

STRUCTURAL GENERAL NOTES - IBC 2018

1. GENERAL
- Provide construction conforming to the 2018 International Building Code with the latest State Amendments. Reference to other standards, specifications, or codes means the latest standard or code published and adopted.
 - The structural general notes apply except where indicated otherwise on the drawings or in the specifications. A detail shown for one condition applies for all like or similar conditions even though not specifically indicated on the drawings.
 - Verify all existing conditions, dimensions, and elevations before starting work. Notify the Architect and Structural Engineer of Record in writing of any discrepancy.
 - The structure is able to resist design loads only when structural work is complete. During construction, the structure is not self-supporting. The Contractor is solely responsible for the design, adequacy, and safety of erection bracing, shoring, temporary supports, and all other means, methods, techniques, sequences, and procedures of construction.
 - Coordinate the structural contract documents with documents from architectural, mechanical, electrical, plumbing, civil, and all other consultants. Notify the Architect and Structural Engineer of Record in writing of any conflict and/or omission.
 - Coordinate and verify floor and roof opening sizes and locations with architectural, mechanical, plumbing, and electrical drawings. For additional openings not shown on the structural drawings refer to the architectural and mechanical drawings.
 - Review of the submittals and/or shop drawings by the Structural Engineer of Record is only for general conformance with the contract documents and does not relieve the Contractor of the responsibility to review and check shop drawings before submittal to the Structural Engineer of Record. The Contractor must review and stamp all submittals prior to submission. The Contractor remains solely responsible for errors and omissions associated with the preparation of shop drawings as they pertain to member sizes, details, and dimensions specified in the contract documents. Do not begin fabrication until shop drawings are completed and reviewed by the Structural Engineer of Record.
 - Electronic drawing files or model files will not be provided to the Contractor or subcontractors, unless agreed to otherwise for additional costs.
 - Do not make shop drawings using reproductions of the contract documents or referencing the contract documents.
 - Provide an allowance of 5% of all structural materials including labor to be fabricated and placed during progress of work as may be directed by the Structural Engineer of Record in addition to all structural materials indicated on the contract documents. Credit any unused quantity to the owner at the end of the project.

2. EXISTING CONDITIONS
- Renovation of existing structures requires thorough coordination of the contract documents with existing conditions. The Contractor must verify all relevant existing conditions, dimensions, and details prior to beginning construction. Report any deviations from conditions or dimensions shown on the contract documents to the Architect and Structural Engineer of Record for review of the condition and possible revision of the contract documents.
 - The nature of structural demolition and stabilization is inherently uncertain. The exact condition and capacity of each structural element cannot be verified prior to the commencement of work. As a result, it is imperative to report any discrepancies between the contract documents and actual field conditions, as well as any element of questionable structural integrity immediately to the Architect and Structural Engineer of Record for review.
 - No attempt has been made to define each specific structural element that must be removed, enhanced, or replaced. It is the responsibility of the Contractor to review the condition of individual elements, particularly rafters, joists, and structural deck boards, to determine which elements can be salvaged, which elements must be replaced, and which elements are questionable. The Contractor should consult with the Architect and Structural Engineer of Record to determine the appropriate procedure for handling elements in questionable condition.
 - Dimensions of or to existing elements shown on design drawings may not be accurate to necessary construction tolerances. Contractor to verify conditions in field and coordinate with design drawings, particularly for elements that will be fabricated off-site.

3. REINFORCED CONCRETE
- Provide reinforced concrete conforming to the following standards:
 - ACI 308-14, Specifications for Structural Concrete for Buildings
 - ACI 318-14, Building Code Requirements for Structural Concrete
 - ACI 302.1R-15, Guide for Concrete Floor and Slab Construction
 - ACI 308R-10, Guide to Design of Slabs-on-Ground
 - Unless noted otherwise, provide concrete with the following minimum Exposure Classes, Type, minimum 28-day compressive strengths, and maximum water-to-cementitious materials ratio:

Location	Exposure Class	Type	Strength	w/cm
Foundations	F0 S0 P0 C1	Normal	3000 PSI	0.55
Ext. Slabs on Grade	F2 S0 P0 C1	Normal	4500 PSI	0.45
Int. Slabs on Grade	F0 S0 P0 C0	Normal	3000 PSI	0.55
 - For Exposure Classes F1, F2, and F3, provide entrained air by volume in concrete based on ACI 318 Table 4.4.1, shown below:

Nominal Maximum Aggregate Size, in.	Exposure Class F1	Exposure Class F2 and F3
3/8	6	7.5
1/2	5.5	7
3/4	5	6
1	4.5	6
1 1/2	4.5	5.5
2	4	5
3	3.5	4.5
 - Fully document and submit for review the proposed materials and mix design for all concrete. The Contractor is responsible for obtaining the required design strength. All concrete test data must be available at the job site.
 - The use of calcium chloride, chlorides ions, or other salts is not permitted.
 - Place concrete at a slump of 5" ± 1".
 - Unless noted otherwise, provide construction or contraction joints in slabs-on-grade such that the maximum area between joints does not exceed 225 square feet with the length not exceeding twice the width.
 - The location of construction joints requires the approval of the Structural Engineer of Record. Unless noted otherwise, thoroughly roughen by mechanical means and clean construction joints.
 - Chamfer or round all exposed corners a minimum of 3/4".
 - Detail concrete reinforcement according to ACI SP-16 detailing manual. Submit shop drawings for approval, showing all fabrication dimensions and locations for placing concrete reinforcing and accessories. Do not begin fabrication until shop drawings are completed and reviewed by the Structural Engineer of Record. Unless specifically approved otherwise, detail all concrete walls and beams in elevation.
 - Unless noted otherwise, provide reinforcing steel conforming to ASTM A615, Grade 60.
 - Provide welded wire fabric mesh in flat sheets conforming to ASTM A1064. Rolls are not permitted. Lap welded wire fabric a minimum of 6' at each splice.
 - Fiber reinforcing may be substituted for welded wire fabric in slabs on-grade with the approval of the Structural Engineer of Record. Provide macro fiber reinforcing conforming to ASTM C1116, Type III, Use Sibra 90/40, Forta Ferro, or Fibermesh 650. 100% virgin polypropylene fibrillated fibers as directed by the fiber reinforcing manufacturer with approval of the Structural Engineer of Record.
 - All reinforcing steel and embedded items securely in place prior to placing concrete. Provide sufficient supports to maintain the position of the reinforcement within specified tolerances during all construction activities. "Sticking" dowels, anchor rods, or other embedded items into wet concrete is not permitted.
 - Lap concrete reinforcing as shown in the "Concrete Lap Splice Length Schedule."
 - The placement of all reinforcing steel must be reviewed by a Professional Engineer registered in the Project State or by a representative responsible to him per ACI 318, 1.3.1.
 - Unless noted otherwise, provide the following concrete cover on all reinforcing steel:

Concrete against earth (not formed):	3"
Formed concrete exposed to earth or weather:	#5 bars and smaller: 1 1/2"
 - Do not place pipes or ducts with a maximum dimension exceeding one-third the slab or wall thickness within the slab or wall unless specifically shown and detailed on the structural drawings.
 - Do not weld or tack weld reinforcing steel unless approved or directed by the Structural Engineer of Record. Provide reinforcing steel conforming to ASTM A706, Grade 60 where welding is approved or directed.

4. STRUCTURAL STEEL
- Provide structural steel detailing, fabrication, and erection conforming to the following standards:
 - AISC 303-05, Code of Standard Practice for Structural Steel Buildings and Bridges
 - AISC 325-17, Steel Construction Manual, 14th Edition
 - AISC 326-09, Detailing for Steel Construction, 3rd Edition
 - AISC 360-16, Specification for Structural Steel Buildings
 - AWS D1.1-10, Structural Welding Code - Steel
 - Provide steel shapes made of material conforming to the following standards, unless noted otherwise:

Wide Flange Shapes and WT Shapes	ASTM A992
Angles and Channels	ASTM A36
Plates	ASTM A572, Grade 50
Hollow Structural Sections (HSS)	ASTM A500, Grade C
Steel Pipe	ASTM A53, Type E or S, Grade B
Headed Studs	ASTM A108, Grade 1010 - 1020
Deformed Bar Anchors	ASTM A1064
 - Unless noted otherwise, install connections so they are snug-tight.
 - Unless noted otherwise, provide anchors for post-installed conditions in concrete that conform to manufacturer's requirements. Install connections so they are pre-tensioned per manufacturer's specifications.
 - Unless noted otherwise, make all connections with 3/4" minimum diameter ASTM F3125 Grade A325 bolts with threads included in the shear plane. Unless noted otherwise, install connections so they are pre-tensioned. Calibrate tightening methods used so they are in conformance with Section 8, Installation and Tightening of RCSC "Specification for Structural Joints using High Strength Bolts", 2009. Use one of the following tightening methods: calibrated wrench, direct tension indicator, or alternative bolt with twist off element. Do not use turn of the nut method, unless continuously observed by testing agency.
 - Make all welded connections in accordance with AWS D1.1, using type E70XX electrodes. Use only certified welders. Proof of certification must be maintained at the job site.
 - Submit shop drawings prepared in accordance with AISC 308. Provide complete welding information using AWS symbols. Use prequalified welded joints per the Steel Construction Manual and AWS D1.1. Do not begin fabrication until shop drawings are completed and reviewed by the Structural Engineer of Record.
 - The design of special connections between steel framing components (including but not limited to braced end connections, moment-resisting connections, modified beam seat connections, and member splice connections) not designed by the Structural Engineer of Record must be performed by a Professional Engineer registered in the Project State.
 - Do not use gas cutting torches to correct fabrication errors in structural steel framing.
 - Provide temporary bracing for structural steel framing until all permanent bracing, moment connections, and floor/roof decks (diaphragms) are completely installed.
 - Paint structural steel in accordance with the project specifications. Do not paint steel surfaces to be encased in concrete, surfaces to receive pre-painting, connections designated as friction type, surfaces to be welded, or surfaces receiving welded studs or DDA in the field.
 - Unless noted otherwise by the Architect, consider steel that is exposed to the public to be Architecturally Exposed Structural Steel (AESS) and should be fabricated per those requirements.

5. COLD FORMED STEEL FRAMING
- Provide cold formed steel framing detailing, fabrication, and erection conforming to the following standards:
 - AISI S100, North American Specification for the Design of Cold-formed Steel Structural Members
 - AISI S200, North American Standard for Cold-formed Steel Framing - General Provisions
 - AWS D1.1-15, Structural Welding Code - Steel
 - AWS D1.3-18, Structural Welding Code - Sheet Steel
 - Provide studs, runner track, and associated accessories of the type and thickness indicated on the drawings or as recommended by the manufacturer for the indicated application.
 - Provide studs, joists, runner track, and accessories manufactured of hot dip galvanized ASTM A1003 steel with the following yield strength:

33,000 PSI	33-mil and 43-mil members
50,000 PSI	54-mil, 68-mil, and 97-mil members
 - Provide minimum #12 (S-12) cadmium or zinc coated screws.
 - Install all cold-formed steel framing in accordance with the manufacturer's recommendations.
 - Provide reinforcement for any member cut for the installation of plumbing or wiring such that the member is of equal strength to the member prior to cutting.
 - Provide lateral bridging in load bearing walls consisting of 1 1/2" cold-rolled channels at 4'-0" on center vertically. Insert channels through stud web holes and screw or weld to each stud using 1 1/2" x 1" x 54-mil (16 gage) clip angles. Provide clip angles 1/4" less than the stud width.
 - Provide joist bridging at 8'-0" on center maximum. Install joist bridging in accordance with the manufacturer's recommendations.
 - Provide un-punched material for all joints and headers.

- Provide solid blocking behind all horizontal panel joints of exterior wall sheathing and interior shear wall sheathing.
 - Design of exterior walls subject to wind pressures is based on lateral bracing of stud flanges provided by sheathing. If sheathing is not applied to both faces of the wall, provide bracing in accordance with SPSA Technical Note No. 2, published March 2000.
 - Provide curtain wall framing installation to accommodate vertical displacement of the primary structure.
 - Submit shop drawings showing the size of all members and all connections sealed by a Professional Engineer registered in the Project State. Review of shop drawings by the Structural Engineer of Record is for general conformance with the contract documents regarding arrangement and sizes of members and the Contractor's interpretation of the design loads and contract document details.
6. OTHER COMPONENTS
- Provide shop drawings for construction of all applicable specialty items including but not limited to concrete pilings, curtain wall glazing systems, light gauge steel framing, ornamental guardrails, guards, handrails, pre-engineered wood trusses, joists, and signage. Shop drawings must indicate the required materials, sizes, and locations for all posts and pickets including anchorage at the base of the posts. Shop drawings must be sealed by a Professional Engineer registered in the Project State.
 - Provide curtain wall glazing system shop drawings that clearly indicate the attachment to the structure on all sides of the exterior glazing system required to adequately resist the applicable wind design pressures.
 - The Glazing Contractor must provide engineering calculations to document compliance with 2012 International Building Code, Sections 2403.2 through 2403.4 for butt-jointed glazing.

7. FOUNDATIONS
- The design of foundations, retaining walls, and slabs-on-grade is based on the following presumed criteria:

Allowable Soil Bearing Pressure	2000 PSF
Equivalent Lateral Fluid Pressure - Active Case	40 PSF/FT
Equivalent Lateral Fluid Pressure - At-Rest Case	60 PSF/FT
Equivalent Lateral Fluid Pressure - Passive Case	150 PSF/FT
Coefficient of Sliding Friction	0.25
Soil Density	110 PCF

 Redesign of foundations may be required if the actual conditions are different than the values listed above. The following conditions could also result in redesign of foundations: presence of expansive soils, high water table, potential for large settlements, or any other recommendations stated by a Geotechnical Engineer.
 - A Geotechnical Engineer must verify the condition and/or adequacy of all subgrades, fills, and backfills prior to the placement of foundations, footings, slabs, walls, etc.
 - If any interference appears between existing foundations and the specified design, notify the Architect so that the foundations may be redesigned as required.
 - Coordinate top of footing elevations with the requirements of other trades including but not limited to plumbing, mechanical, or electrical.
 - Place all column footings and wall footings monolithically with adjacent footings at the same elevation.
 - All footings must bear on original undisturbed soil where possible.
 - Remove all organic soils and replace with clean structural fill at the direction of the Geotechnical Engineer. Place fill soils in 10" maximum (loose) lifts at moisture contents within 4% of optimum moisture content. Compact all fill within 10'-0" of the building limit to the following minimum densities:
 - Within 18" of finished grade: 98% of maximum Standard Proctor
 - Below 18" of finished grade: 95% of maximum Standard Proctor
 - Field density tests must be made as described by the Geotechnical Engineer to verify adequate compaction and design bearing pressure.
 - Sides of foundations must be formed unless conditions permit earth forming. Foundations placed against the earth require the following precautions: slope sides of excavations as approved by the Geotechnical Engineer and clean up sloping before and during concrete placement.
 - Where footing steps are necessary, slope no steeper than one vertical to two horizontal.
 - Do not backfill against basement walls (walls supported at the top and bottom) until slabs, framing, and diaphragms are in place to provide support at top and bottom of wall. Concrete diaphragms must reach 65% of their design 28 day compression strength prior to backfilling.
 - Deposit backfill evenly against both sides of the wall until the lower finished grade is reached.
 - Unless noted otherwise, place all slabs on grade on a 10 mil polyethylene vapor retarder and a crushed stone base over a properly compacted subgrade. Lap joints of vapor retarder a minimum of 6 inches and seal watertight by taping edges and ends.

8. DESIGN LOADS
- Live Loads:

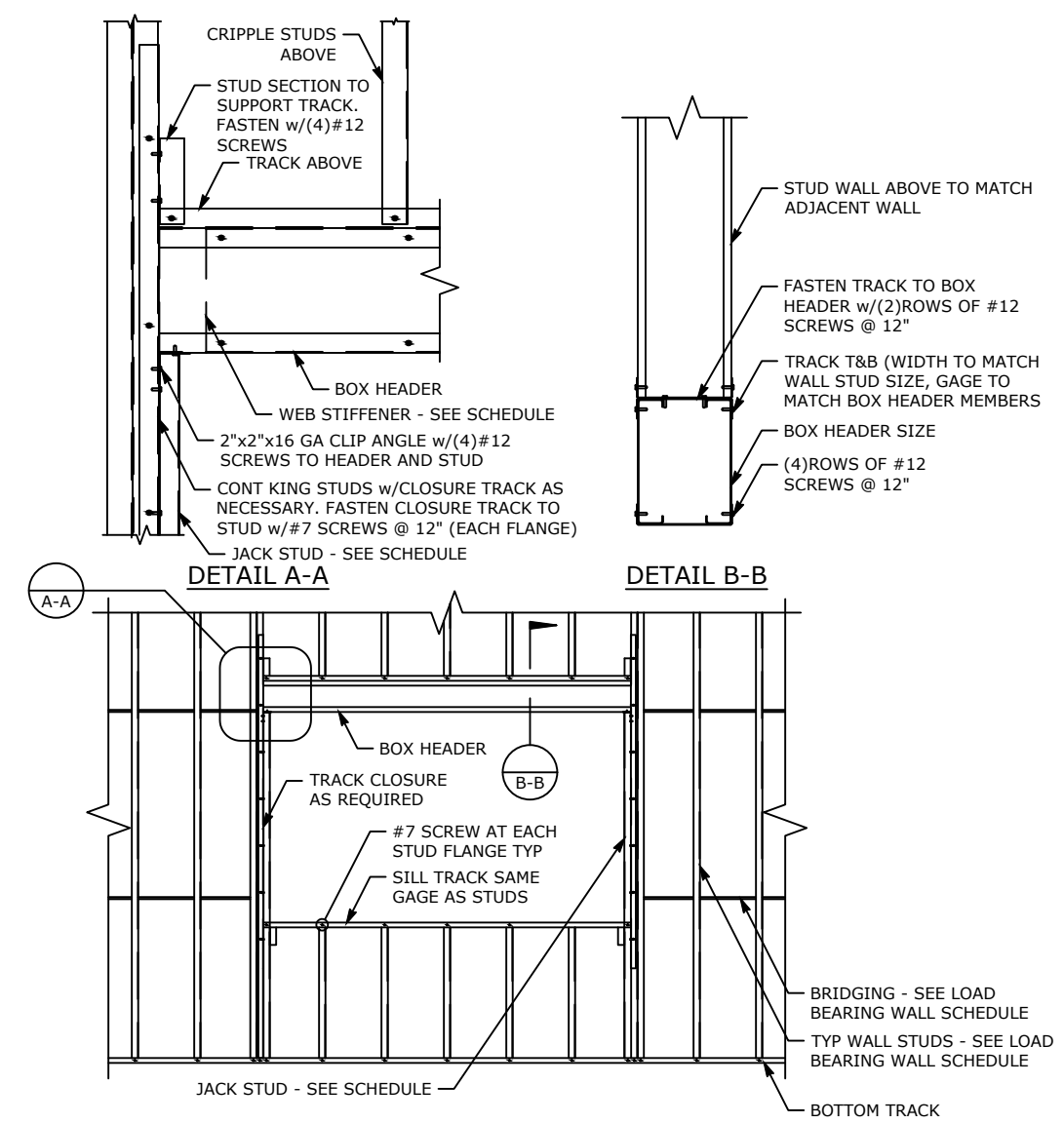
Roof	20 PSF
------	--------
 - Dead Loads:

Roof	10 PSF
------	--------
 - Wind Design Data:

Risk Category	Category II
Exposure Category	Exposure C
Basic Design Wind Speed	V: 106PH
Allowable Stress Design Wind Speed	Vsd: 82 MPH
Enclosure Classification	Enclosed Building
Internal pressure Coefficient	+ 0.18
Components and Cladding Pressures	See Table
 - Seismic Design Data:

Risk Category	Category II
Seismic Importance Factor	I = 1.0
Mapped Spectral Response Accelerations	S _a = 0.185g S ₁ = 0.085g
Site Class	Site Class D
Spectral Response Coefficients	S _{ds} = 0.197g S _{d1} = 0.137g
Seismic Design Category	Category C
Basic Seismic Force Resisting System	Concrete shear walls
Response Modification Factor	R = 2.5
Seismic Response Coefficient	C _s = 0.079
Design Base Shear	Structure complies with ASCE 7-16, Appendix 11B, Section 4 - Alterations. Loadings and member capacities change less than 10%. Seismic Retrofit Not Required.
Analysis Procedure	Equivalent Lateral Force Procedure
 - Snow Loads:

Ground Snow Load	P _g = 5PSF
Snow Load Importance Factor	I = 1.0
Snow Exposure Factor	C _e = 1.0
Thermal Factor	C _t = 1.0
Flat-Roof Snow Load	P _f = 5 PSF



COLD-FORM METAL STUD LOAD BEARING WALL HEADER SCHEDULE

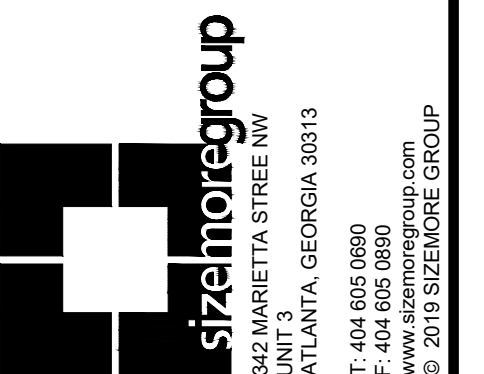
HEADER	SPAN	BOX STUD SIZE	WALL STUD SIZE
H1	0'-0" - 4'-0"	(2) R60X16-43	(2) R60X16-43

PROVIDE UNPUNCHED MATERIAL FOR ALL HEADERS.

ELEVATION 1 SCALE: N.T.S. S001

NO.	DESCRIPTION	DATE
	100% CONSTRUCTION DOCUMENTS	12-09-2021

EMORY UNIVERSITY - BOILER
1495 CLIFTON ROAD
ATLANTA, GA 30322



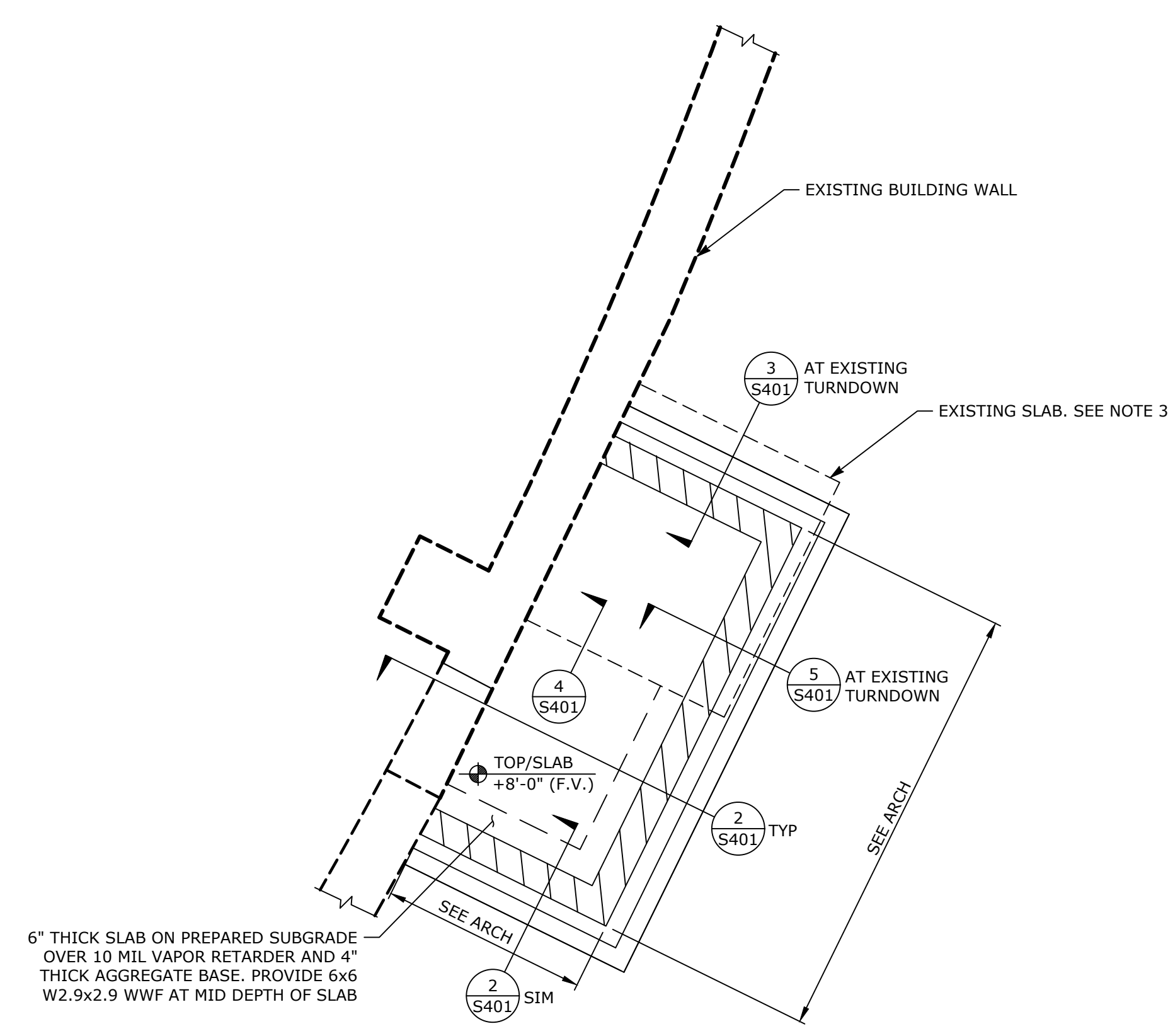
THIS DRAWING IS AN INSTRUMENT OF SERVICE AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT. NO PART OF THIS DOCUMENT SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM, ELECTRONICALLY OR MECHANICALLY, FOR ANY PURPOSE, WITHOUT THE EXPRESS WRITTEN PERMISSION OF THE ARCHITECT.

STRUCTURAL GENERAL NOTES

PROJECT # 21290EMOWDB
DATE: 12/08/21
DRAWN BY: KDB
CHECKED BY: BP

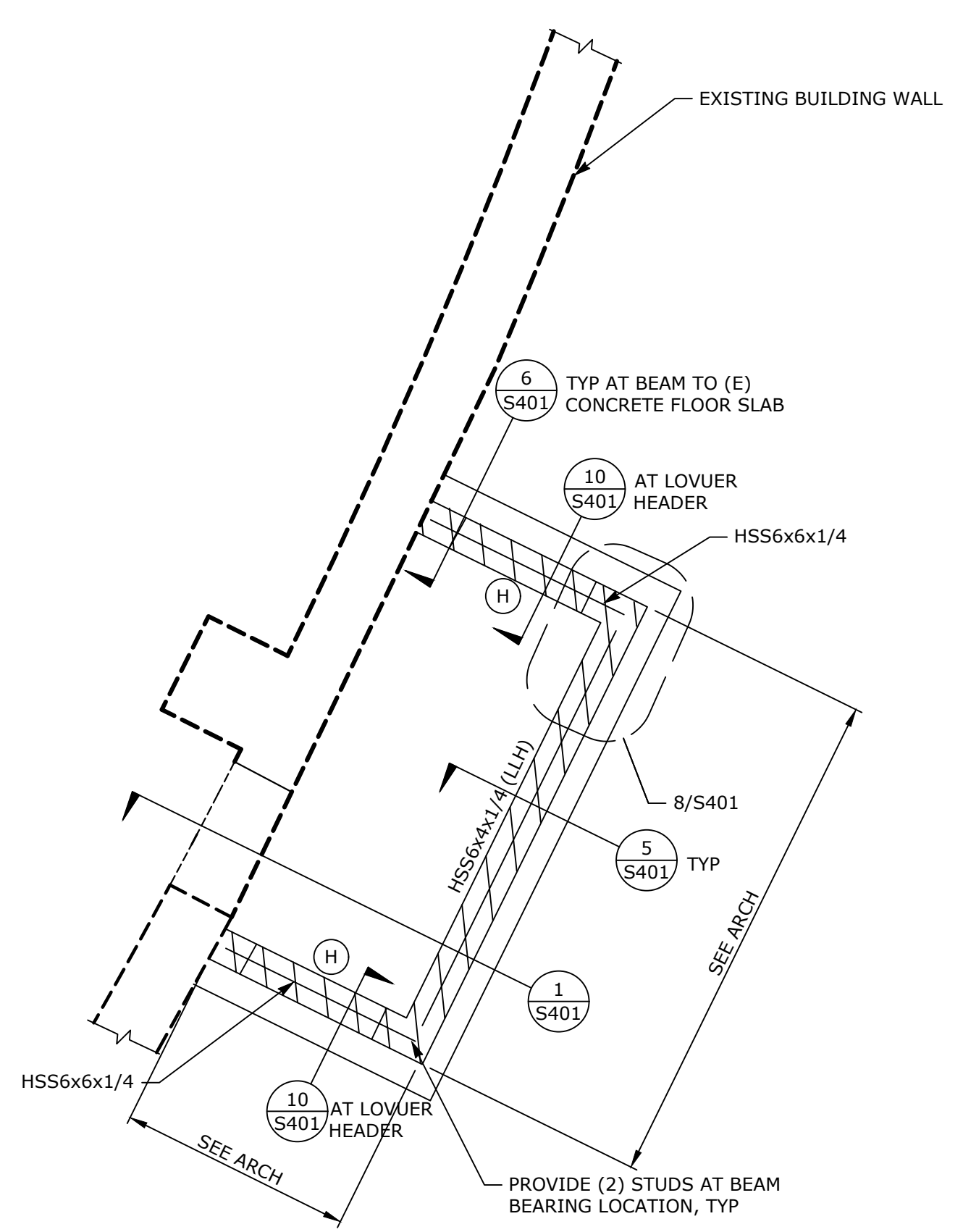
S001
SCALE

DATE	DESCRIPTION
12/08/21	100% CONSTRUCTION DOCUMENTS



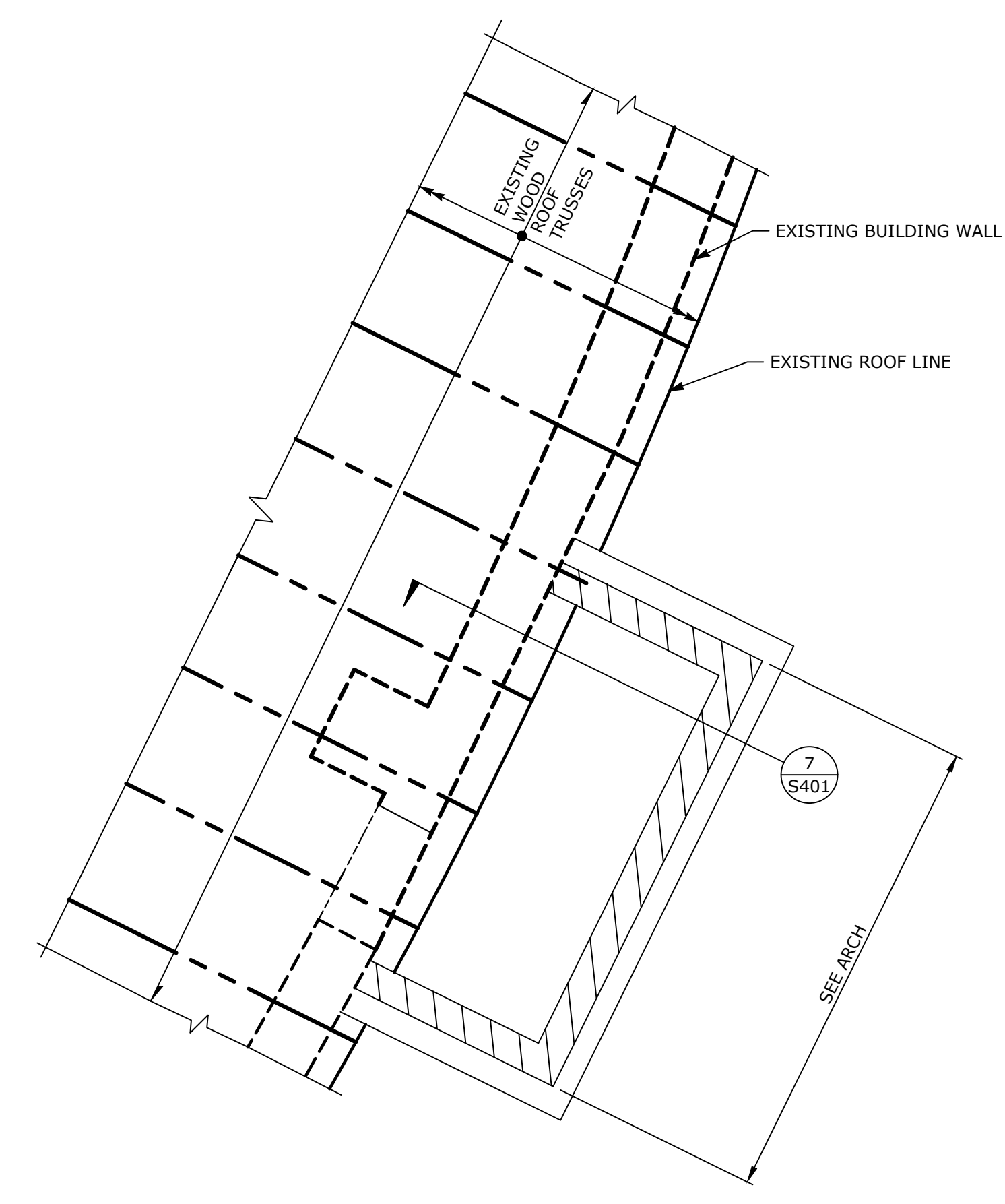
PARTIAL PLAN CHIMNEY FOUNDATION PLAN
 SCALE: 1/2"=1'-0"
S101

- NOTES:**
 1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, SLOPES, AND OTHER INFORMATION NOT SHOWN. FIELD VERIFY (F.V.) ALL ELEVATIONS AND DIMENSIONS.
 2. REF. LEVEL 1 ELEVATION= 0'-0".
 3. EXISTING FOUNDATION/ TURNED DOWN SLAB FOR CHIMNEY IS PRESUMPTIVE. INVESTIGATORY DEMOLITION WILL BE REQUIRED FOR FIELD VERIFICATION OF THESE CONDITIONS. REVISION TO THE NOTED DETAILS MAY BE REQUIRED BASED ON EXISTING CONDITION. CONTRACTOR TO PROVIDE APPROPRIATE ALLOWANCE FOR INVESTIGATION & MODIFICATIONS OF THE FOUNDATION.
- INDICATES EXISTING WALLS.
 - /// INDICATES NEW 600S162-43 METAL STUDS @ 16" o/c w/600T162-68 TOP & BOTTOM TRACK. PROVIDE 7/16" APA RATED PLYWOOD SHEATHING FASTENED TO STUDS w/ S12 SCREWS @ 6" o/c IN PERIMETER AND IN FIELD.
 - INDICATES 4" SPLIT FACE CMU VENER. ATTACH VENER TO METAL WALL STUDS w/ HOHMANN AND BARNARD 345-BT FLEXIBLE TIE w/ #8 SCREWS @ 16" o/c HORIZONTAL AND VERTICAL.



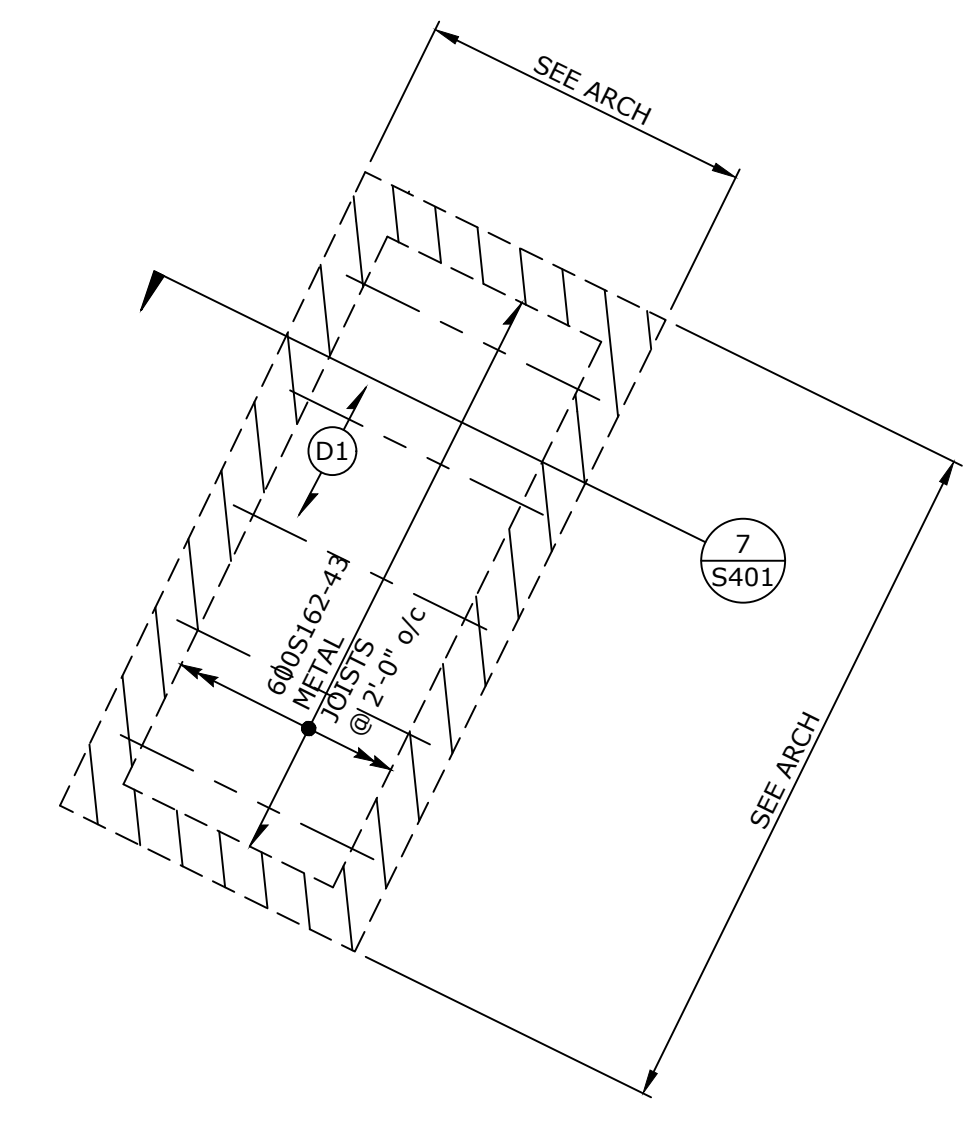
PARTIAL PLAN TYPICAL CHIMNEY FLOOR PLAN FRAMING
 SCALE: 1/2"=1'-0"
S101

- NOTES:**
 1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, SLOPES, AND OTHER INFORMATION NOT SHOWN. FIELD VERIFY (F.V.) ALL ELEVATIONS AND DIMENSIONS.
 2. REF. LEVEL 1 ELEVATION= 0'-0".
- INDICATES EXISTING WALLS.
 - /// INDICATES NEW 600S162-43 METAL STUDS @ 16" o/c w/600T162-68 TOP & BOTTOM TRACK. PROVIDE 7/16" APA RATED PLYWOOD SHEATHING FASTENED TO STUDS w/ S12 SCREWS @ 6" o/c IN PERIMETER AND IN FIELD.
 - (H) INDICATES HEADER AT OPENING FOR MECHANICAL LOUVER. COORDINATE w/ MECHANICAL AND ARCHITECTURAL. SEE 1/S001 FOR HEADER SIZE AND ADD'L INFORMATION FOR CFS HEADER. SEE 10/S401 FOR DETAIL AT LOUVER.



PARTIAL PLAN CHIMNEY FRAMING PLAN AT EXISTING ROOF FRAMING
 SCALE: 1/2"=1'-0"
S101

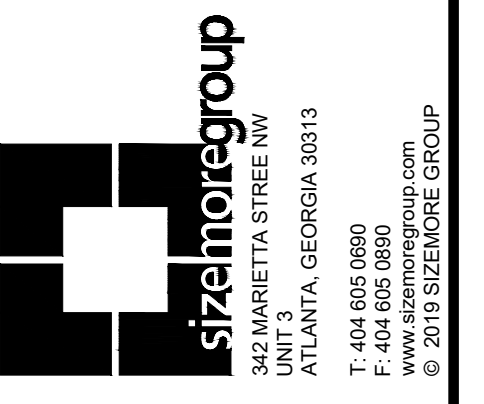
- NOTES:**
 1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, SLOPES, AND OTHER INFORMATION NOT SHOWN. FIELD VERIFY (F.V.) ALL ELEVATIONS AND DIMENSIONS.
 2. REF. LEVEL 1 ELEVATION= 0'-0".
- INDICATES EXISTING WALLS.
 - /// INDICATES NEW (2) 600S162-43 METAL STUDS @ 16" o/c w/600T162-68 TOP & BOTTOM TRACK. PROVIDE 7/16" APA RATED PLYWOOD SHEATHING FASTENED TO STUDS w/ S12 SCREWS @ 6" o/c IN PERIMETER AND IN FIELD.



PARTIAL PLAN CHIMNEY ROOF FRAMING PLAN
 SCALE: 1/2"=1'-0"
S101

- NOTES:**
 1. SEE ARCHITECTURAL DRAWINGS FOR DIMENSIONS, ELEVATIONS, SLOPES, AND OTHER INFORMATION NOT SHOWN. FIELD VERIFY (F.V.) ALL ELEVATIONS AND DIMENSIONS.
 2. REF. LEVEL 1 ELEVATION= 0'-0".
- /// INDICATES NEW (2) 600S162-43 METAL STUDS @ 16" o/c w/600T162-68 TOP & BOTTOM TRACK. PROVIDE 7/16" APA RATED PLYWOOD SHEATHING FASTENED TO STUDS w/ S12 SCREWS @ 6" o/c IN PERIMETER AND IN FIELD.
 - (D1) INDICATES 3/4" THICK APA RATED PLYWOOD SHEATHING FASTENED TO METAL JOISTS w/ S12 SCREWS @ 6" o/c IN FIELD AND AT PERIMETER.

EMORY UNIVERSITY - BOILER
 1495 CLIFTON ROAD
 ATLANTA, GA 30322

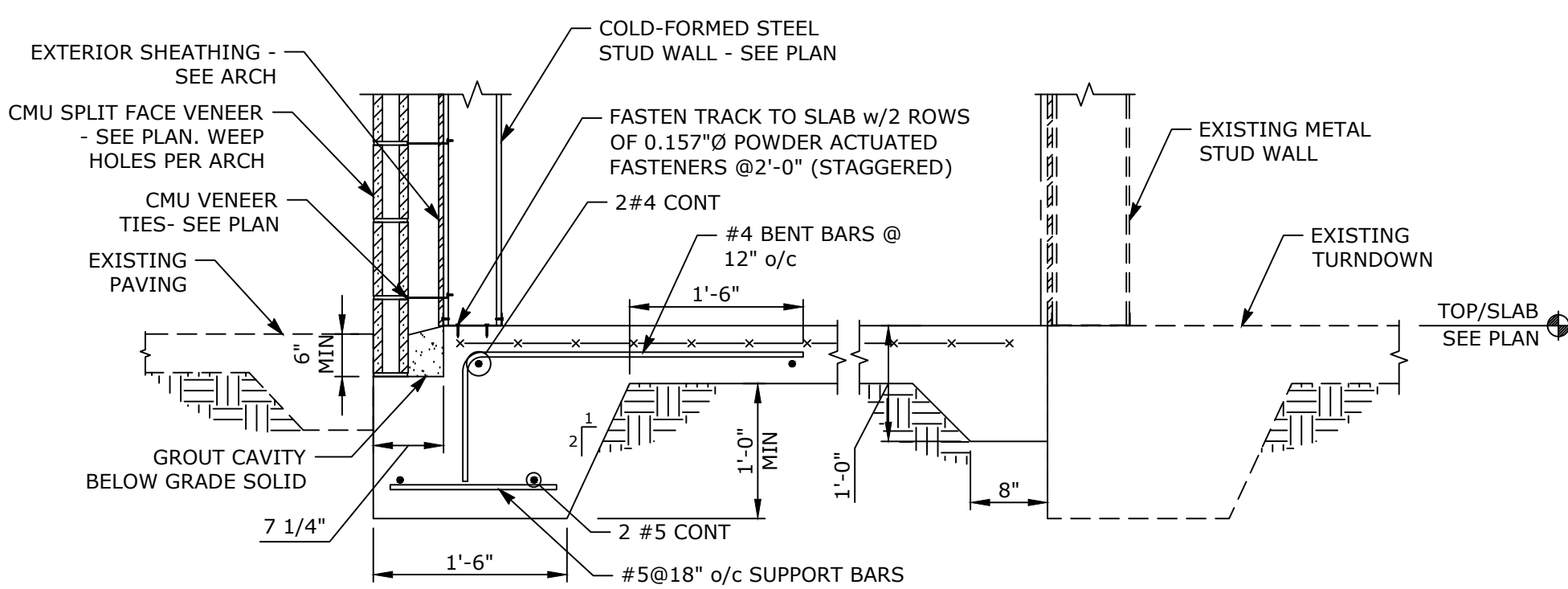
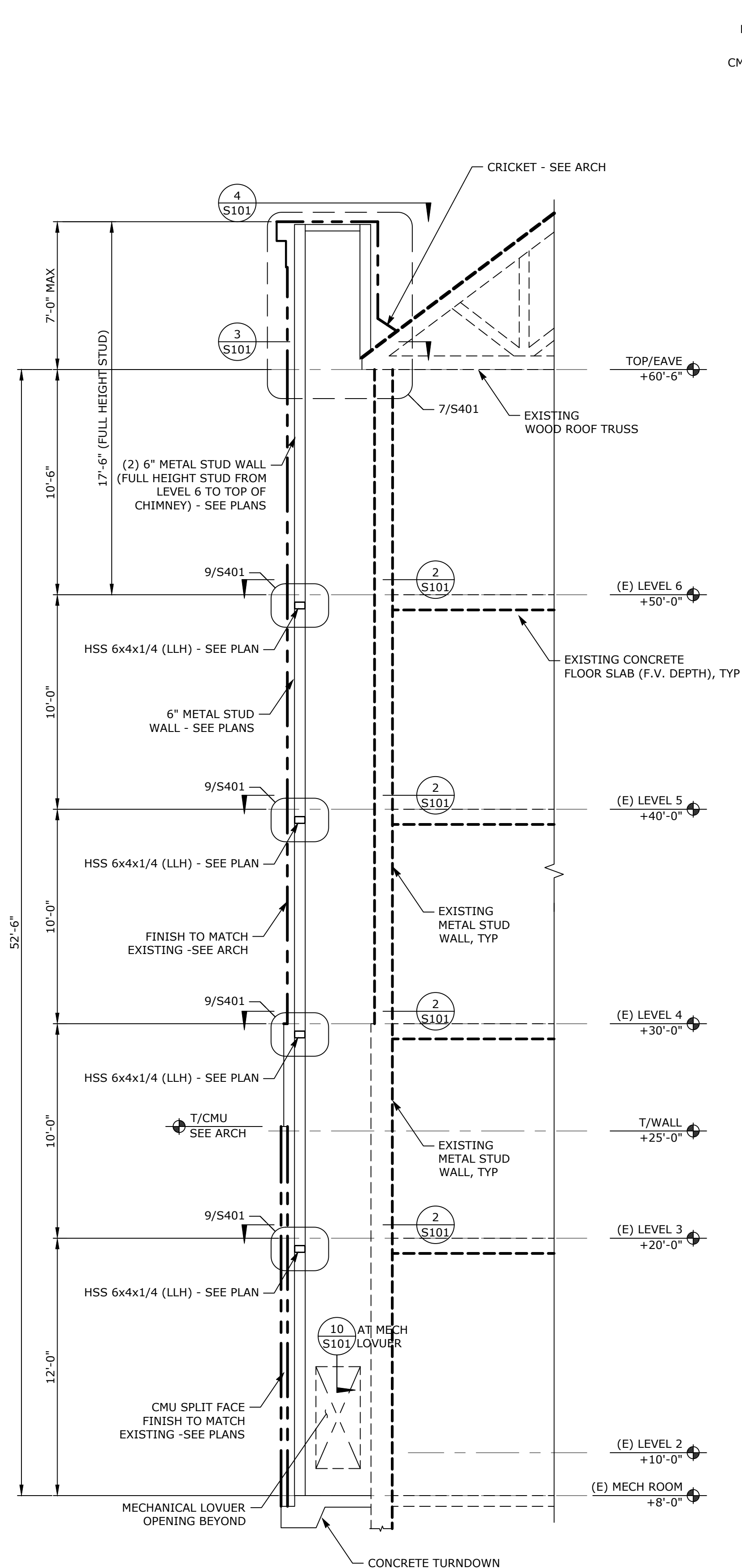


THIS DRAWING IS AN INSTRUMENT OF SERVICE AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT. NO PART OF THIS DOCUMENT SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM, ELECTRONICALLY OR MECHANICALLY, FOR ANY PURPOSE, WITHOUT THE EXPRESS WRITTEN PERMISSION OF THE ARCHITECT.

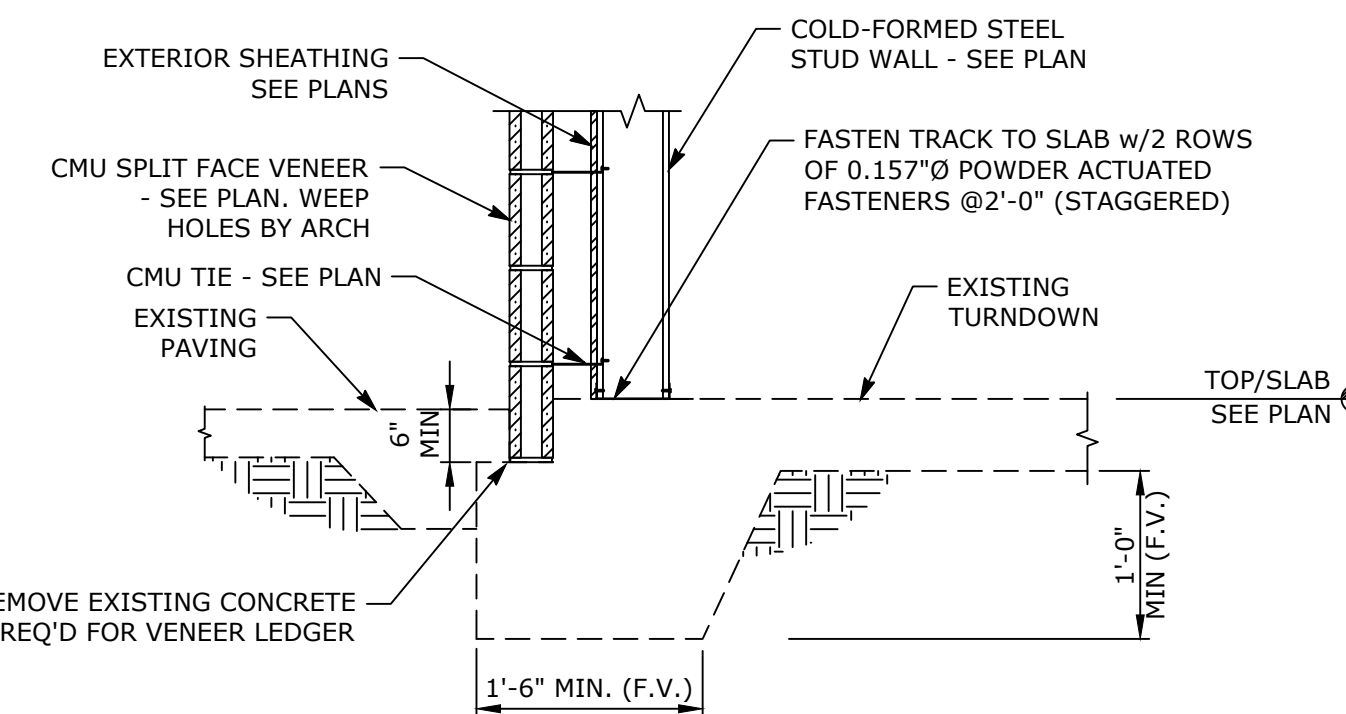
STRUCTURAL FOUNDATION AND FRAMING PLAN - CHIMNEY

PROJECT # 21290EMOWDB
 DATE: 12/08/21
 DRAWN BY: KDB
 CHECKED BY: BP

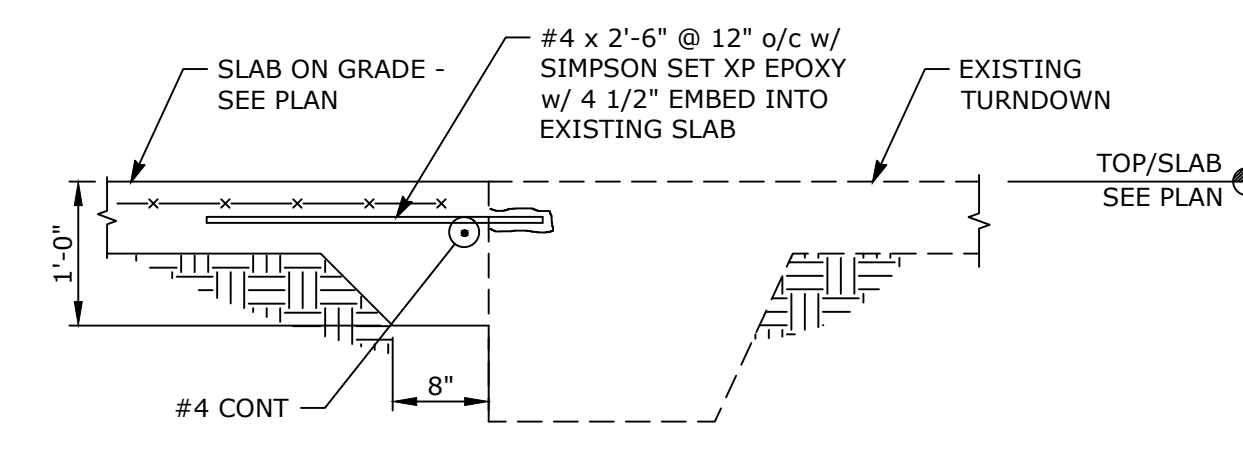
S101
 SCALE



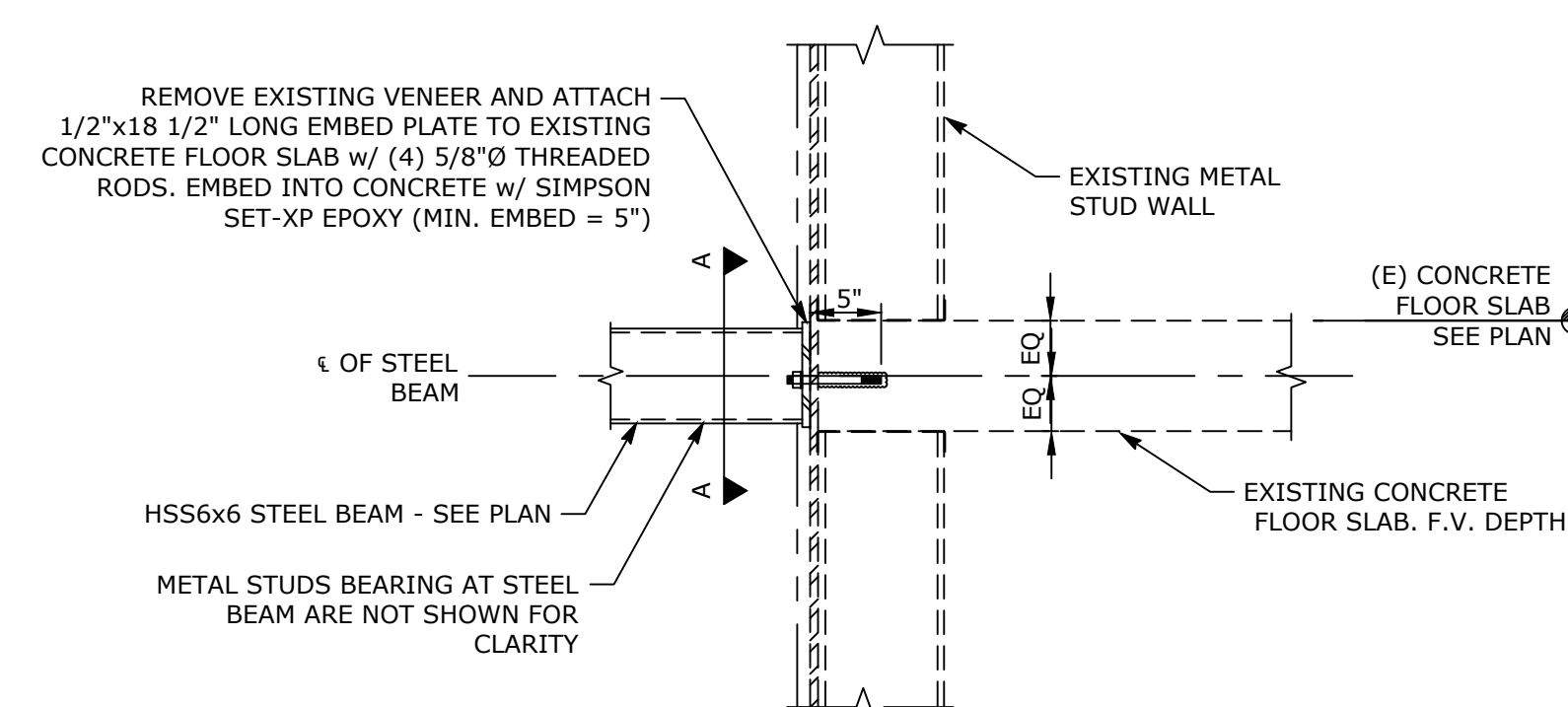
SECTION TYPICAL TURNDOWN SLAB AT EXTERIOR CHIMNEY WALL
 SCALE: 3/4"=1'-0" S401



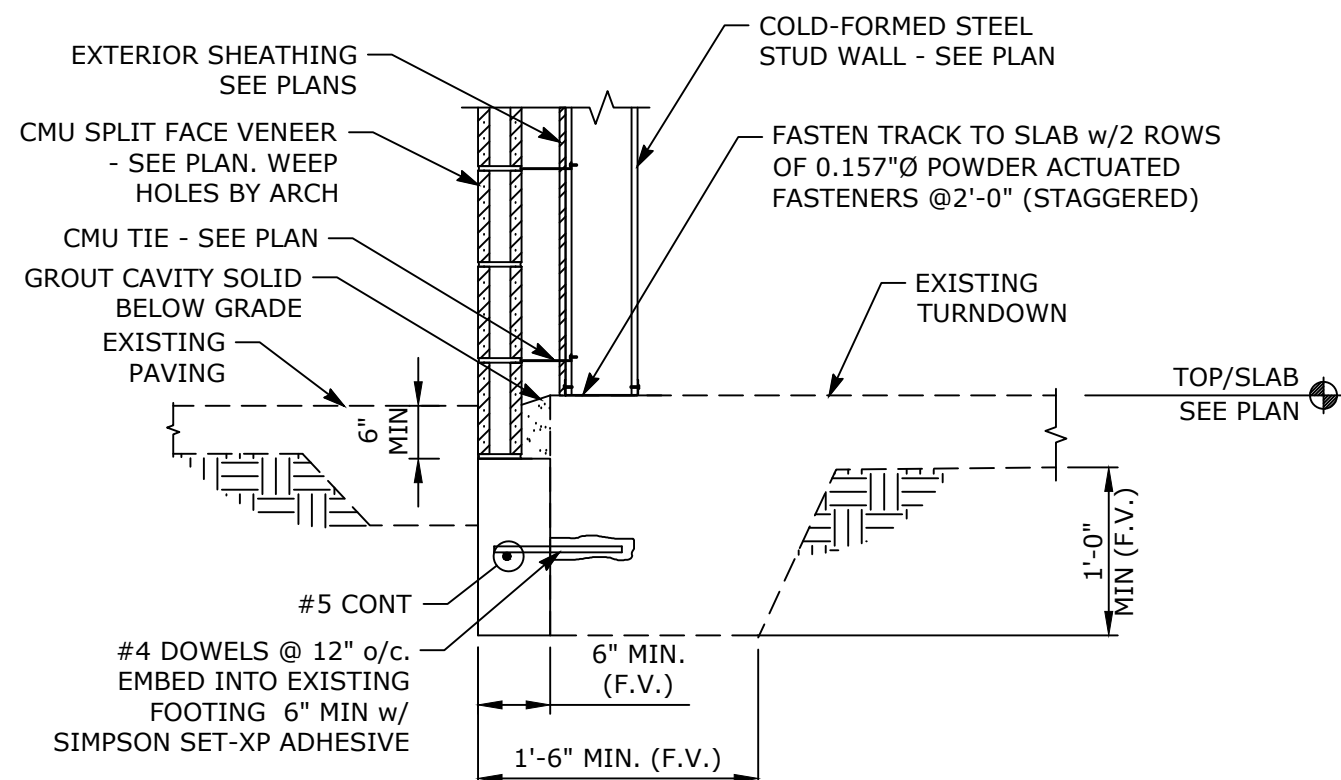
SECTION TYPICAL EXTERIOR CHIMNEY WALL AT EXISTING TURNDOWN
 SCALE: 3/4"=1'-0" S401



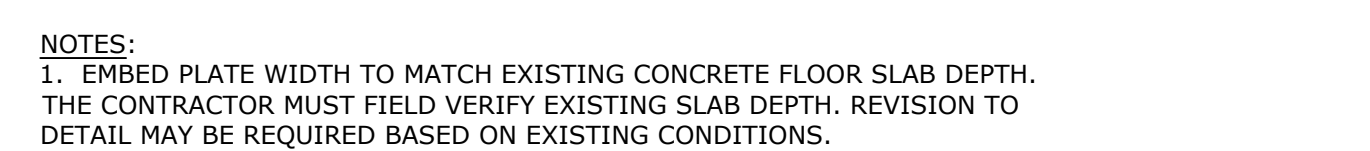
SECTION NEW SLAB AT EXISTING TURNDOWN
 SCALE: 3/4"=1'-0" S401



SECTION TYPICAL STEEL BEAM AT EXISTING CONCRETE FLOOR
 SCALE: 1"=1'-0" S401

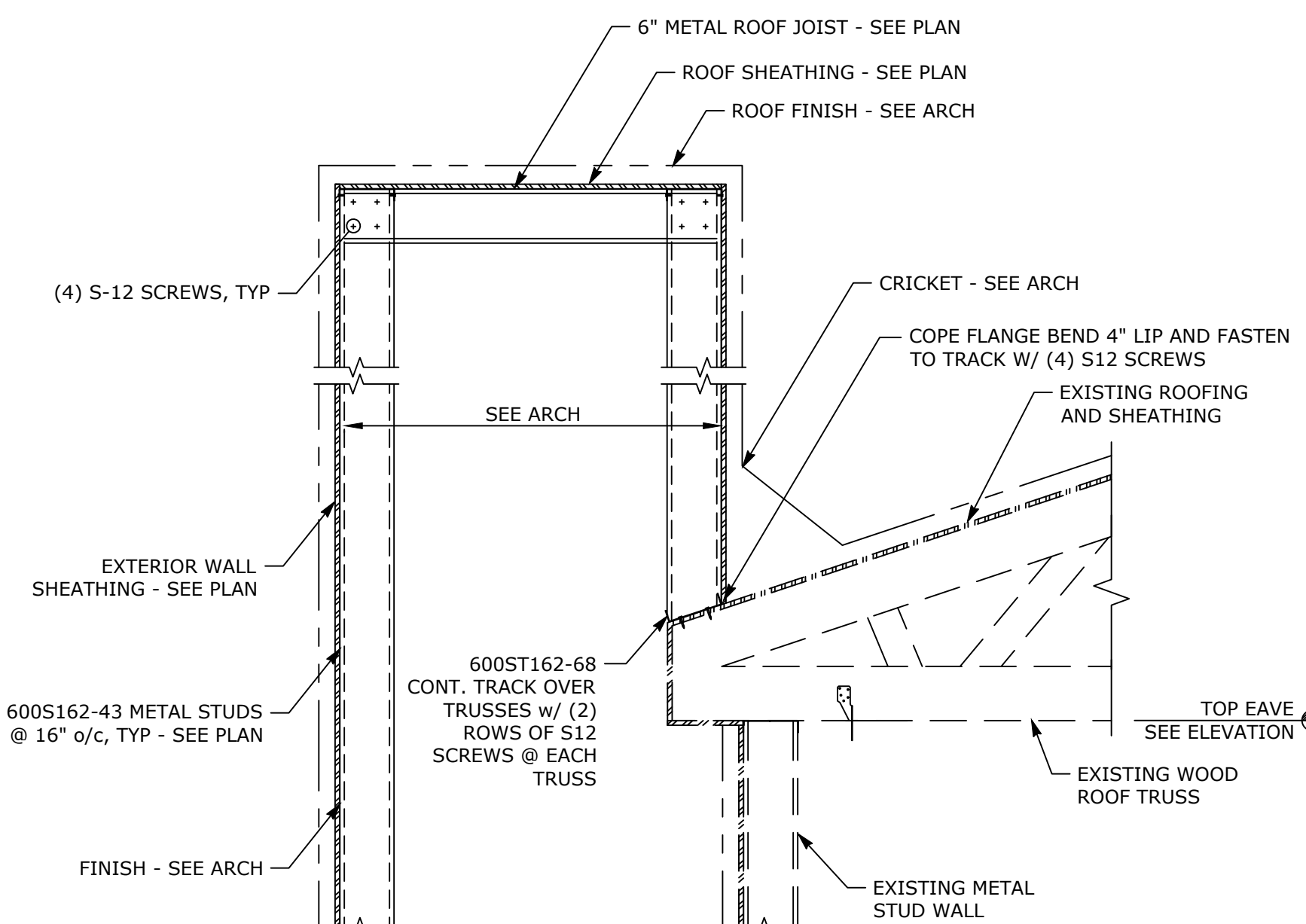


SECTION TYPICAL EXTERIOR CHIMNEY WALL AT EXISTING TURNDOWN
 SCALE: 3/4"=1'-0" S401

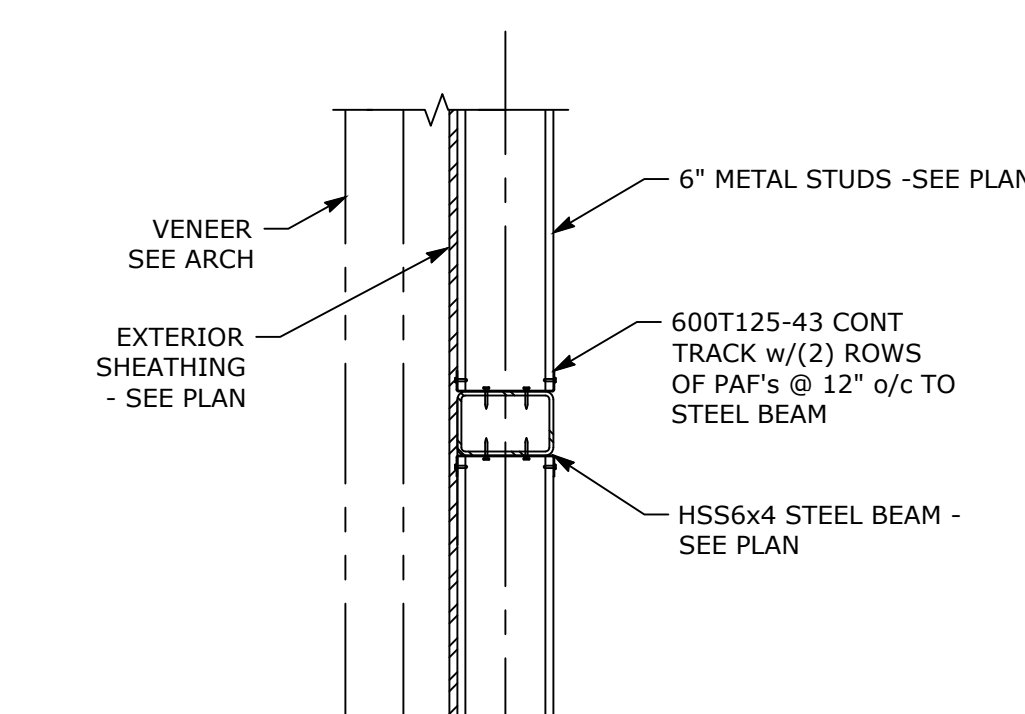


DETAIL A-A

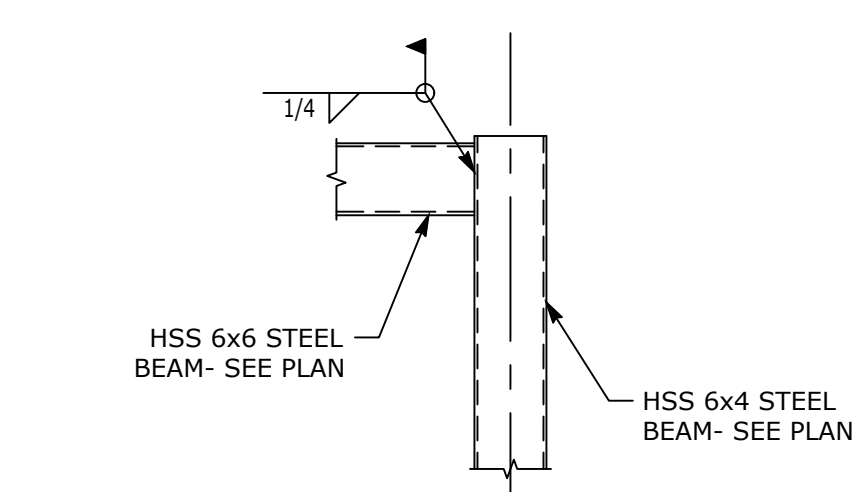
NOTES:
 1. EMBED PLATE WIDTH TO MATCH EXISTING CONCRETE FLOOR SLAB DEPTH. THE CONTRACTOR MUST FIELD VERIFY EXISTING SLAB DEPTH. REVISION TO DETAIL MAY BE REQUIRED BASED ON EXISTING CONDITIONS.



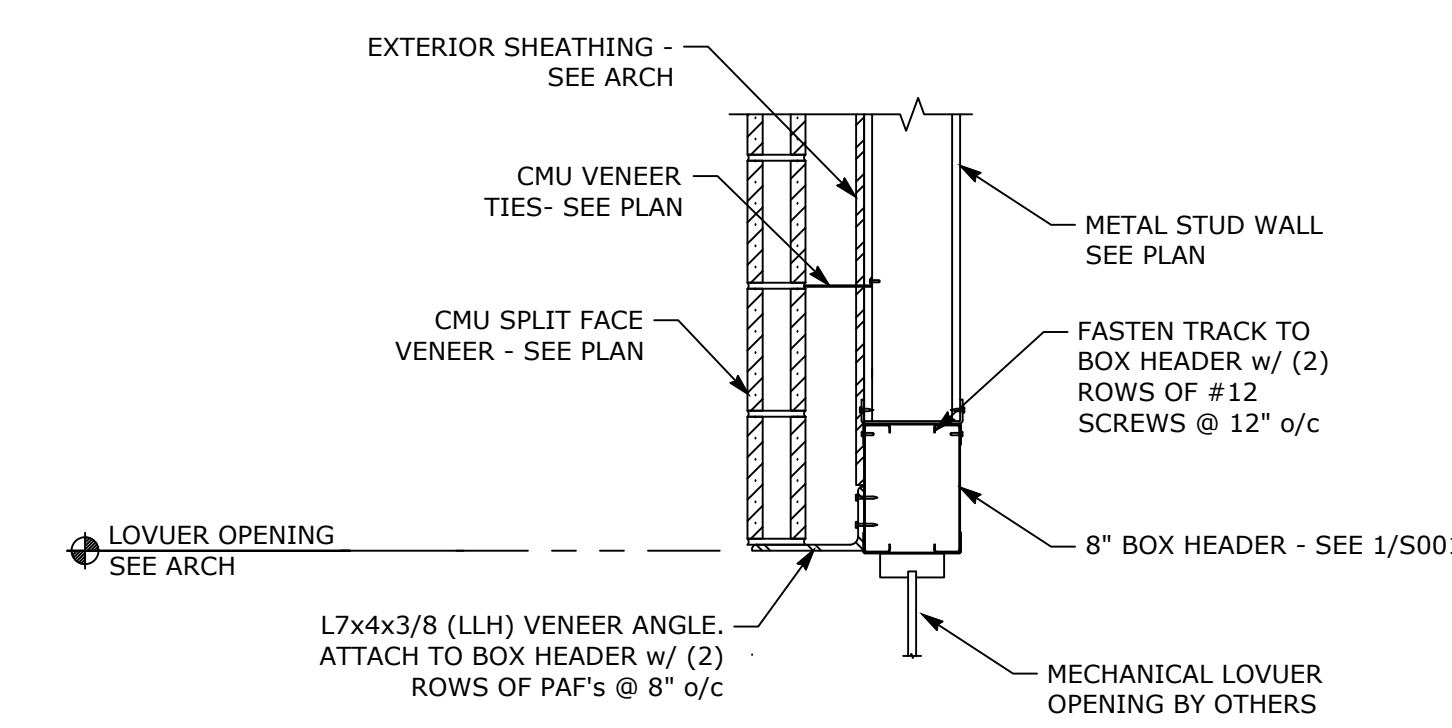
SECTION AT METAL STUD CHIMNEY
 SCALE: 3/4"=1'-0" S401



SECTION TYPICAL METAL STUDS AT STEEL BEAM
 SCALE: 1"=1'-0" S401



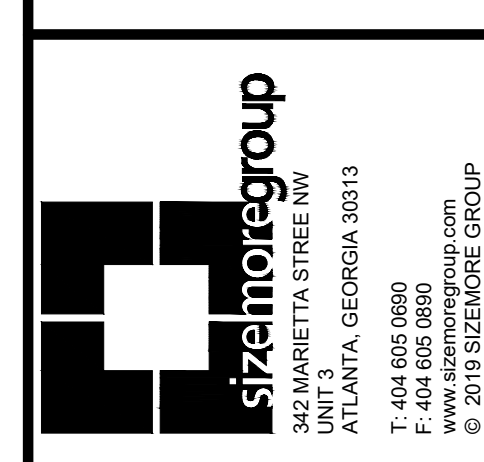
DETAIL TYPICAL TUBE BEAM TO TUBE BEAM CONNECTION
 SCALE: 3/4"=1'-0" S401



SECTION TYPICAL VENEER LINTEL AND HEADER AT MECHANICAL LOUVER OPENING
 SCALE: 1"=1'-0" S401

DATE	DESCRIPTION
12-08-2021	100% CONSTRUCTION DOCUMENTS

EMORY UNIVERSITY - BOILER
 1495 CLIFTON ROAD
 ATLANTA, GA 30322



THIS DRAWING IS AN INSTRUMENT OF SERVICE AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT. NO PART OF THIS DOCUMENT SHALL BE REPRODUCED OR TRANSMITTED IN ANY FORM, ELECTRONICALLY OR MECHANICALLY, FOR ANY PURPOSE, WITHOUT THE EXPRESS WRITTEN PERMISSION OF THE ARCHITECT.

STRUCTURAL ELEVATIONS, SECTIONS, AND DETAILS

PROJECT # 21290EMOWDB
 DATE: 12/08/21
 DRAWN BY: KDB
 CHECKED BY: BP

S401
 SCALE