED TO BE USED IN LIEU OF LAP LIENGTHS STELLS AND. TAVE THE FOLLOWING CONCRETE COVER UNLESS NOTED CONCRETE EXPOSED TO EARTH OR WEATHER: """"""""""""""""""""""""""""""""""""</li></ul>	6.08 6.09 6.10 6.11 6.12 6.13 6.13 6.14 6.13 6.14 7.00 7.01 7.02 7.03	BOLT SPACING = 3", A WHERE BEAM REACT PROPORTIONED TO S SHOWN IN THE UNIFG BEAM SIZE, SPAN, AN CONNECTIONS FOR SHO WITH FINISH COAT. TOUCH SAME PAINT USED FOR SHO CONCRETE OR FIREPROOFI SURFACES RECEIVING WELL PAINTED. PLACE NON-SHRINK, HIGH-S AFTER SETTING AND LEVEL STEEL CONSTRUCTION SHA SCHEDULE OF SPECIAL INSF BOLTED CONNECTION "SPECIFICATION FOR ALL FILLET WELDS SH ALL COMPLETE PENE GREATER SHALL BE O ASTM E164. WELDING OF HEADED INSPECTE DI N ACCOM ALL STUDS AND DBA' WRITTEN REPORTS S INDICATING ANY NON RE-INSPECT NON-CO PROVIDE TEMPORARY BRAC COMPLETELY INSTALLED. T STRUCTURE IS COMPLETED SHEAR CONNECTORS: PROV HEADED SHEAR CONNECTO STEEL DECK AS SHOWN ON RECOMMENDATIONS OF THI DEFORMED BAR ANCHORS OF STRUCTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND T UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHO STRUCTURAL STEEL IN ACCOMANUFACTURER. PROVIDE PROVIDE CAP PLATES AT AL MINIMUM THICKNESS. AT NO CAP PLATES ALL AROUND T UNLESS NOTED OTHERWISE PLATES, BOLTS, AND ANCHO HIGH-PERFORMANCE CORR GALVANIZING IS DAMAGED O ASTM A780. STRUCTURAL DESIGN OF ST EMBEDS, POSTS, AND HANG SUBMITTAL SECTION OF THE SIDE LAPS SHALL BE GRAD SETEL DECKING: UNLESS NOTED OTHERWISE CONFORMING TO STEEL DEC GRADE 50 MINIMUM. TYPE E ROOF DECK SHALL BE GRAD SIDE LAPS SHALL BE GRAD SIDE LAPS SHALL BE CONTIN STRUCTURAL DESIGN OF STEL STEEL FLOOR DECK SHALL BE PUDDLE WELDS AT THE FOL AT FRAMING PERPEN MAXIMUM AND AT EA AT FRAMING PERPEN MAXIMUM AND

CHORED TO STRUCTURE AT TOP OF WALL AND AT EACH TYPICAL DETAILS. PROVIDE BOND BEAMS REINFORCED ROOF ANCHORAGE LEVEL AND AT TOP OF WALL. BMITTED FOR APPROVAL SHOWING ALL FABRICATION OR PLACING REINFORCING STEEL AND ACCESSORIES. DN. INDICATE CONTROL JOINT LOCATIONS. NO FIL SHOP DRAWINGS ARE COMPLETED AND APPROVED. ROVIDE VERTICAL CONTROL JOINTS ACCORDING TO THE

BUILDING CORNERS

MUM

D MAJOR BEAM OR JOIST BEARINGS AL JOINT REINFORCING AT CONTROL JOINTS IN THE NOT AT CORNERS OR INTERSECTIONS. BOND BEAMS, BENEATH STEEL BEAM AND JOIST BEARINGS, AND EMENTS SHALL EXTEND UNINTERRUPTED ACROSS IDE RAKED JOINTS IN THESE ELEMENTS TO MATCH THE

INFORCE ALL WALLS AS FOLLOWS: \_\_\_\_\_\_#5@48" VERTICAL \_\_\_\_\_#5@48" VERTICAL CORDANCE WITH ASTM C1019. CAST AND LABORATORY

FOR EACH 5000 SQUARE FEET OF MASONRY WALL SEVEN DAYS FOR INFORMATION, THREE CUBES AT 28 ND HOLD ONE CUBE IN RESERVE.

TRUCTION SHALL CONFORM TO THE AISC 360, RAL STEEL BUILDINGS".

ACCORDANCE WITH AISC'S DETAILING MANUAL UCTION, 3<sup>RD</sup> EDITION" SHALL BE SUBMITTED FOR IALL BEGIN UNTIL SHOP DRAWINGS ARE COMPLETED

RUCTURAL STEEL WIDE FLANGES AND TEES SHALL E 50; ROUND, SQUARE, AND RECTANGULAR HSS ASTM A500, GRADE C; ROUND PIPES SHALL CONFORM TO THER SHAPES AND PLATES SHALL CONFORM TO ASTM

SHALL BE BOLTED OR WELDED:

ONFORM TO RCSC "SPECIFICATION FOR STRUCTURAL ENGTH BOLTS". BOLTS SHALL CONFORM TO ASTM A325, 3/4" DIAMETER, UNLESS NOTED OTHERWISE. ALL BOLTS BEARING TYPE WITH BOLTS PRE-TENSIONED, UNLESS OVIDE DIRECT TENSION INDICATORS (LOAD INDICATING ICE WITH ASTM F959 OR TENSION CONTROL BOLTS ICCORDANCE WITH ASTM F1852 FOR ALL HIGH-STRENGTH

TO THE "STRUCTURAL WELDING CODE" OF THE CIETY, AWS D1.1. USE E70XX ELECTRODES. WELDING TORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS ONS PROCEDURES". WELDERS SHALL CARRY PROOF-OF-IR PERSONS.

M TO ASTM F1554, GR 55, S1 (WELDABLE) UNLESS IOR RODS TO BE GALVANIZED, THE END OF THE ANCHOR COM THE CONCRETE SHALL BE STEEL DIE STAMPED WITH REQUIRED BY SUPPLEMENT S3. DO NOT USE GAS ECTING FABRICATION ERRORS IN THE STRUCTURAL

"THE CODE OF STANDARD PRACTICE", ALL STRUCTURAL FERRED SUBMITTAL PER OPTION 3B, EXCEPT AS

OPTION 1. FORCES ARE NOT SHOWN, COLUMN SPLICES ARE TO BE SELECTED AND COMPLETED FROM CASES I–X OF TABLE CONSTRUCTION MANUAL".

CTORED LOAD LEVEL AND LRFD SHALL BE USED FOR ERRED SUBMITTAL SECTION OF THE GENERAL NOTES. L BE DESIGNED AS FOLLOWS: JBLE ANGLE, OR 3/8" THICK SINGLE-PLATE SHEAR

TH OF THE BEAM, WELDED OR BOLTED WITH VERTICAL S ARE NOT SHOWN, CONNECTIONS SHALL BE

PORT 60% OF THE TOTAL UNIFORM LOAD CAPACITY (ULC) LOAD TABLES OF THE AISC MANUAL, FOR THE SPECIFIED RADE OF STEEL. FOR COMPOSITE BEAMS, PROPORTION OF THE ULC.

NDARD PRIMER PAINT. PRIMER SHALL BE COMPATIBLE AREAS DAMAGED IN HANDLING AND ERECTION WITH THE DAT. STEEL SURFACES TO BE WELDED OR ENCASED IN CONNECTIONS DESIGNATED AS SLIP CRITICAL TYPE, OR SHEAR CONNECTORS IN THE FIELD SHALL NOT BE

NGTH GROUT (MINIMUM 6,000 PSI) UNDER BASE PLATES AND PRIOR TO PLACING ELEVATED SLAB CONCRETE. E INSPECTED BY A QUALIFIED SPECIAL INSPECTOR. SEE TONS FOR ADDITIONAL INFORMATION.

HALL BE INSPECTED IN ACCORDANCE WITH RCSC UCTURAL JOINTS USING HIGH-STRENGTH BOLTS". BE VISUALLY INSPECTED. TION WELDS IN MATERIALS 5/16" INCH THICK OR

ASONIC TESTED IN ACCORDANCE WITH AWS D1.1 AND

NCE WITH AWS D1.1. TEST 15% OF ALL STUDS. RETEST NANY MEMBER WHOSE STUDS FAILED INITIAL TESTING. IL BE SUBMITTED DESCRIBING ALL INSPECTIONS AND NFORMING WORK.

ORMING WORK AFTER IT IS CORRECTED. G OF STRUCTURAL FRAMING UNTIL ALL PERMANENT DNS, AND FLOOR AND ROOF DECKS (DIAPHRAGMS) ARE STRUCTURAL ELEMENTS ARE UNSTABLE UNTIL THE

ACCORDANCE WITH THE PLANS. AWS D1.1, TYPE B, 3/4" DIAMETER, SOLID FLUXED FUDS AUTOMATICALLY END WELDED THROUGH THE E DRAWINGS AND IN ACCORDANCE WITH THE ANUFACTURER.

A'S): FLUX FILLED BARS AUTOMATICALLY END WELDED TO DANCE WITH THE RECOMMENDATION OF THE TERIAL WITH MINIMUM YIELD STRENGTH OF 60 KSI. OLUMNS. AT BEARING CONDITIONS, PROVIDE 3/4"

EARING CONDITIONS, PROVIDE 1/4" THICKNESS. WELD DLUMNS. L EXPOSED STRUCTURAL AND MISCELLANEOUS STEEL,

SHALL BE GALVANIZED OR PAINTED WITH AN APPROVED ON RESISTANT COATING SYSTEM. CLEAN AREAS WHERE MISSING AND REPAIR GALVANIZING TO COMPLY WITH

STAIRS, LANDINGS, AND GUARDRAILS (INCLUDING) S) IS A DEFERRED SUBMITTAL. SEE DEFERRED ENERAL NOTES.

EEL DECK SHALL BE GALVANIZED (MINIMUM G60), NSTITUTE (SDI) STANDARDS. FLOOR DECK SHALL BE OF DECK SHALL BE GRADE 80 MINIMUM AND ALL OTHER MINIMUM.

CK TYPE AND GAUGE. ASTENED TO STEEL FRAMING WITH 5/8" DIAMETER VING SPACINGS:

ULAR TO DECK FLUTES: 12" ON CENTER MAXIMUM AND AT CH DECK UNIT.

TO DECK FLUTES: 24" ON CENTER MAXIMUM. STENED WITH #10 SELF-TAPPING SCREWS AT A MAXIMUM TER TO CENTER BETWEEN SUPPORTS.

STENED TO STEEL FRAMING WITH 5/8" DIAMETER VING SPACINGS, UNLESS NOTED OTHERWISE: ULAR TO DECK FLUTES (AT 1-1/2" DECK): 12" ON CENTER

EDGE FLUTE OF EACH DECK UNIT. JLAR TO DECK FLUTES (AT 3" DECK): 8" ON CENTER EDGE FLUTE OF EACH DECK UNIT. O DECK FLUTES: 12" ON CENTER MAXIMUM.

TER TO CENTER BETWEEN SUPPORTS. JS OVER 3 SPANS MINIMUM WHERE SUPPORTING

STRICT ACCORDANCE WITH MANUFACTURER'S

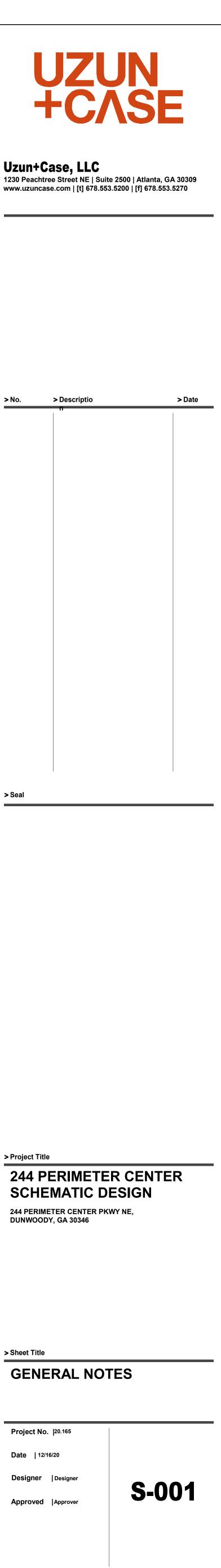
MBER OF THE STEEL DECK INSTITUTE. SUBMIT

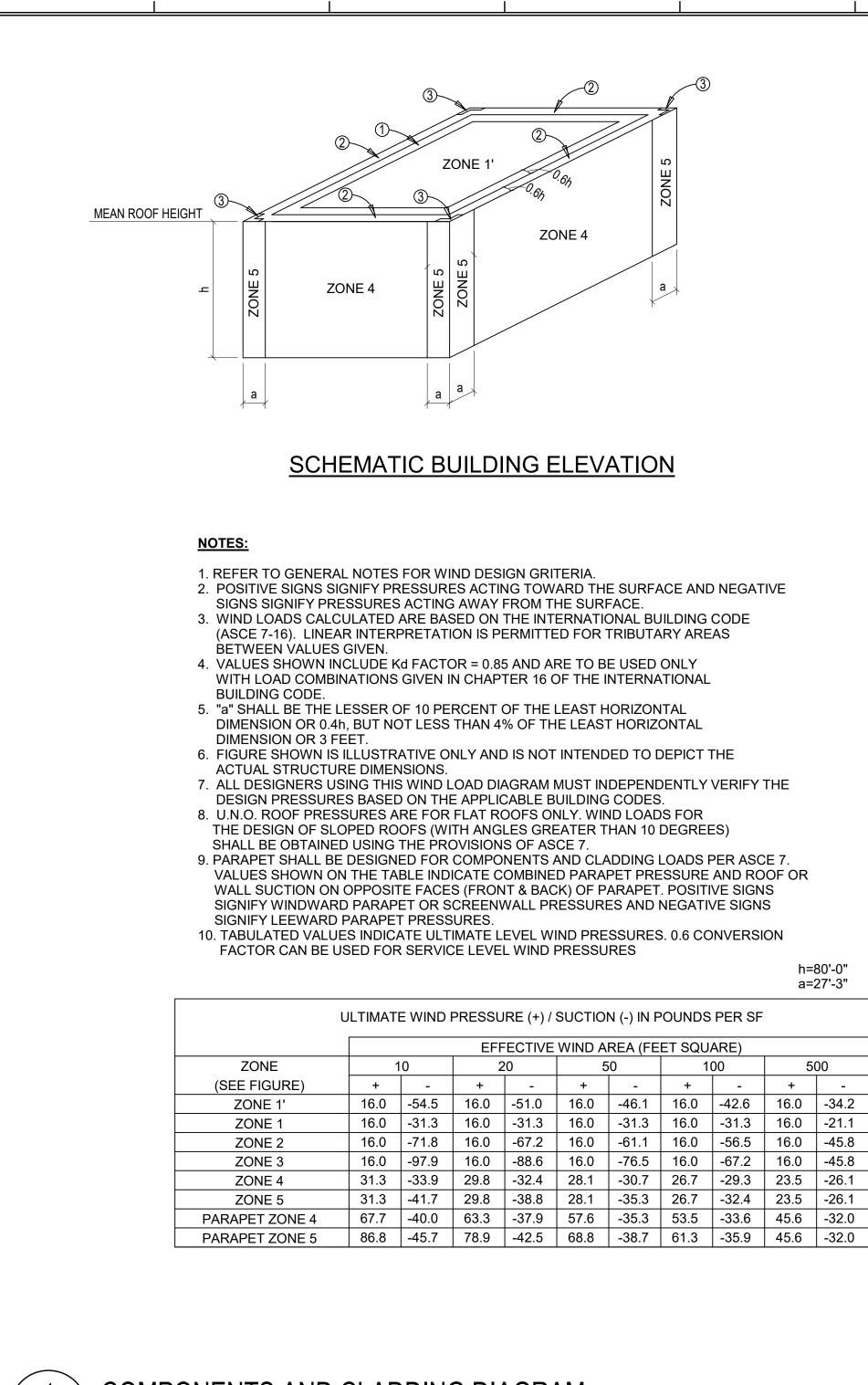
AN TABLES, FOR REVIEW. HANDLING AND ERECTION WITH GALVANIZING REPAIR

- 7.09 USE WELDING WASHERS WHEN DECK IS LESS THAN 22 GAUGE THICK. WELDING WASHERS SHALL BE MINIMUM 16 GAUGE AND HAVE A NOMINAL 3/8" DIAMETER HOLE. 8.00 COLD FORMED METAL FRAMING 8.01 UNLESS NOTED OTHERWISE, COLD FORMED METAL FRAMING SHALL COMPLY WITH ASTM A1003, STRUCTURAL GRADE, TYPE H, METALLIC COATED. ALL MEMBERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A653, OR ALUMINUM/ZINC COATED IN ACCORDANCE WITH ASTM A792. 8.02 PRIOR TO FABRICATION, THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS INDICATING ALL MEMBER GAUGES, SPACINGS, AND SIZES. INCLUDE COMPLETE DETAILS OF ALL CONNECTIONS. FABRICATION SHALL NOT BEGIN UNTIL SUCH SHOP DRAWINGS ARE SUBMITTED AND APPROVED. 8.03 COLD FORMED METAL FRAMING IS A DEFERRED SUBMITTAL. SEE DEFERRED SUBMITTAL SECTION OF THE GENERAL NOTES. SUBMITTAL SHALL INCLUDE GAUGES, SIZES, SPACING, AND CONNECTION DETAILS. DESIGN LOADS SHALL BE AS SPECIFIED BY THE CONTRACT DOCUMENTS AND/OR THE APPLICABLE CODES, WHICHEVER IS MORE STRINGENT. 8.04 DESIGN COLD FORMED METAL FRAMING SYSTEMS TO WITHSTAND THE DESIGN LOADS WITHOUT EXCEEDING THE FOLLOWING DEFLECTION CRITERIA: MEMBERS SUPPORTING MASONRY L/600 OR 1/2" MAX MEMBERS SUPPORTING OTHER MATERIAL
   L/360 OR 1" MAX 9.00 POST-INSTALLED ANCHORS AND ADHESIVE ANCHORED REBAR 9.01 UNLESS NOTED OTHERWISE, POST-INSTALLED CONCRETE ANCHORS SHALL COMPLY WITH ICC-ES ACCEPTANCE CRITERIA FOR ANCHORS IN CRACKED CONCRETE AND SEISMIC APPLICATIONS. 9.02 POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS UNLESS APPROVED OTHERWISE BY THE ARCHITECT. 9.03 UNLESS NOTED OTHERWISE, ALL ANCHORS SHALL HAVE A COATING THAT PROVIDES CORROSION RESISTANCE EQUIVALENT TO MECHANICALLY GALVANIZED PER ASTM GALVANIZED B695 CLASS 55. 9.04 PLACE POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR AND EMBEDMENTS. 9.05 DRILL AND PREPARE HOLES AND INSTALL ANCHORS IN ACCORDANCE WITH EVALUATION REPORTS AND MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS. 9.06 POST-INSTALLED ANCHORS SHALL BE INSPECTED BY A QUALIFIED SPECIAL INSPECTOR IN ACCORDANCE WITH THE PROJECT STATEMENT OF SPECIAL INSPECTIONS AND THE EVALUATION REPORT. UNLESS NOTED OTHERWISE IN THE EVALUATION REPORT, THE SPECIAL INSPECTOR SHALL INSPECT THE INITIAL INSTALLATION OF EACH TYPE OF
- ANCHOR AND PERIODICALLY INSPECT INSTALLATIONS THEREAFTER.
   9.07 MECHANICAL EXPANSION ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193. ACCEPTABLE MECHANICAL ANCHORS FOR USE IN CONCRETE INCLUDE THE FOLLOWING:

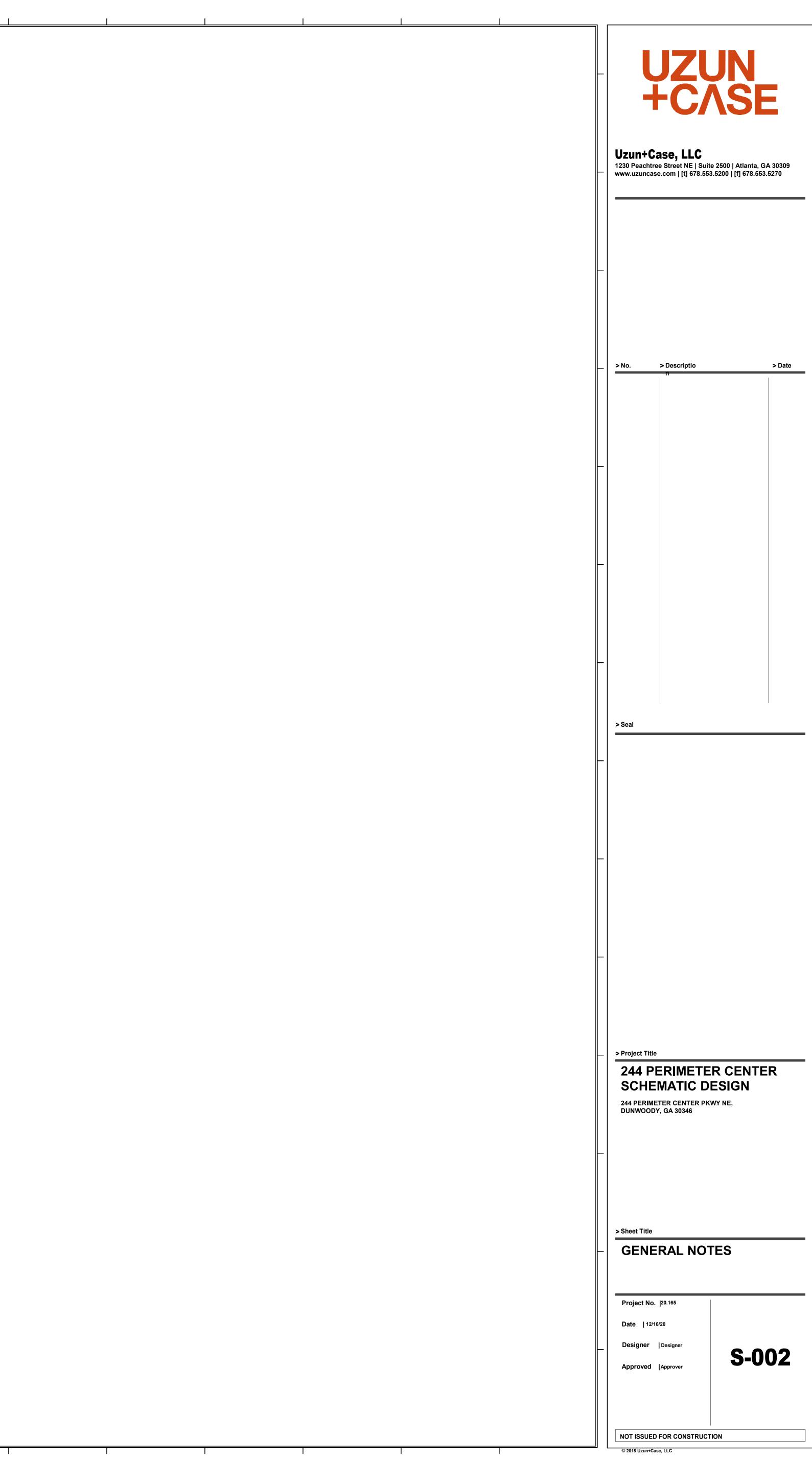
   HILTI KWIK BOLT TZ (ICC-ES ESR 1917)
   SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC-ES ESR 3037)
  - SIMPSON STRONG-TIE STRONG-BOLT 2 (ICC-ES ESR 303
     DEWALT POWER STUD+ SD2 (ICC-ES ESR 2502)
- 9.08 MECHANICAL SCREW ANCHORS FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193. ACCEPTABLE MECHANICAL SCREW ANCHORS FOR USE IN CONCRETE INCLUDE THE FOLLOWING:
   HILTI KWIK HUS-EZ (ICC-ES ESR 3027)
  - DEWALT SCREW-BOLT+ (ICC-ES ESR-3889)
    SIMPSON STRONG-TIE TITEN HD (ICC-ES ESR 2713)
- 9.09 ADHESIVE ANCHORS, INCLUDING REBAR, FOR USE IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ACI 355.4 AND ICC-ES AC308. ADHESIVE ANCHORS SHALL BE INSTALLED INTO DRY HOLES DRILLED USING A CARBIDE DRILL BIT INTO CONCRETE THAT HAS CURED FOR AT LEAST 21 DAYS. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION. ACCEPTABLE ADHESIVE ANCHORS FOR USE IN CONCRETE INCLUDE THE FOLLOWING:
  - HILTI HIT-HY 200 (ICC-ES ESR 3187)
     HILTI HIT-RE 500 V3 (ICC-ES ESR 3814)
  - DEWALT AC200+ (ICC-ES ESR 4027)
- SIMPSON STRONG-TIE AT-XP (IAPMO UES ER 263)
   9.10 MECHANICAL EXPANSION ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC01. ACCEPTABLE MECHANICAL ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY INCLUDE THE FOLLOWING:
  - HILTI KWIK BOLT 3 (ICC-ES ESR 1385)
  - DEWALT POWER STUD+ SD1 (ICC-ES ESR 2966)
    SIMPSON STRONG-TIE WEDGE-ALL (ICC-ES ESR 1396)
- 9.11 MECHANICAL SCREW ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC106. ACCEPTABLE MECHANICAL SCREW ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY INCLUDE THE FOLLOWING:

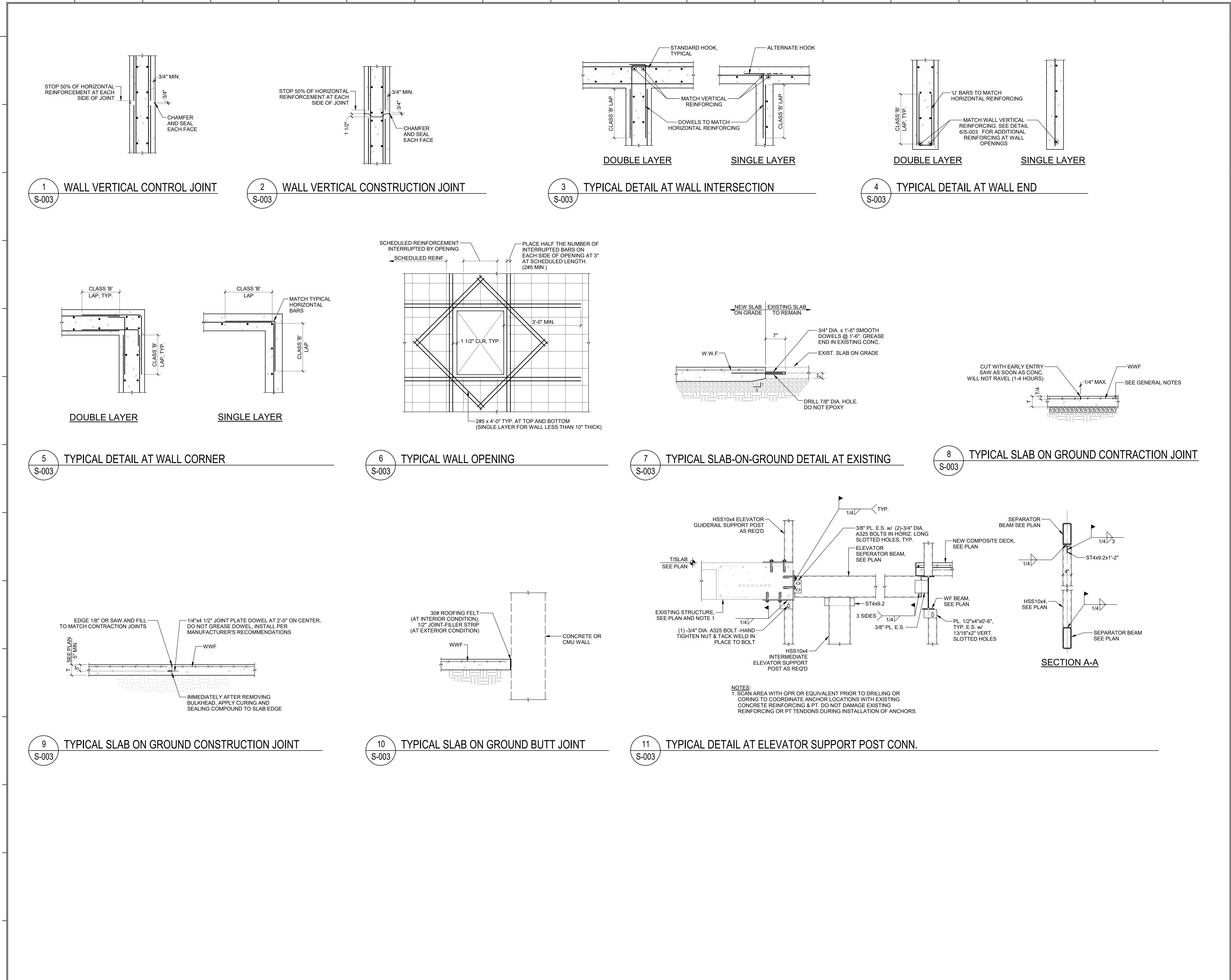
   HILTI KWIK HUS-EZ (ICC-ES ESR 3056)
  - DEWALT SCREW-BOLT+ (ICC-ES ESR-4042)
  - SIMPSON STRONG-TIE TITEN HD (ICC-ES ESR 1056)
- 9.12 ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC58. ACCEPTABLE ADHESIVE ANCHORS FOR USE IN SOLID-GROUTED CONCRETE MASONRY INCLUDE THE FOLLOWING:
  - HILTI HIT-HY 270 (ICC-ES ESR 4143)
    DEWALT AC100+ GOLD ADHESIVE (ICC-ES ESR 3200)
  - SIMPSON STRONG-TIE AT-XP (IAPMO UES ER 281)
- 10.00 DEFERRED SUBMITTALS
- 10.01 DEFERRED SUBMITTALS INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING BUILDING COMPONENTS:
  - GLAZED SYSTEMS (INCLUDING BUT NOT LIMITED TO WINDOW UNITS, CURTAIN WALLS, AND STOREFRONTS THAT EXCEED TEN (10) FEET IN HEIGHT)
  - WOOD STRUCTURES
  - STRUCTURAL STEEL CONNECTIONS
    METAL STAIRS, TREADS, AND ASSOCIATED LANDINGS
  - GUARDRAILS AND HANDRAILS
  - METAL LADDERS, SHIPS' LADDERS, AND SAFETY CAGES
  - COLD FORMED METAL FRAMING
    MICROPILES AND THEIR CONNECTION TO PILE CAPS
- 10.02 SHOP DRAWINGS AND CALCULATIONS FOR THESE COMPONENTS SHALL BE SUBMITTED TO THE ARCHITECT UNLESS NOTED OTHERWISE. DEFERRED SUBMITTALS SHALL BE SEALED, SIGNED, AND DATED BY THE DELEGATED COMPONENT STRUCTURAL ENGINEER REGISTERED IN THE PROJECT STATE. REFER TO OTHER SECTIONS OF THESE GENERAL NOTES FOR SPECIFIC REQUIREMENTS OF DEFERRED SUBMITTALS. REVIEW OF DEFERRED SUBMITTALS SHALL BE FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND SHALL NOT RELIEVE THE CONTRACTOR AND ITS ENGINEER OF THE FULL RESPONSIBILITY FOR DESIGN. DO NOT BEGIN FABRICATION UNTIL DEFERRED SUBMITTALS ARE APPROVED.
- 10.03 DEFERRED SUBMITTAL BUILDING COMPONENTS SHALL ACCOMMODATE ALLOWABLE BUILDING DEFLECTIONS AND DRIFT AS SET FORTH BY THE BUILDING CODE AND CONSTRUCTION TOLERANCES AS DEFINED BY THE AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND ACI "SPECIFICATION FOR TOLERANCES FOR CONCRETE CONSTRUCTION AND MATERIALS". SEE SPECIFICATIONS FOR DESIGN CRITERI

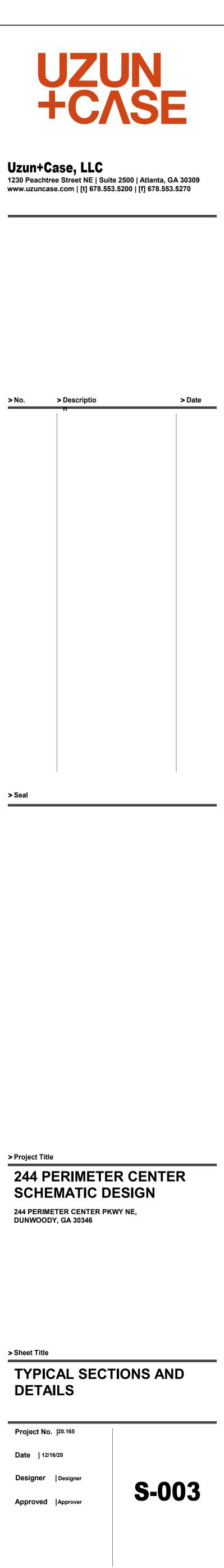




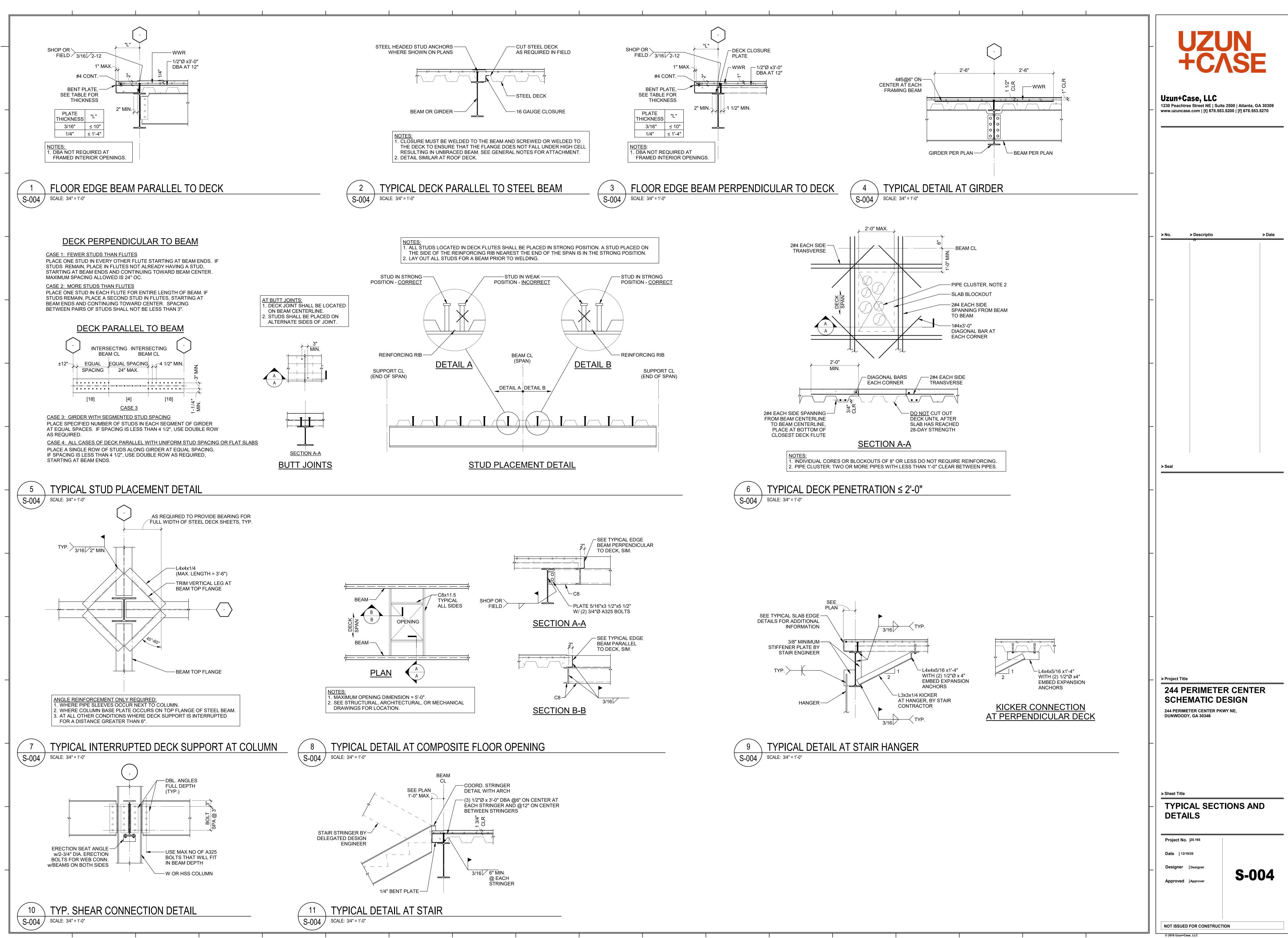
COMPONENTS AND CLADDING DIAGRAM 1 S-002

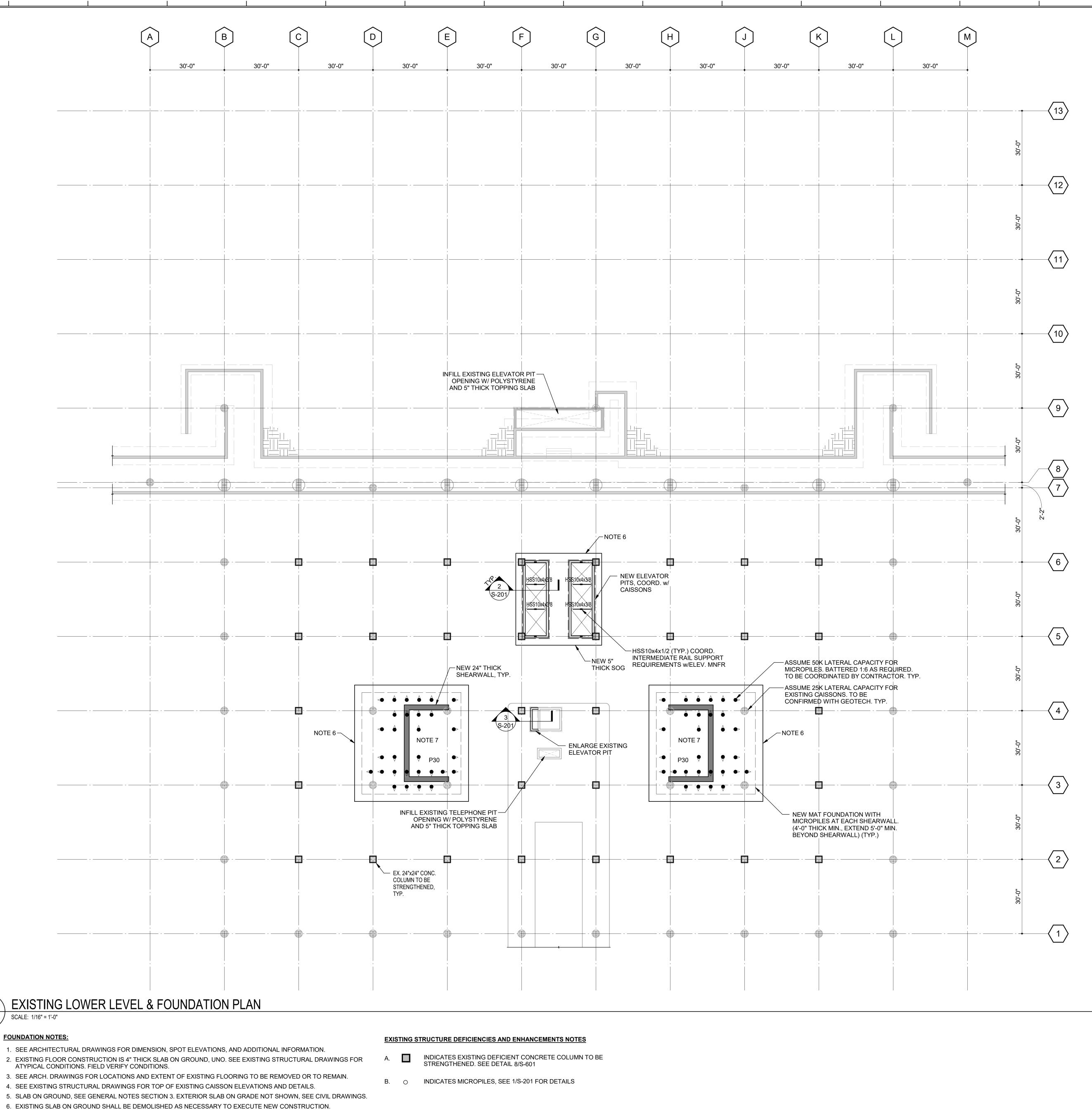


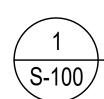




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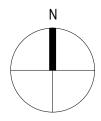


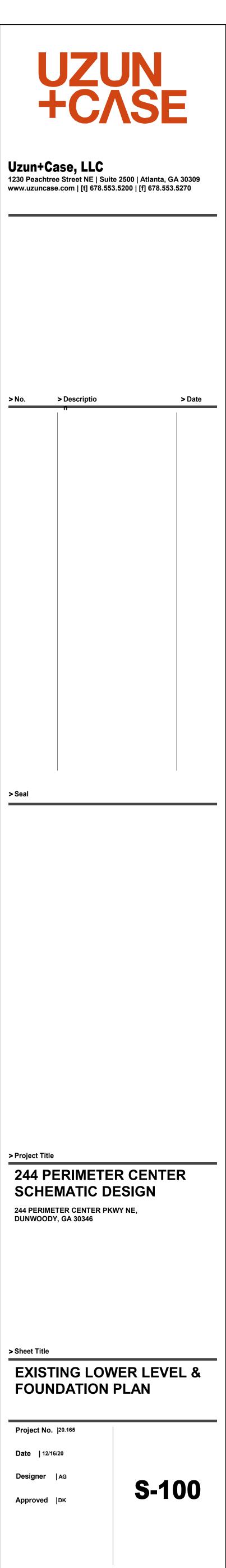


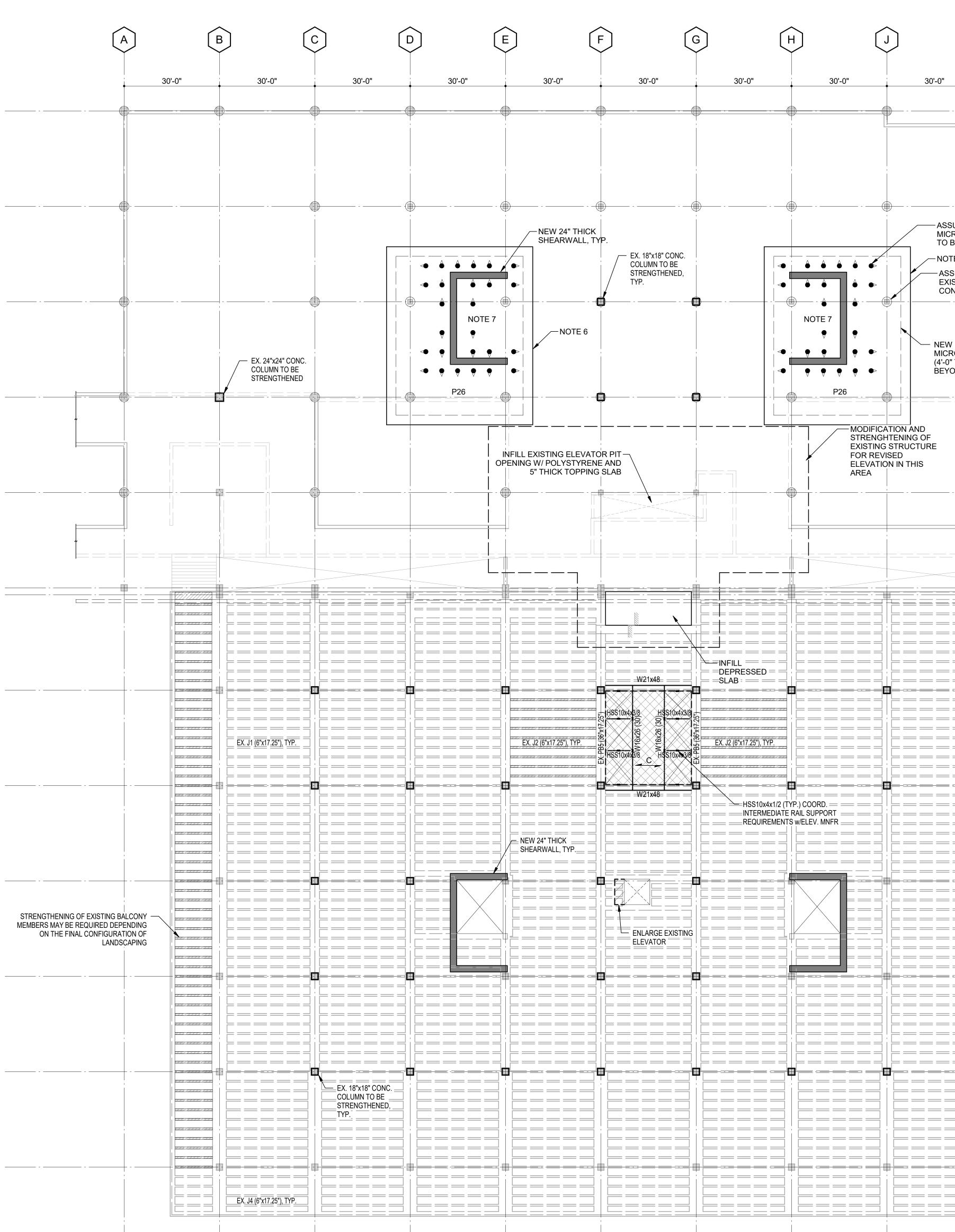
#### **EXISTING LOWER LEVEL & FOUNDATION PLAN** S-100 SCALE: 1/16" = 1'-0"

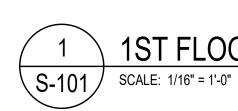
#### FOUNDATION NOTES:

- 7. DEMOLISH AND REPLACE EXISTING STAIR AS NECESSARY TO EXECUTE NEW CONSTRUCTION.









## 1ST FLOOR EXISTING PLAN

#### FOUNDATION NOTES:

- F1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSION, SPOT ELEVATIONS, AND ADDITIONAL INFORMATION.
- F2. EXISTING FLOOR CONSTRUCTION IS 4" THICK SLAB ON GROUND, UNO. SEE EXISTING STRUCTURAL DRAWINGS FOR ATYPICAL CONDITIONS. FIELD VERIFY CONDITIONS.
- F3. SEE ARCH. DRAWINGS FOR LOCATIONS AND EXTENT OF EXISTING FLOORING TO BE REMOVED OR TO REMAIN. F4. SEE EXISTING STRUCTURAL DRAWINGS FOR TOP OF EXISTING CAISSON ELEVATIONS AND DETAILS.
- F5. SLAB ON GROUND, SEE GENERAL NOTES SECTION 3. EXTERIOR SLAB ON GRADE NOT SHOWN, SEE CIVIL DRAWINGS.
- F6. EXISTING SLAB ON GROUND SHALL BE DEMOLISHED AS NECESSARY TO EXECUTE NEW CONSTRUCTION.
- F7. DEMOLISH AND REPLACE EXISTING STAIR AS NECESSARY TO EXECUTE NEW CONSTRUCTION.

#### PLAN FRAMING NOTES:

1. TOP OF NOMINAL EXISTING SLAB ELEVATION (SOUTH BUILDING) = 990'-0". TO BE FIELD VERIFIED.

2. TOP OF NOMINAL EXISTING SLAB ELEVATION (NORTH BUILDING) = 986'-0". TO BE FIELD VERIFIED.

3. EXISTING TYPICAL FLOOR CONSTRUCTION IS 3 1/4" THICK SLAB, SPANNING BETWEEN 17'-1/4" DEEP CONC. BEAMS & JOISTS. JOISTS ARE SUPPORTED BY POST-TENSIONED GIRDERS. ALL EXISTING FRAMING IS SEMI-LIGHTWEIGHT (120 PCF MAX.), UNO. EXISTING BEAMS & JOISTS ARE SPACED EQUALLY BETWEEN COLUMN LINES, UNO. SEE EXISTING

STRUCTURAL DRAWINGS FOR ATYPICAL CONDITIONS. FIELD VERIFY CONDITIONS. 4. - C INDICATES SPAN DIRECTION OF SLAB CONSTRUCTION AT NEW ELEVATOR LOBBY: 3" 20 GAUGE (MINIMUM) GALVANIZED COMPOSITE FLOOR DECK TOPPED WITH 3.25" SEMI-LIGHTWEIGHT (115 PCF)

CONCRETE, REINFORCED WITH 6x6-W2.9xW2.9 WWR (6 1/4" TOTAL). 5. ALL MEP SLEEVES AND PENETRATIONS TO BE SUBMITTED TO A/E FOR REVIEW PRIOR TO CUTTING, CORING, OR

DRILLING. NOT ALL MEP OPENINGS OR PENETRATIONS ARE SHOWN ON STRUCTURAL DRAWINGS. 6. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS, LOCATIONS OF DRAINS, SLOPES, AND CONTOURS OF AREAS SLOPED TO DRAIN NOT SHOWN. COORDINATE BEFORE POURING.

K L	M	
30'-0" 30'-0"		
	30'-0"	
SUME 50K LATERAL CAPACITY FOR CROPILES. BATTERED 1:6 AS REQUIRED.		
D BE COORDINATED BY CONTRACTOR. TYP. DTE 6 SSUME 25K LATERAL CAPACITY FOR XISTING CAISSONS. TO BE ONFIRMED WITH GEOTECH. TYP.	300	
W MAT FOUNDATION WITH	300"	
0" THICK MIN., EXTEND 5'-0" MIN. YOND SHEARWALL) (TYP.)	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	
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**EXISTING STRUCTURE DEFICIENCIES AND ENHANCEMENTS NOTES** 

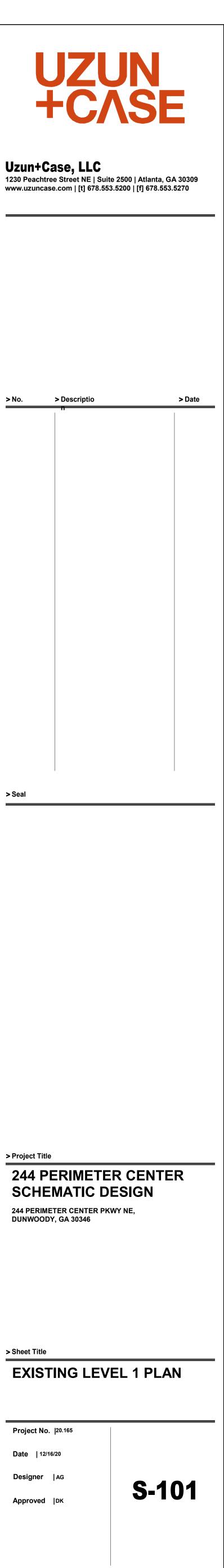
INDICATES DEFICIENT BEAM TO BE REINFORCED WITH FRP. SEE SHEET S7.01, STRENGTHENING SECTIONS, DETAILS AND

SCHEDULES FOR ADDITIONAL INFORMATION. .  $\square$  INDICATES EXTENTS OF EXISTING STRUCTURAL SYSTEM TO BE

DEMOLISHED FOR NEW CONSTRUCTION

INDICATES EXISTING DEFICIENT CONCRETE COLUMN TO BE STRENGTHENED. SEE DETAIL 8/S-601

D. O INDICATES MICROPILES, SEE 1/S-201 FOR DETAILS



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		<u>⊢</u> .	EX. PB7 (26"x17.25")	EX. J26 (6"x	17.25"), TYP.
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PLAN FRAMING NOTES:

- TOP OF NOMINAL SLAB ELEVATION = 1002'-6". TO BE FIELD VERIFIED.
   EXISTING TYPICAL FLOOR CONSTRUCTION IS 3 1/4" THICK SLAB, SPANNING BETWEEN 17'-1/4" DEEP CONC. BEAMS & JOISTS. JOISTS ARE SUPPORTED BY POST-TENSIONED GIRDERS. ALL EXISTING FRAMING IS SEMI-LIGHTWEIGHT (120 PCF MAX.), UNO. EXISTING BEAMS & JOISTS ARE SPACED EQUALLY BETWEEN COLUMN LINES, UNO. SEE EXISTING STRUCTURAL DRAWINGS FOR ATYPICAL CONDITIONS. FIELD VERIFY CONDITIONS.
- 3. C \_ INDICATES SPAN DIRECTION OF SLAB CONSTRUCTION AT NEW ELEVATOR LOBBY: 3" 20 GAUGE (MINIMUM) GALVANIZED COMPOSITE FLOOR DECK TOPPED WITH 3.25" SEMI-LIGHTWEIGHT (115 PCF) CONCRETE, REINFORCED WITH 6x6-W2.9xW2.9 WWR (6 1/4" TOTAL).
- ALL MEP SLEEVES AND PENETRATIONS TO BE SUBMITTED TO A/E FOR REVIEW PRIOR TO CUTTING, CORING, OR DRILLING. NOT ALL MEP OPENINGS OR PENETRATIONS ARE SHOWN ON STRUCTURAL DRAWINGS.
- 5. SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS, LOCATIONS OF DRAINS, SLOPES, AND CONTOURS OF AREAS SLOPED TO DRAIN NOT SHOWN. COORDINATE BEFORE POURING.

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				EX. 18"x18" CO COLUMN TO E STRENGTHEN TYP.						C	
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										30'-0"	
			EX. B43 (30"x17.								9
	2" EXISTING EX				DETAIL 4/S	DR INFILL OF EXISTING S-601, TYP. COORD. LC	CATIONS WITH			30'-0"	
		EE ARCH.				P, AND FIELD VERIFY I					
								<pre>clipical control control</pre>		2:-2"	7
							EX. J26 (6"x EX. J26 (6"x EX. J26 (6"x	17.25°), TYP.		30,-0	
											6
		EX. J2 (6"x17.		510x4x3/8	), TYP.					30'-0"	
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				HSS HSS HSS HSS HSS HSS HSS HSS HSS HSS	0x4x1/2 (TYP.) COORD. RMEDIATE RAIL SUPPORT JIREMENTS w/ELEV. MNFR						
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EXISTING STRUCTURE DEFICIENCIES AND ENHANCEMENTS NOTES

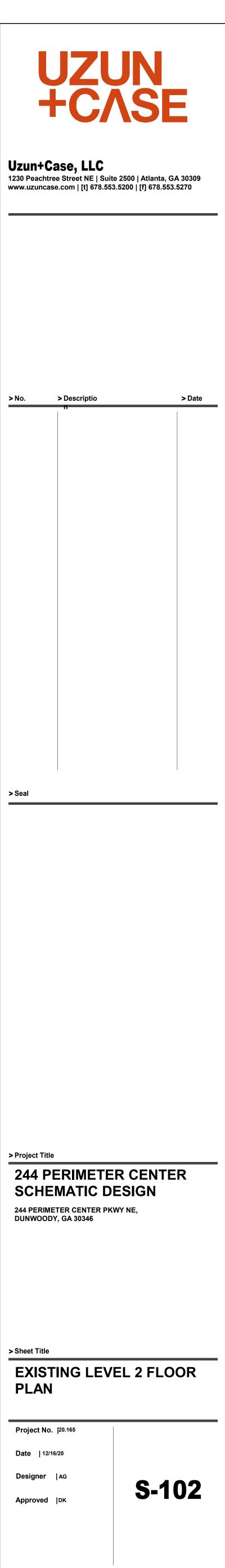
С

A. INDICATES DEFICIENT BEAM TO BE REINFORCED WITH FRP. SEE SHEET S7.01, STRENGTHENING SECTIONS, DETAILS AND

B. SCHEDULES FOR ADDITIONAL INFORMATION. B. DEMOLISHED FOR NEW CONSTRUCTURAL SYSTEM TO BE

INDICATES EXISTING DEFICIENT CONCRETE COLUMN TO BE STRENGTHENED. SEE DETAIL 8/S-601 N

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3. <u>C</u> INDICATES SPAN DIRECTION OF SLAB CONSTRUCTION AT NEW ELEVATOR LOBBY: 3" 20 GAUGE (MINIMUM) GALVANIZED COMPOSITE FLOOR DECK TOPPED WITH 3.25" SEMI-LIGHTWEIGHT (115 PCF) CONCRETE, REINFORCED WITH 6x6-W2.9xW2.9 WWR (6 1/4" TOTAL).

 ALL MEP SLEEVES AND PENETRATIONS TO BE SUBMITTED TO A/E FOR REVIEW PRIOR TO CUTTING, CORING, OR DRILLING. NOT ALL MEP OPENINGS OR PENETRATIONS ARE SHOWN ON STRUCTURAL DRAWINGS.
 SEE ARCHITECTURAL DRAWINGS FOR EDGE OF SLAB DIMENSIONS, LOCATIONS OF DRAINS, SLOPES, AND CONTOURS OF AREAS SLOPED TO DRAIN NOT SHOWN. COORDINATE BEFORE POURING.

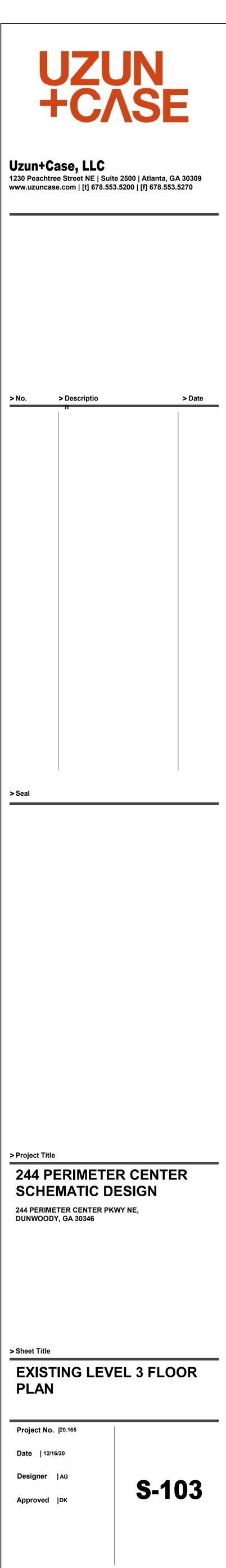
30'-0		30'-0"	E 30'-0"	F [ 30'-0"	G 30'-0"	H 30'-0"	J 30'-0"	30'-0"	30'-0"	
			NEW 24" THICK							300"
			SHEARWALL, TYP.		EX. 18"x18" CONC. COLUMN TO BE STRENGTHENED, TYP.					
										30-0"
				EX. B33 (30"x17.25")		PER DETAIL 4/S-6	ARCH., MEP, AND FIELD			300"
										30'-0"
			EX. J2 (6"x17.25"), TYP.	W21x48 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8 HSS10x4x3/8	396					300
			NEW 24" THICK SHEARWALL, TYP.		HSS10x4x1/2 INTERMEDIAT REQUIREMEN					
				! <u></u>						30-0
	CONC									
										300

EXISTING STRUCTURE DEFICIENCIES AND ENHANCEMENTS NOTES

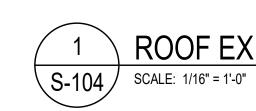
A. INDICATES DEFICIENT BEAM TO BE REINFORCED WITH FRP. SEE SHEET S7.01, STRENGTHENING SECTIONS, DETAILS AND SCHEDULES FOR ADDITIONAL INFORMATION.

B. STRUCTURAL SYSTEM TO BE DEMOLISHED FOR NEW CONSTRUCTION

C. INDICATES EXISTING DEFICIENT CONCRETE COLUMN TO BE STRENGTHENED. SEE DETAIL 8/S-601



A	В	
30'-0"		30'-0"
2" EXISTING EXPANSION JOINT, SEE ARCH		



## ROOF EXISTING PLAN

PLAN FRAMING NOTES:

1. TOP OF NOMINAL SLAB ELEVATION = 1029'-0". TO BE FIELD VERIFIED.

2. EXISTING TYPICAL FLOOR CONSTRUCTION IS 3 1/4" THICK SLAB, SPANNING BETWEEN 17'-1/4" DEEP CONC. BEAMS & JOISTS. JOISTS ARE SUPPORTED BY POST-TENSIONED GIRDERS. ALL EXISTING FRAMING IS SEMI-LIGHTWEIGHT (120 PCF MAX.), UNO. EXISTING BEAMS & JOISTS ARE SPACED EQUALLY BETWEEN COLUMN LINES, UNO. SEE EXISTING STRUCTURAL DRAWINGS FOR ATYPICAL CONDITIONS. FIELD VERIFY CONDITIONS.

3. \_\_\_\_\_C \_\_\_ INDICATES SPAN DIRECTION OF SLAB CONSTRUCTION AT NEW ELEVATOR LOBBY: 3" 20 GAUGE (MINIMUM) GALVANIZED COMPOSITE FLOOR DECK TOPPED WITH 3.25" SEMI-LIGHTWEIGHT (115 PCF)

CONCRETE, REINFORCED WITH 6x6-W2.9xW2.9 WWR (6 1/4" TOTAL). 4. ALL MEP SLEEVES AND PENETRATIONS TO BE SUBMITTED TO A/E FOR REVIEW PRIOR TO CUTTING, CORING, OR

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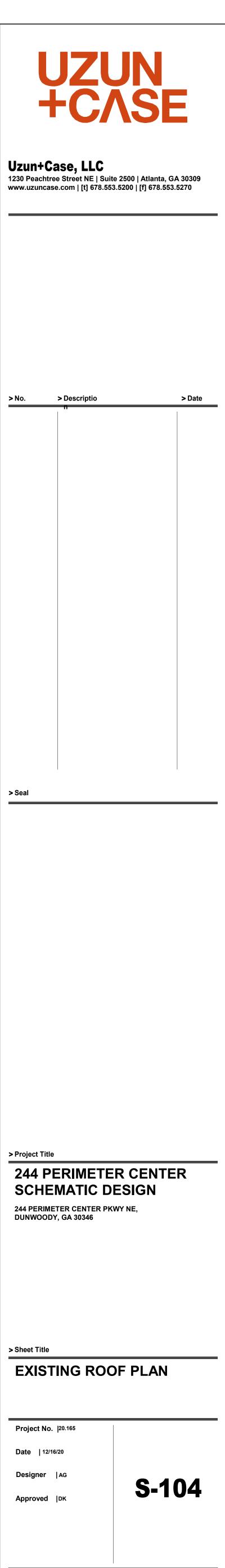
C	D	E F	G	H	L	K	L	M	
30'-						)" 30'-0"			
		EX. RJ2 (6"x17.25"), TYP.	EX. RJ2 (6'X						
				STRUCTURI	TING PENTHOUSE E ABOVE, FIELD STING CONDITIONS				90-0- 9
			EX: PB40 (26"x29.25")	INFILL EXIS <sup>®</sup> DETAIL 4/S-	TING ELEVATOR OVERF TING OPENING PER 601, TYP. COORD. LOC ., MEP, AND FIELD VERI ONDITIONS	ATIONS			30.0
			W21x48						300"
			H\$\$10x4x3/8 H\$\$10x4x3/8	17.25"), TYP.					
		NEW 24" THICK SHEARWALL, TYP.		HSS10x4x1/2 (TYP.) COORD. INTERMEDIATE RAIL SUPPORT REQUIREMENTS w/ELEV. MNFR					
		EX. JOIST (6"x17.25"), TYP.	ENLARGE EXISTING ELEVATOR	x17.25"), TYP.					4
									1 

EXISTING STRUCTURE DEFICIENCIES AND ENHANCEMENTS NOTES

A. INDICATES DEFICIENT BEAM TO BE REINFORCED WITH FRP. SEE SHEET S7.01, STRENGTHENING SECTIONS, DETAILS AND SCHEDULES FOR ADDITIONAL INFORMATION.

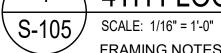
B. DINDICATES EXTENTS OF EXISTING STRUCTURAL SYSTEM TO BE DEMOLISHED FOR NEW CONSTRUCTION

C. INDICATES EXISTING DEFICIENT CONCRETE COLUMN TO BE STRENGTHENED. SEE DETAIL 8/S-601



(	A	В	c			F	G (H		
	30'-0"	30'-0"	30'-0"	30'-0"	30'-0"	30'-0"	30'-0"	30'-0"	30'-0"
	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)
	W16x26 (20) c=1"	(1) W16x26 (20) c=1" (1) S <sup>2</sup> (1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	W16x26 (20) c=1"	(11) W16x26 (20) c=1" (11) (11) (11) (11) (11) (11) (11) (11)	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1" (£ 5)	W16x26 (20) c=1" (1)	W16x26 (20) c=1
	W16x26 (20) c=1"	W16x26 (20) c=1" 4x25 X24x55 X24x55	W16x26 (20) c=1"	W16x26 (20) c=1" W16x26 (20) c	W16x26 (20) c=1" \$	W16x26 (20) c=1" ¥6	W16x26 (20) c=1" 452 X 757 X 7	W16x26 (20) c=1" 45 X X X X X X X X X X X X X X X X X X	W16x26 (20) c=1
	W30x	148 (118) c=2"	W10x54 COLUMN W16x26 (20) c=1"	W18x40 (20)	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W18x40 (20)	W16x26 (20) c=1
17 F.J.	W16x26 (20) c=1"	Image: Wight of the second s	W16x26 (20) c=1"	Image: W14x22 (10)         W14x22 (10)           Image: W14x22 (10)         W14x22 (10)	(12) W16x26 (20) c=1" (12)	W16x26 (20) c=1"	W16x26 (20) c=1" (2)	X	( <u>−</u> W16x26 (20) c=1
	W16x26 (20) c=1"	W16x26 (20) c=1" W16x26 (20) c	W16x26 (20) c=1"	V14x22 (10) W14x22 (10) W14x22 (10)	W16x31 (20) c=3/4"	W16x31 (29) c=3/4"	W16x31 (20) c=3/4"	W14x22 (10) W14x22 (10)	1) 52 W16x26 (20) e=1
	W30x	148 (118) c=2"	W16x26 (20) c=1"		W16x31 (20) c=3/4"	W16x31 (29) c=3/4"	W16x31 (20) c=3/4" ि ए	W14x22 (10	W16x26 (20) c=1
Ŕ	W16x26 (20) c=1"	<u>(1)</u> W16x26 (20) c=1" <u>(2)</u>	W16x26 (20) c=1"	<u> <u> </u> <u></u></u>	W16x31 (20) c=3/4" (	W16x31 (29) c=3/4"			₩16x26 (20) c=1
	W16x26 (20) c=1"	W24x55 (11, W24x55 (11, W24x5		Since     W16x26 (8)       W14x22 (10)     (8)       (8)     0	] W16x31 (20) c=3/4" [9] 93	W16x31 (29) c=3/4"	W16x31 (20) c=3/4" [ ©		(11) 55x45 M16x26 (20) c=1
	W18x35 (20)	W18x35 (20)	W16x31 (20)	₩16x31 (20)	₩ ₩16x26 (20) c=1"	W10x54 COLUMN	W10x54 COLUMN X	W18x35	W16x31 (20)
	₩18x35 (29)				W16x26 (20) c=1"	W16x31 (20) c=3/4" W16x31 (20) c=3/4"	W16x26 (20) c=1" W16x26 (20) c=1"	W16x31 (20)	
18 vef (0 0 0	W(18v35 (29)	W18X76 (9,9,0 M18X46		21x44 (7.5.7)	W16x26 (20) c=1" (115 (115 W16x26 (20) c=1" (25 W16x26 (20) c=1" (25) W16x26 (25) W16x26 (20) c=1" (25) W16x26 (25)	W 10x31 (20) c=3/4	W16X26 (20) C=1" (1 (2'2'1) 44X W16X26 (20) C=1" (2'2') 44X W16X26 (2'2') 44X W16X26 (2'2') C=1" (2'2') 44X W16X26 (2'2') 44X W16X26 (2'2') C=1" (2'2') 44X W16X26 (2'2')		
	W18x35 (29) W16x26 (20)	➤ W16x31		×	₩16x26 (20) c=1"		₩16x26 (20) c=1"		
	W16x31	 			₩16x26 (20) c=1" 🗧	W16x31 (20) c=3/4" ₩16x31 (20) c=3/4" 至			
	9780 W16x31	155 81 W16x31			W16x26 (20) c=1"	W16x31 (20) c=3/4"	W16x26 (20) c=1" W W51X26 (20) c=1" W		
	W16x31	W16x31	5 4 4 1		W16x31 (20)	W16x31 (20)	W16x31 (20)	·	
		W16x31	# W16x31	W16x31	W16x31 (20)	W16x31 (20)	W16x31 (20)	• W16x31	W16x31
	3" MINIMUM	W16x31 ද	W16x31	W16x31	(1)         W16x26 (20) c=1"         (2)           (2)         (2)         (2)         (2)           (2)         (2)         (2)         (2)	W16x26 (20) c=1"	W16x26 (20) c=1" $\widehat{\Sigma}$	W16x31	W16x31
	SEE ARCH.	₩16x31	W16x31	₩16x31	L) 6222 (20) c=1" 2222 W16x26 (20) c=1" 2222 CM	W16x26 (20) c=1"	W16x26 (20) c=1" KIX	₩16x31 Š	W16x31
		W16x31 (29)	W18x35 (20)	W18x35 (20)	W16x26 (20) c=1"	W18x40 (29)	W16x26 (20) c=1"	z1'-0" ₩18x35 (20) ÷	W18x35 (20)
		6) W18x35 (29) (1	W16x26 (20) c=1"	<u>()</u> <u>()</u> <u>()</u> <u>()</u> <u>()</u> <u>()</u> <u>()</u> <u>()</u>	W16x26 (20) c=1" ସି		B W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1
		W10x35 (29) 118x35 (29) W10x35	W16x26 (20) c=1"	W24x55 (11 M24x55 (50) c=1 W24x55 (11	W16x26 (20) c=1" W16x26 (20) c=1"	HSS10x4x3/8 C HSS10x4x3	W24X55 (11 M240 (07) 92X92W	W16x26 (20) c=1" K74x25 (20) c=1" K74x25 (20) c=1" K74x26	W16x26 (20) c=1
		W18x35 (20)	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"		W(16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1
		₩16x26 (20) c=1" (1 12 12 12 12 12 12 12 12 12 1	W16x26 (20) c=1"	(11) (11) (11) (11) (11) (11) (11) (11)	₩16x26 (20) c=1" €	W18x40 (29) W16x26 (20) c=1"		(TYP.) COORD. INTERMEDIATE EQUIREMENTS w/ELEV. MNFR W16x26 (20) c=1"	RAIL W16x26 (20) c=1
	W10 STEEL COLUMN	MV21X414 ( W16x26 (20) c=1" X24X25 V24X25 (10) X25 V24X25 (10) X24X25 V24X25 (10) X24X25 (10) X24X25 V24X25 (10) X24X25 (10) X24X25 V24X25 (10) X24X25 (10) X24X25 V24X25 (10) X24X25 (10) X24X25 (10) X24X25 V24X25 (10) X24X25 (10) X24X5 (10) X2	W16x26 (20) c=1"	24 x 22 (1 54 x 22 (1) 54 x 2	W16x26 (20) c=1"	W18x40 (20) c=3/4"	W16x26 (20) c=1"	W16x26 (20) c=1" 55	W16x26 (20) c=1
	TYP.	W16x26 (20) c=1"	W16x26 (20) c=1"	≥ ₩ W14x22 (10)	✓ NOTE 7 W14x22 ] W16x31 (29) c=3/4"	W14x22 (8) W14x22 (10)	W10x68 COLUMN W16x31 (29) c=3/4"	NOTE 7 ≥ W14x22 (10)	W16x26 (20) c=1
		<u>کر ایک ایک ایک ایک ایک ایک ایک ایک ایک ایک</u>		M24x68 (29)	W10x68 COLUMN	(01) 22XFIM (01) 2	W18x35 (29)	(62) 8974722 (10) W14x22 (10)	( <u>⊊</u> W16x26 (20) c=1
		W21x44 (7, M24x55 (11, W24x55 (11, W24x555	W18x35 (20)	11         W16x26 (8)           W14x22 (10)         W14x22 (10)	W18x35 (29) 901 801 801 801 801 801 801 801 801 801 8	W18x40 (29) U18x40 (29) W18x35 (29)	W18x35 (29)	W16x26 (8) W14x22 (10) W14x22 (10)	4 <sup>2</sup> (11) 924 W16x26 (20) c=1
		W16x26 (20) c=1"	W16x26 (20) c=1"		W16x31 (29) c=3/4"	W16x31 (29) c=3/4"	W16x31 (29) c=3/4"		W16x26 (20) c=1
		€ W16x26 (20) c=1" €	W16x26 (20) c=1"	₩16x26 (20) c=1" <u></u> ₩16x26 (20) c=1" <u></u>	W10x68 COLUMN			W16x26 (20) c=1" ₩16x26 (20) c=1" 至	W16x26 (20) c=1
		5 (11.5		4x55 (11,5,1 4x55 (11,5,1 4x55 (11,5,1 4x55 (11,5,1)	W16x26 (20) c=1" W16x26 (20) c=1" W16x26 (20) c=1" W16x26 (20) c=1"	W16x26 (20) c=1" W16x26 (20) c=1" W16x26 (20) c=1" ¥	W16x26 (20) c=1" W16x26 (20) c=1" W16x26 (20) c=1" Y4	W16x26 (20) c=1"	W16x26 (20) c=1
		W16x26 (20) c=1" W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1"	W16x26 (20) c=1
			W10x54 COLUMN		1			++	1
		W16x26 (20) c=1"		W16x26 (20) c=1"         (1)           'S'         'S'           'S'         'S'	W16x26 (20) c=1" (1) برم برج برج		W16x26 (20) c=1" (1)	W16x26 (20) c=1" (F)	W16x26 (20) c=1
		W16x26 (20) c=1" 43,112 W16x26 (20) c=1" 52,24,1 W16x26 (20) c=1" 52,24,1 W16x26 (20) c=1" 52,24,1	W16x26 (20) c=1"	W16x26 (20) c=1" X24x25 (20) C=1" X24x5 (20) C=1" X24x	W16x26 (20) c=1" \$	W16x26 (20) c=1" ¥	W16x26 (20) c=1" S3XF	W16x26 (20) c=1" (32%	W16x26 (20) c=1
		₩ ₩ ₩	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)	W16x31 (20)

4TH FLOOR PLAN



FRAMING NOTES:

1, [x'-x"] INDICATES TOP OF SLAB. TOP OF SLAB = +1032'-6" AT FINISHED FLOOR, UNLESS NOTED OTHERWISE.

TOP OF STEEL = -0'-6 1/4" (RELATIVE TO TOP OF SLAB), UNLESS NOTED OTHERWISE.

C INDICATES SPAN DIRECTION OF SLAB CONSTRUCTION: 3" 20 GAUGE (MINIMUM) GALVANIZED COMPOSITE FLOOR DECK TOPPED WITH 3.25" SEMI-LIGHTWEIGHT (115 PCF) CONCRETE, REINFORCED WITH 6x6-W2.9xW2.9 WWR (6 1/4" TOTAL).
 R INDICATES SPAN DIRECTION OF 3" 20 GAUGE (MINIMUM) GALVANIZED ROOF DECK

A. SPACE STEEL BEAMS EQUALLY BETWEEN COLUMNS, UNLESS NOTED OTHERWISE.

EDGE OF SLAB AT OPENINGS IS LOCATED 6" FROM THE BEAM CENTERLINE, UNLESS NOTED OTHERWISE.
 COLUMNS ARE W10X49 U.N.O.

COLOMINS ARE W 10249 U.N.O.
 BRACED FRAME FOR WOOD STRUCTURE ABOVE SHALL ALIGN WITH SHEARWALL.

7. BRACED FRAME FOR WOOD STRUCTURE ABOVE SHALL ALIGN W

8. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF DRAINS AND FLOOR SLOPE INFORMATION.
 9. COORDINATE ELEVATOR OPENING DIMENSIONS WITH PURCHASED EQUIPMENT.

<u> </u>	NUMBER OF 3/4"Ø x5" SHEAR STUDS, SEE TYPICAL STUD PLACEMENT DETAILS	TYPICAL 
н	CAMBER <u>W18x40 (44) c=1 1/2" XXK</u> H	

L REBAR LAYOUT: 2#4x5'-0" +HK @4", /──2#3x7'-6" @4", EACH SIDE EACH SIDE 2#4x5'-0" +HK @4", EACH SIDE HK = 180°, TYP. CORNER COLUMN EXTERIOR COLUMN

FRAMING LEGEND:

INDICATES 3" 20 GAUGE (MINIMUM) GALVANIZED ROOF DECK

LRFD BEAM

└── INDICATES BRACED FRAME,

SEE ELEVATIONS

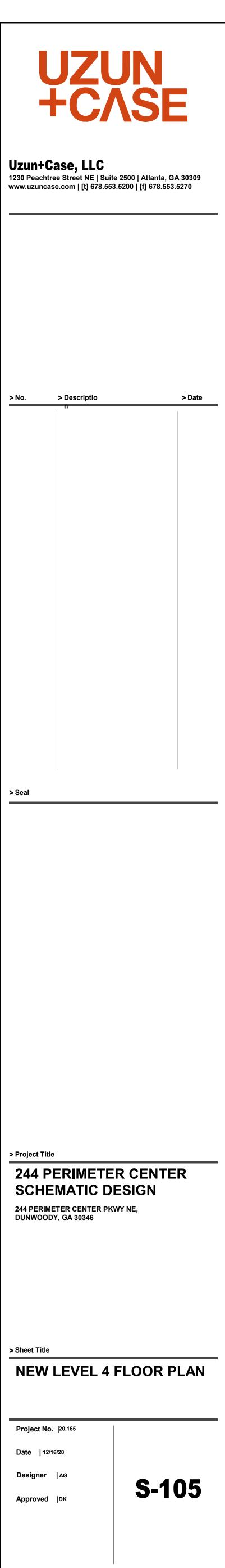
END REACTION

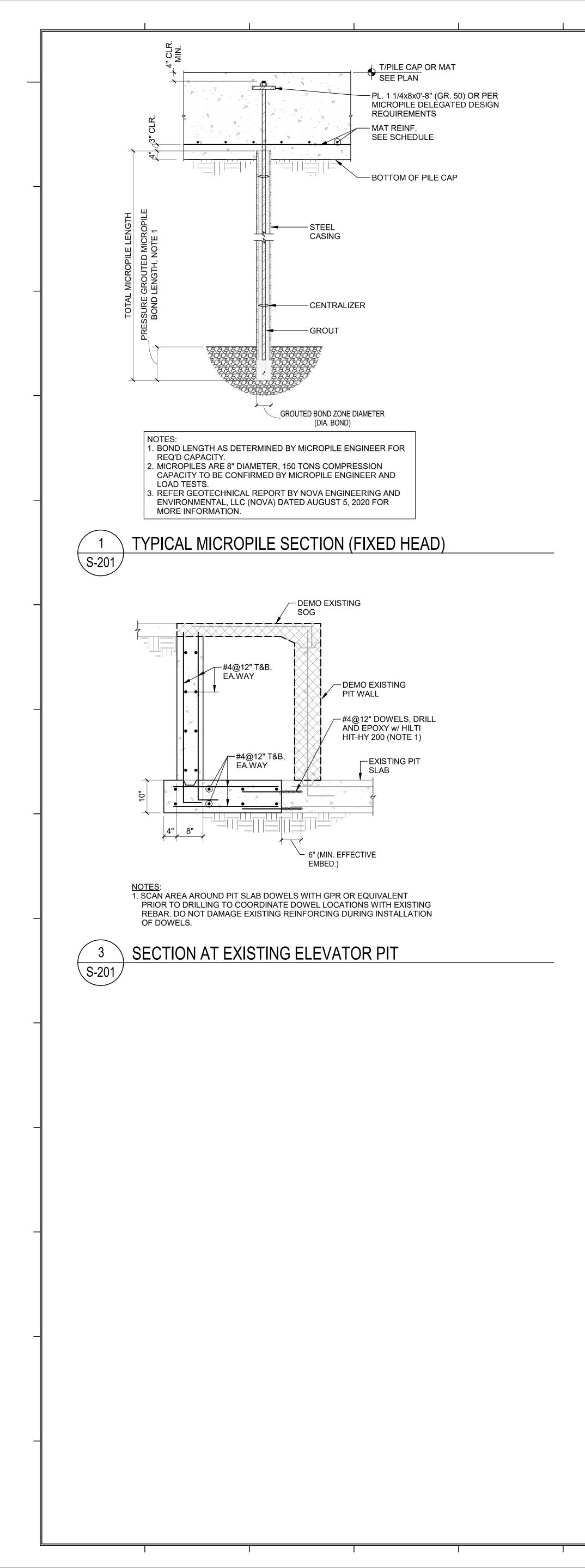
( H	<) (1	[N		
	30'-0" W16x31 (20)	30'-0" W16x31 (20)		$\frown$
I	+	· · · · · · · · · · · · · · · · · · ·		13
W24x55 (11,5,11)	W16x26 (20) c=1" (1 (1) W16x26 (20) c=1" (1) W16x26 (20) c=1" (1)W16x26	W16x26 (20) c=1" (2'5'2') W16x26 (20) c=1" (2'5'2') W16x26 (20) c=1" (2'5'2')	30'-0"	
W24x55	W16x26 (20) c=1" 434	W16x26 (20) c=1"	с Э́с	
c=1"ı	W16x26 (20) c=1"	W16x26 (20) c=1"	24" SHEAR WALL BELOW	12
1,5, <u>11</u> 1	W16x26 (20) c=1"	W16x26 (20) c=1"	BRACED FRAME FROM WOOD STRUCTURE ABOVE (TYP.)	
W24x55 (11,5,111)	W16x26 (20) c=1" K16x26	W10X20 (20) c=1 ( <u>(1,5,1)</u> W16X26 (20) c=1" ( <u>1,5,1)</u> W16X26 (20) c=1"	30'-0"	
> c=1"ı	W16x26 (20) c=1"	W16x26 (20) c=1"		11
c=1" =	W16x26 (20) c=1" 🔶	W16x26 (20) c=1"		
W24x55 (11,5,11)	W16x26 (20) c=1" (F (1) W16x26 (20) c=1" (F (2) W16x26 (20) c=1" (F) W16x26	W10X20 (20) C=1 ( <u>1</u> <u>5</u> W16x26 (20) C=1" ( <u>1</u> W16x26 (20) C=1" (	300"	
	W18x35 (20)	W18x35 (20)	·	10
W18x46	₩16x31 ₩16x31	W16x31	30'-0"	
W18	<u>W16x31</u>	₩16x31 <sup>99</sup>	30	
	W16x31	W16x31		9
	W16x31	₩16x31		
W18x46	W16x31	W16x31	30,-0"	
	W16x31	W16x31		8
	W18x35 (20)			
6 (9,9 <u>)</u>	W18x35 (29)	5 (9,9,9)	30'-0" 2'-2"	
W18x76.(	W18x35 (29)	W18x65 (	0C C	
) ) ) )				6
2 <sup>,</sup> 11 "1=2	W16x26 (20) c=1"	5,7)		
W24x55 (11,5,11)	W16x26 (20) c=1"	W21x44 (7,5,7)	30-0"	
c=1"	W16x26 (20) c=1"	5		
1	W16x26 (20) c=1"	1		5
W24x55 (11,5,11)		W21x44 (7,5,7)	30'-0"	
M24x5	W16x26 (20) c=1"			
c=1"ı	W16x26 (20) c=1"	WOOD COLUMN ABOVE, (TY NOT SHOWN FOR CLARITY		4
(1,5, <u>11)</u>	W16x26 (20) c=1"	(7,5,7)	5	
W24x55 (11,5,11)	W16x26 (20) c=1"	W21x44 (7,5,7)	30-0"	
< c=1"	W16x26 (20) c=1"	<b> </b>		3
c=1"	W16x26 (20) c=1"	(7,5,7)		
	W16x26 (20) c=1"	W21x44 (7,5,7)	30'-0"	
		24" SHEAR WALL BELOW BRACED FRAME FROM WO STRUCTURE ABOVE (TYP.)		
c=1"ı	W18x35 (20)			2
W27x84 (11,5,111)	W18x35 (29)	W18x65 (9,9,9)	30'-0"	
	W18x35 (29)	W18x6	30	
0)	71'-0" ////////////////////////////////////	<b>4</b>		1

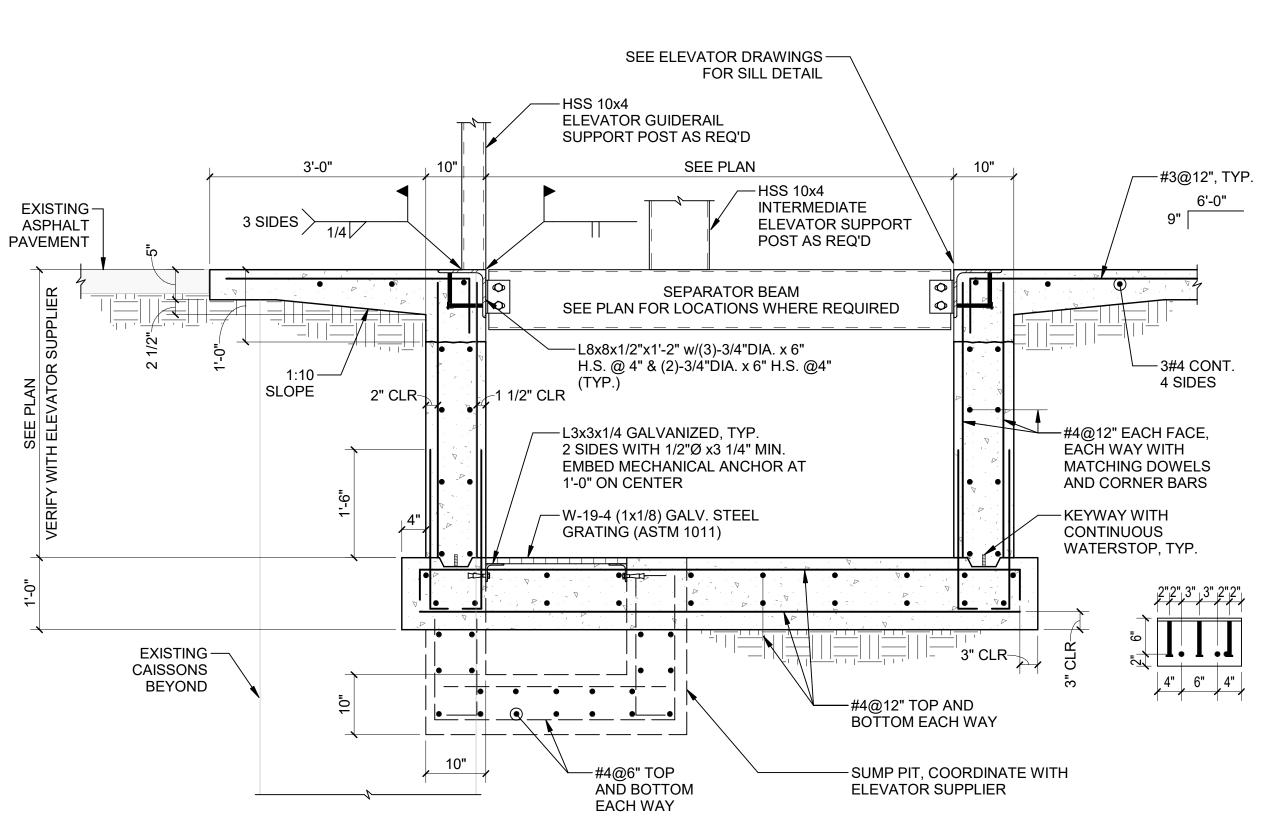
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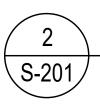
4", 4", 2#3x7'-6" @4", EACH SIDE, EACH WAY INTERIOR COLUMN

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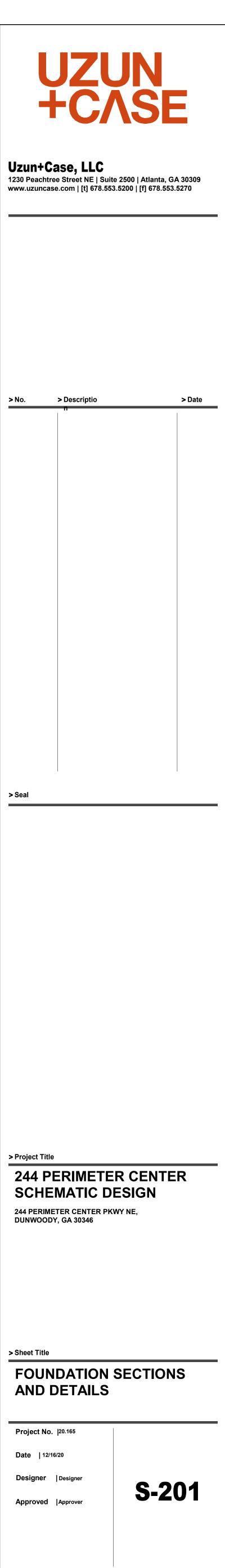




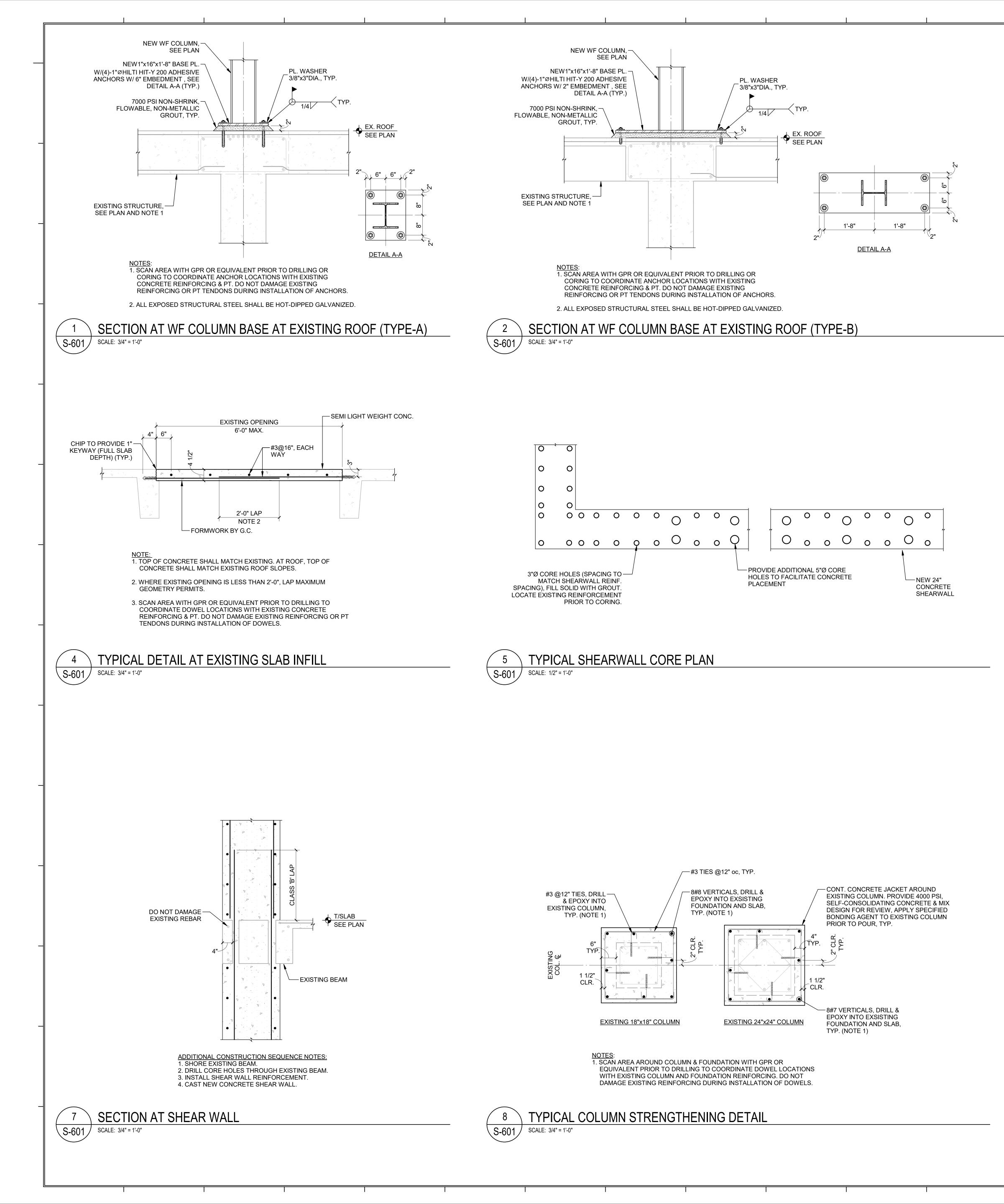


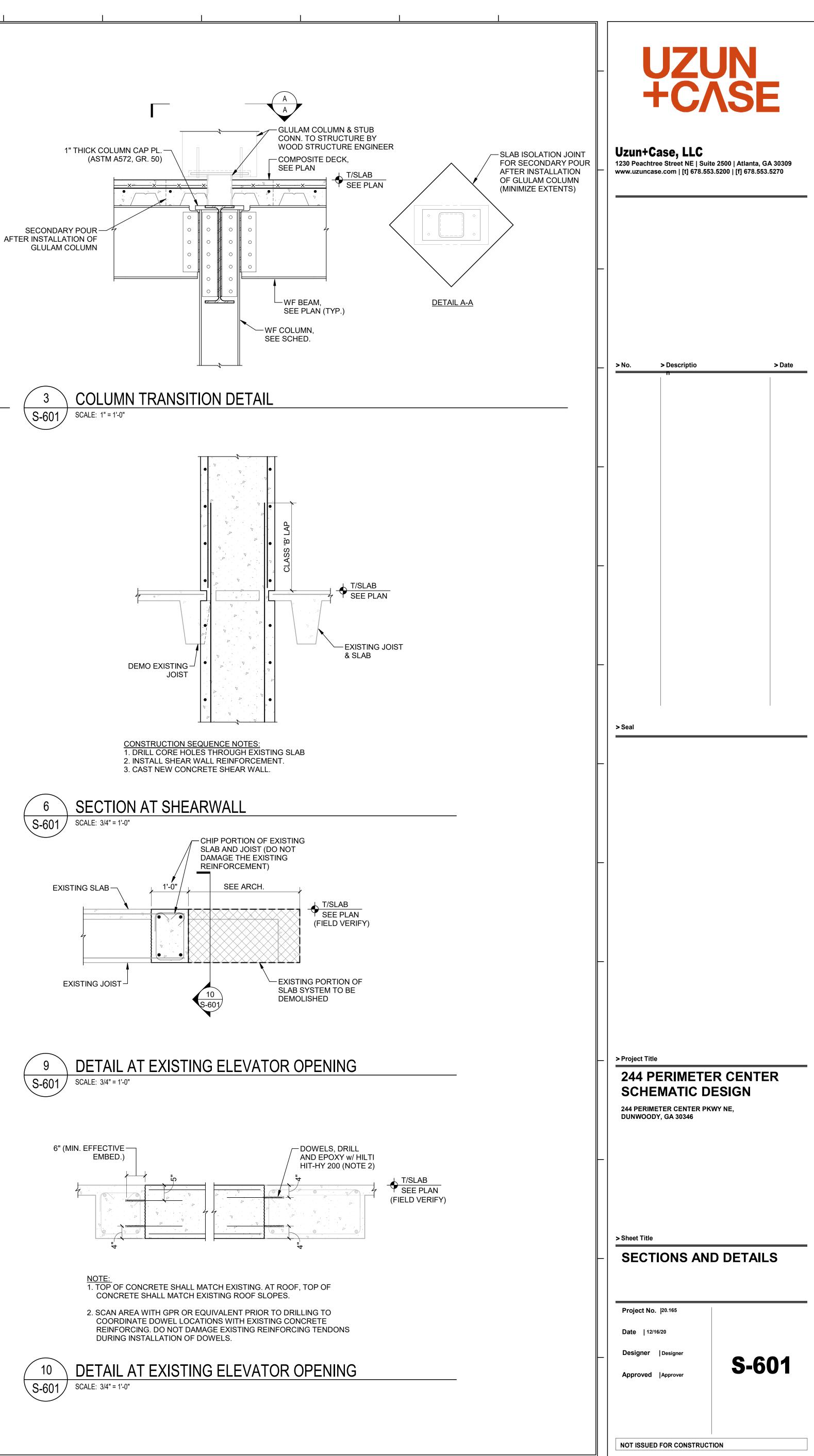


TYPICAL ELEVATOR PIT SECTION

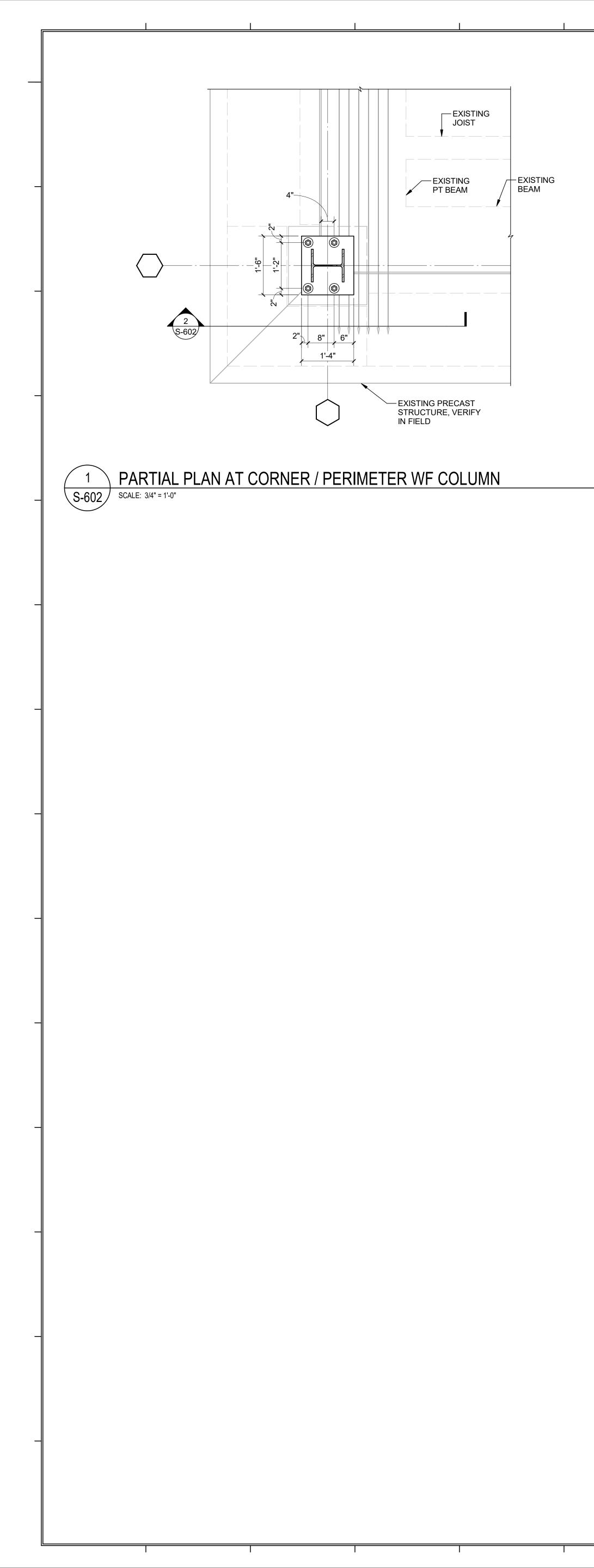


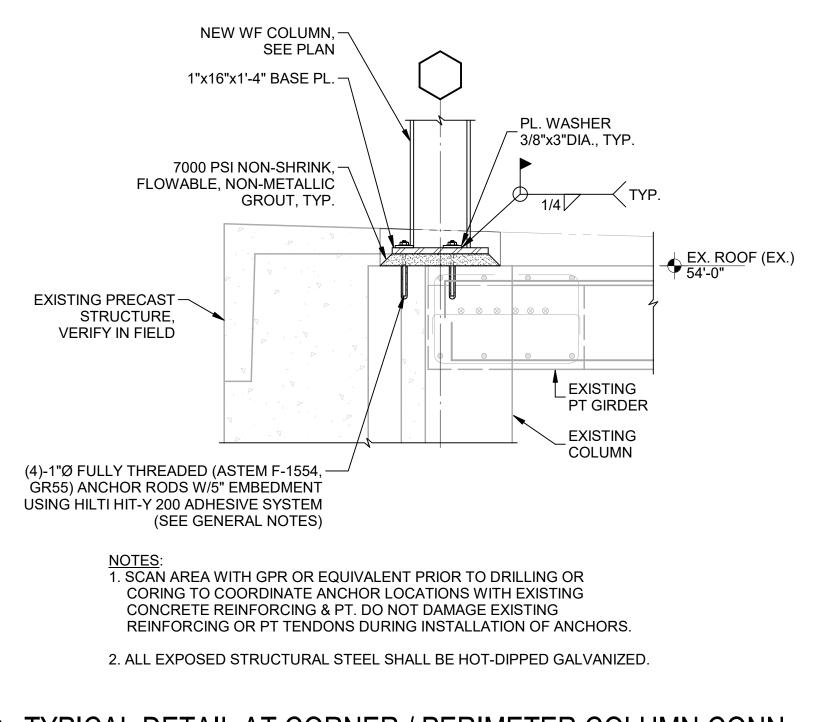
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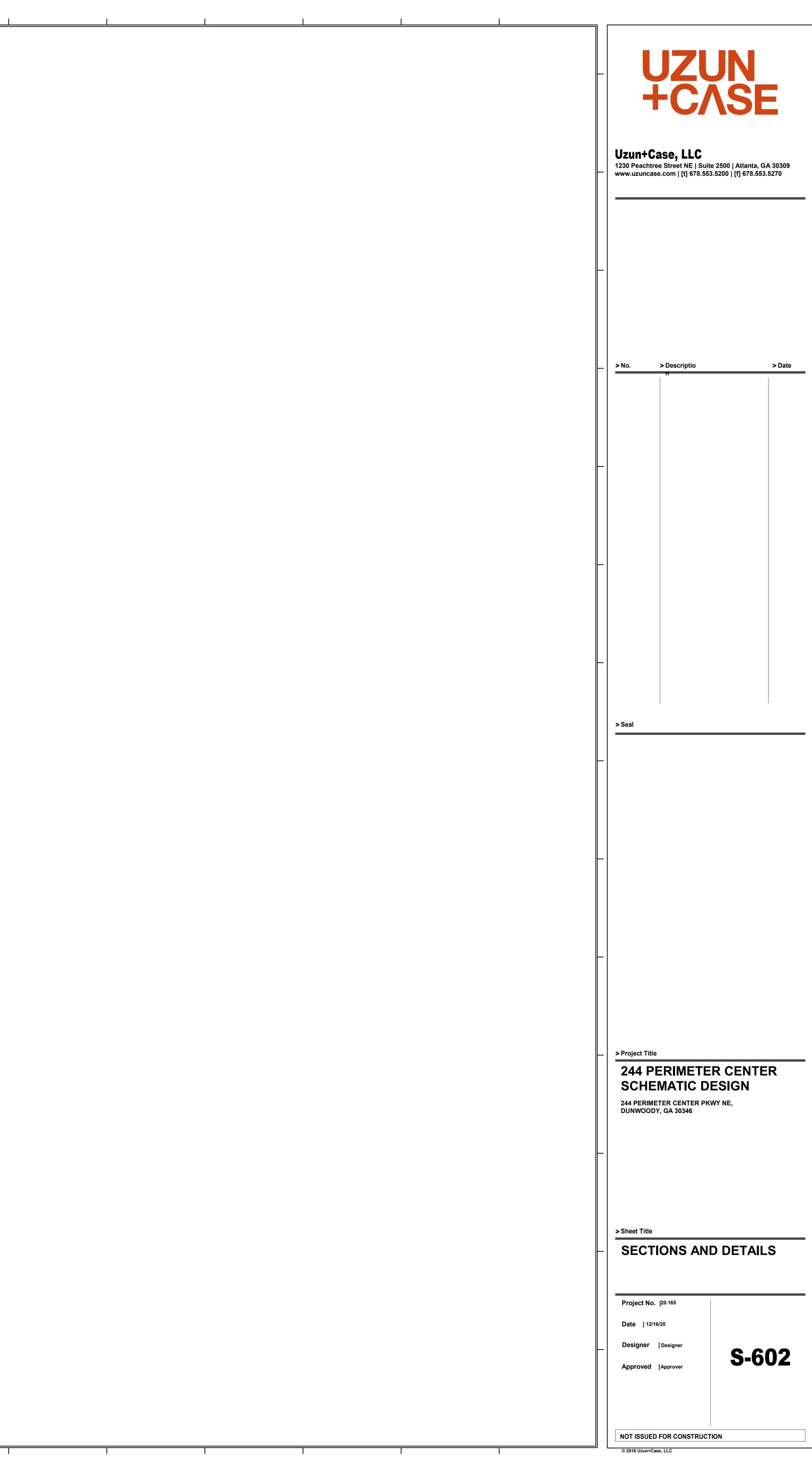


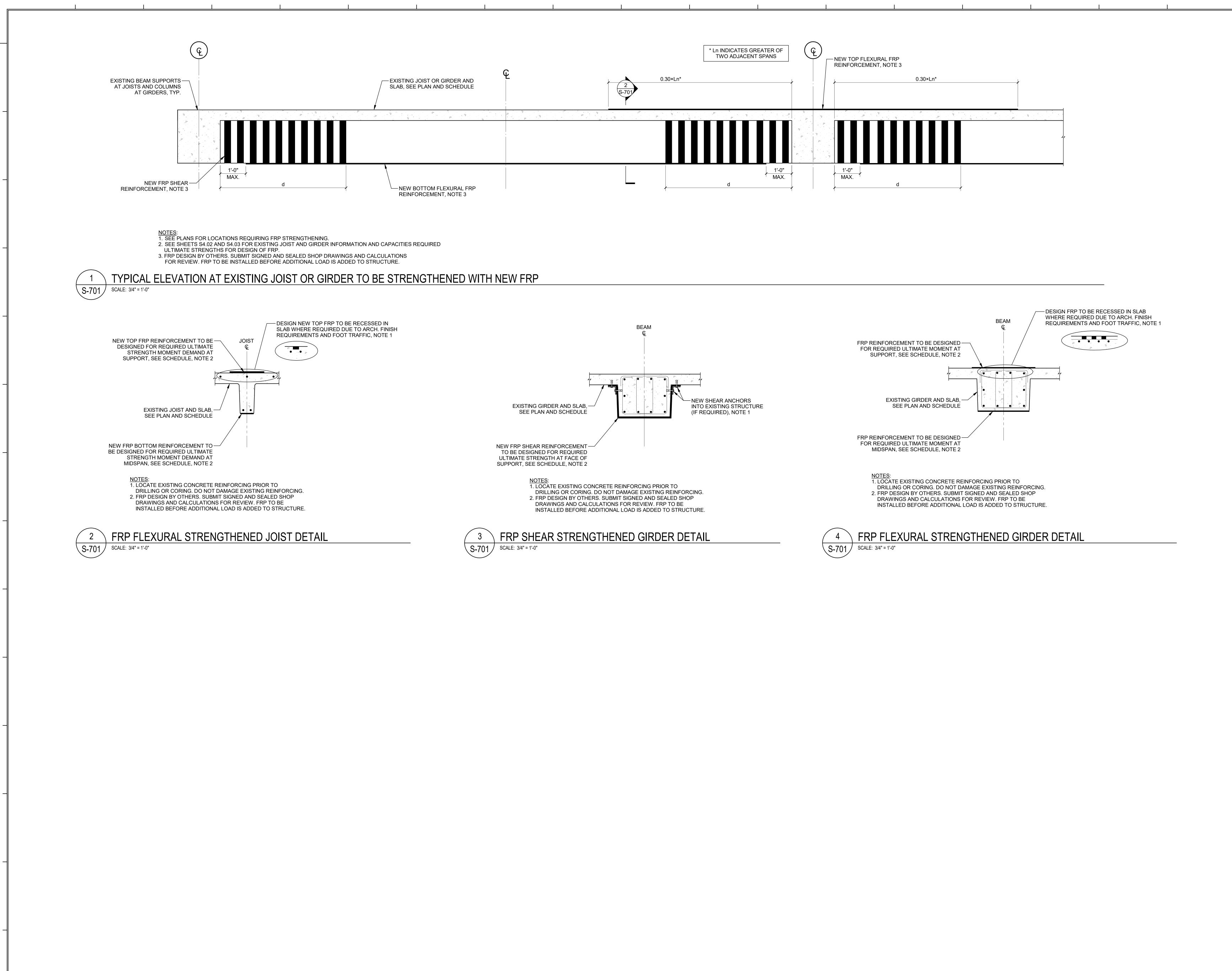
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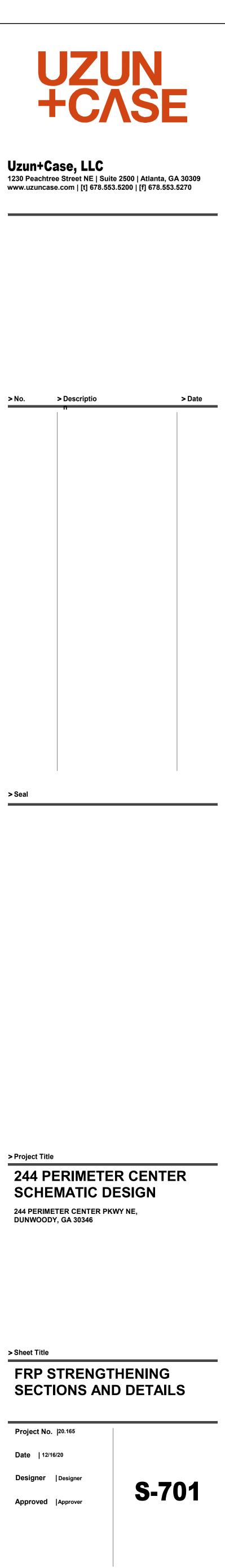




TYPICAL DETAIL AT CORNER / PERIMETER COLUMN CONN. 2 S-602 SCALE: 3/4" = 1'-0"







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